

November 2017

February 2018

C-147 Registration Package for Merchant Recycling Containment and Recycling Facility Section 35 T21S R33E, Lea County



View to northeast from the center of the proposed containment showing the vegetated sand dunes and the rig drilling the Merchant State Unit 503H

Prepared for:

**ADVANCE ENERGY PARTNERS HAT MESA, LLC
11490 Westheimer Rd. STE 950
Houston, TX 77077**

Prepared by:

**R.T. Hicks Consultants, Ltd.
901 Rio Grande NW F-142
Albuquerque, New Mexico**

R. T. HICKS CONSULTANTS, LTD.

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February 12, 2018

Ms. Olivia Yu and Mr. Bradford Billings
NMOCD District 1
1625 French Drive
Hobbs, NM 88240
Via E-Mail

RE: Advance Energy, Merchant Containment C-147 Registration

Dear Ms. Yu and Mr. Billings:

On behalf of Advance Energy, R.T. Hicks Consultants submits the attached registration update to allow OCD to track conformance of registered containments to specific stipulations of the Rule. This submission includes the following changes to the original submission:

1. An equivalency demonstration written by experts for the proposed 40-mil HDPE secondary liner. Although the language of the Rule is clear¹ and OCD has approved this change as a variance for others, we have re-named the equivalency demonstration a variance.
2. Form C-147 with the registration and permit box checked for the recycling facility and containment.

Appendix A includes the following elements that should be included as “variances” for the purpose of OCD’s database:

3. The Avian Protection Plan, which has been approved by OCD for other containments and meets the requirement of the rule
4. A game fence, in lieu of a 4-strand barbed wire fence.

We included the original cover letter as it provides the demonstration that construction and use of the registered containment is consistent with the Rule. If you have any questions or concerns regarding this amendment to the registration or the attached C-147, please contact me. As always, we appreciate your work ethic and attention to detail.

Sincerely,
R.T. Hicks Consultants



Randall Hicks
Principal

Copy: Advance Energy

¹ Secondary liners shall be 30-mil LLDPE string reinforced or equivalent with a hydraulic conductivity no greater than 1×10^{-9} cm/sec

R. T. HICKS CONSULTANTS, LTD.

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November 30, 2017

Ms. Olivia Yu
Mr. Bradford Billings
NMOCD District
1625 French Drive
Hobbs, NM 88240
Via E-Mail

Original Transmission of Registration

RE: Advance Energy, Merchant Containment C-147 Registration

Dear Ms. Yu and Mr. Billings:

On behalf of Advance Energy, R.T. Hicks Consultants submits the attached registration. Grading, compaction and geotechnical testing of the containment and liner foundation will be complete Saturday, December 2. Please note that the as-built drawings show some depressions in the the sloping bottom of the containment. The slope will be re-graded on Saturday to create a smooth sloping surface with no such depressions. As indicated in previous correspondence, Advance will begin lining the containment on December 4. No variances from the Rule are necessary and this submittal demonstrates compliance with all mandates of the Rule for the containment. Since the recycling facility meets the criteria of 19.15.34.9.B.7, the facility also requires a registration. Thus, the Rule does not require approval by OCD in advance of using the containment. This submission includes:

1. An equivalency demonstration written by experts for the proposed 40-mil HDPE secondary liner. We see that OCD has previously concurred that this material is equivalent to the 30-mil LLDPEr specified in the Rule for a secondary liner. We maintain that the language of the Rule is clear¹ and a variance is not required.
2. Form C-147 with the registration box checked for the recycling facility and containment.
3. Site specific information that demonstrates compliance with siting criteria for the location.
4. Appendix A that includes
 - a. Engineering design for the containment
 - b. Specifications for the liner system and a liner installation protocol.
 - c. The proposed Avian Protection Plan is exactly the same as those approved by OCD for other containments. Thus, the plan meets the requirement of the rule that the “otherwise protective of wildlife, including migratory birds” and a variance is not required.
 - d. Specifications for a game fence in lieu of a 4-strand barbed wire fence. Because feral pigs, javelena and deer are present in the area, an 8-foot high game fence is required in order to comply with Section 19.15.34.12 D.1 of the Rule². The specification for fencing provided in 19.15.34.12 D.2 contradicts D.1 because pigs will move beneath the lower strand of a 4-strand, 4-foot high barbed wire fence and deer will jump over. Thus, compliance with D.2 results in a violation of D.1. We maintain that compliance with D.1 is the critical component of the Rule and operators need not be required to

¹ Secondary liners shall be 30-mil LLDPE string reinforced or equivalent with a hydraulic conductivity no greater than 1×10^{-9} cm/sec

² The operator shall fence or enclose a recycling containment in a manner that deters unauthorized wildlife and human access and shall maintain the fences in good repair.

submit a variance request in order to follow Best Management Practices and comply with the Rule.

5. A design/construction plan, operating and maintenance plan, and closure plan.

In compliance with 19.15.34.10 of the Rule, this submission is copied to Merchant Livestock Company who represents the surface owner of the private surface upon which the containment is constructed.

If you have any questions or concerns regarding this amendment to the registration or the attached C-147, please contact me. As always, we appreciate your work ethic and attention to detail.

Sincerely,
R.T. Hicks Consultants

A handwritten signature in black ink, appearing to read "Randall Hicks", written in a cursive style.

Randall Hicks
Principal

Copy: Advance Energy
Merchant Livestock

Statement Explaining Why the Applicant Seeks ~~Equivalency~~* ~~§ 8(a)(6)~~

The prescriptive mandates of the Rule that are the subject of this demonstration of equivalency are the following with *emphasis* added:

19.15.34.12 A. (4) All primary (upper) liners in a recycling containment shall be geomembrane liners composed of an impervious, synthetic material that is resistant to ultraviolet light, petroleum hydrocarbons, salts and acidic and alkaline solutions. All primary liners shall be 30-mil flexible PVC, 45-mil LLDPE string reinforced or 60-mil HDPE liners. *Secondary liners shall be 30-mil LLDPE string reinforced or equivalent with a hydraulic conductivity no greater than 1×10^{-9} cm/sec.* Liner compatibility shall meet or exceed the EPA SW-846 method 9090A or subsequent relevant publications.


There are several reasons for requesting OCD agree that 40-mil HDPE for the secondary liner is equivalent to 30 mil LLDPEr. In addition to the reasons described in detail in the attached documents of support are the following logistical reasons:

1. The 30-mil LLDPEr is comprised of two 15-mil panels of LLDPE that are bonded together with the string mesh between. A minor concern is a puncture to the upper panel could allow fluid to wick along the string mesh. Thus, chemicals that may be slightly deleterious to 30-mil LLDPE can attack the 15-mil upper panel from the top surface and from the fluid in the wet mesh. A single layer of HDPE eliminates this potential problem.
2. Non-destructive testing of liner seams is not possible with the string-reinforced LLDPE material.
3. HDPE is more available in the market than LLDPEr

As the installed cost of the 30-mil LLDPEr is essentially the same as the smooth 40-mil HDPE, economics is not a factor in the decision to use HDPE in lieu of LLDPE.

We believe that the attached documents provide the demonstration that 40-mil HDPE is equivalent to 30-mil LLDPEr as a secondary liner. Some attributes of LLDPE superior to HDPE and some are inferior. On balance, we believe that performance is equivalent, based upon our reading of the attached documents and consideration of the logistical concerns listed above.

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November 3, 2017

Mr. Rod Kirch
Externe Plastics Plus
15931 Interstate 35 Frontage Road
Moore, TX 78057

Project: Containment Liner

RE: Hydraulic Performance of HDPE

Dear Rod:

40-mil HDPE provides equal or better protection of fresh water, public health and the environment than 30-mil string reinforced LLDPE. Also the 40-mil HDPE liner has a hydraulic conductivity no greater than 1×10^{-9} cm/sec. Liner compatibility shall meet or exceed the EPA SW-846 method 9090A or subsequent relevant publications.

Respectfully,

A handwritten signature in black ink, appearing to read "Clay Reichert".

Clay Reichert, P.E.
Technical Manager

R.K. FROBEL & ASSOCIATES
Consulting Engineers

Mr. Randall Hicks, PG
R.T. Hicks Consultants Ltd.
901 Rio Grande NW
Suite F-142
Albuquerque, New Mexico 87104

November 26, 2017

RE: Technical Memorandum
40 mil HDPE Geomembrane Equivalency as a Secondary Liner System
Merchant Recycling Containment and Recycling Facility
Lea County, New Mexico

Dear Mr. Hicks:

At your request, I have investigated the suitability of application for 40 mil HDPE geomembrane as an equivalent secondary liner to 30 mil scrim reinforced LLDPE (LLDPEr) in the Merchant Recycling Facility Containment Design. In summary, it is my professional opinion that the specified 40 mil HDPE geomembrane will provide a secondary liner system that is equal to or better than 30 mil scrim reinforced LLDPEr and will provide the requisite protection of fresh water, public health and the environment for many years and especially for the estimated design and operation life of the Merchant Recycling Containment.

I have reviewed your C-147 Registration Package and the Design documentation provided by R.T Hicks Consultants. It is understood that the lining system is composed of a 60 mil HDPE Primary liner, geonet drainage layer and a 40 mil HDPE Secondary liner. In consideration of the secondary lining system application, size of impoundment and depth, design details as well as the chemical nature of typical processed water, it is my professional opinion that the 40 mil HDPE geomembrane will provide the requisite barrier against processed water loss and will function effectively as a secondary liner. The following are discussion points that hopefully will exhibit the equivalency of a 40 mil HDPE secondary liner to that of a 30 mil LLDPEr.

The nature and formulation of the 40 mil HDPE resin is the same as the Primary 60 mil HDPE. The major difference is that the 40 mil HDPE is lower in thickness (more flexible and less puncture resistant). However, in covered conditions, HDPE will resist aging and degradation and remain intact for many decades. In fact, a secondary liner of 40 mil HDPE will outlast an exposed 60 mil HDPE liner. According to the Geosynthetic Research Institute (GRI) study on lifetime prediction (GRI Paper No. 6), the half life of HDPE (GRI GM 13) exposed is > 36 years and the half-life of HDPE covered or buried is greater than 100 years (the Merchant Recycling Facility life span is expected to be only 10 years maximum). It is understood that in order to ensure compliance of materials, the primary 60 mil HDPE to be used must meet or exceed GRI GM 13 Standards. Likewise, the secondary liner that is not exposed to the same environmental and chemical conditions must meet or exceed GRI GM 13 for non-reinforced HDPE. Adhering to the

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minimum requirements of the GRI Specifications, 40 mil HDPE when used as a secondary liner will be equally as protective as the primary 60 mil HDPE liner (reference: www.geosynthetic-institute.org/grispecs) and equally as protective as a 30 mil scrim reinforced LLDPEr liner.

Durability of Geomembranes is directly affected by exposure conditions. Buried or covered geomembranes are not affected by the same degradation mechanisms (UV, Ozone, Chemical, Stress, Temperature, etc) as are fully exposed geomembranes. In this regard, the secondary liner material and thickness can be much less robust than the fully exposed primary liner which in this case is 60 mil HDPE. This is also the case for landfill lining systems where the secondary geomembrane in a bottom landfill cell may be 40 mil HDPE.

Thermal Fusion Seaming Requirements. Thermal seaming and QC seam test requirements for geomembranes are product specific and usually prescribed by the sheet manufacturer. Dual wedge thermal fusion welding is commonly used on HDPE and QC testing by air channel (ASTM D 5820) is fully acceptable and recognized as an industry standard. In this regard, there should be no exception requirement for seaming and QC testing as both the Primary and Secondary geomembranes are HDPE. This is fully covered in comprehensive specifications for both the Primary and Secondary geomembranes (Reference: www.ASTM.org/Standards).

Potential for Leakage through the Primary and Secondary Liners. Leakage through geomembrane liners is directly a function of the height of liquid head above any hole or imperfection. The geonet drainage media provides immediate drainage to a low point or sump and thus no hydrostatic head or driving gradient is available to push leakage water through a hole in the secondary liner. In this regard, secondary geomembrane materials can be (and usually are) much less in thickness and also polymer type. Hydraulic Conductivity through the 40 mil HDPE liner material is extremely low due to the polymer type, structure and crystallinity and exceeds requirements of EPA SW-846 Method 9090A.

Chemical Attack. Chemical attack to polymeric geomembranes is directly a function of type of chemical, temperature and exposure time. Again, the HDPE Primary provides the chemically resistant liner and is QC tested to reduce potential defects or holes. If there is a small hole, the geonet drain takes any leakage water immediately to the sump for extraction. Thus, exposure time is very limited on a secondary liner in addition to low temperature, little volume and virtually no head pressure. In this regard, a chemically resistant geomembrane material such as 40 mil HDPE can be specified for the secondary and is a fully acceptable alternate to 30 mil scrim reinforced LLDPEr.

Mechanical Properties Characteristics. Geomembranes of different polymer and/or structure (i.e., reinforced vs non-reinforced) can not be readily compared using such characteristics as tensile stress/strain, tear, puncture and polymer requirements. For a 40 mil HDPE liner material to function as a Secondary liner it should meet or exceed the manufacturers minimum requirements for Density, Tensile Properties, Tear, Puncture as

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well as other properties such as UV resistance. The sheet material must also meet or exceed GRI GM 13 minimum requirements. In this regard, a 40 mil HDPE will be equivalent to a 30 mil LLDPE as a secondary liner for the conditions listed below:

- The subgrade or compacted earth foundation will be smooth, free of debris or loose rocks, dry, unyielding and will support the lining system.
- The side slopes for the containment shall be equal to or less than 3H:1V.
- The physical properties and condition of the subgrade or liner foundation (i.e., density, slope, moisture) will be inspected and certified by a Professional Engineer that it meets or exceeds specification requirements.
- Immediately prior to installation, the installation contractor shall inspect and sign off on the subgrade conditions that they meet or exceed the HDPE manufacturer and installers requirements.
- A protective geotextile will be placed on the finished and accepted subgrade between subgrade and the 40 mil HDPE Secondary liner.
- A 200 mil geonet will be placed over the 40 mil HDPE Secondary Liner.
- A 60 mil HDPE Primary liner will be placed over the 200 mil geonet drainage layer.

If you have any questions on the above technical memorandum or require further information, give me a call at 720-289-0300 or email geosynthetics@msn.com

Sincerely Yours,

R K Frobel

Ronald K. Frobel, MSCE, PE

References:

C-147 Registration Package
Merchant Recycling and Containment
Section 35 T 21 SR 33 E Lea County
November 2016

Design Documents
Merchant Recycling and Containment

Geosynthetic Research Institute (GRI) Published Standards and Papers 2017
www.geosynthetic-institute.org

ASTM Geosynthetics Standards 2017
www.ASTM.org/Standards

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November 2, 2017

Attn: Mr. Rod Kirch

Project: Containment Liner

RE: UV Resistance and Chemical Resistance of HDPE

Dear Rod,

Polyethylene (PE) has a simple chain structure with a repeating unit, $-(CH_2)-$. Its physical properties are greatly dependent upon the chain length, structure, and density. HDPE is a semi-crystalline polymer which consists of amorphous (disorder region) and crystalline (ordered structure phases). Due to the long linear chain structure, the degree of crystallinity of HDPE is much higher than LLDPE. The differences in crystallinity can affect the oxidation behavior partly because the diffusion of oxygen through the amorphous region is much easier than through the crystalline region.

Regarding the UV resistance of the two type of polyethylene, attached is the GRI White Paper #6, Geomembrane Lifetime Prediction: Unexposed and Exposed Condition. On page 10, the authors stated "The nature of the LLDPE resin and its formulation is very similar to HDPE. The fundamental difference is that LLDPE is a lower density, hence lower crystallinity, than HDPE; e.g., 10% versus 50%. This has the effect of allowing oxygen to diffuse into the polymer structure quicker, and likely decreases Stages A and C. How much is uncertain since no data is available, but it is felt that the lifetime of LLDPE will be somewhat reduced with respect to HDPE." On page 23, Table 6, it listed the lifetime prediction of various types of geomembrane, including 1.5mm HDPE and 1.0mm LLDPE.

According to Geosynthetics from David I. Cook in Table 6, page 17, the chemical resistance of HDPE is rated "Excellent" and LLDPE is rated "Good". We also attached part of the report, p.15 to 18, for your information.

Please feel free to contact me if you have any question.

Sincerely,

A handwritten signature in black ink, appearing to read "Connie Wong", with a stylized flourish extending from the end.

Connie Wong, Ph.D.,
Support Engineer
cowong@gseworld.com

These EN requirement standards instruct manufacturers to declare the mean value and tolerance of certain product characteristics. The required characteristics depend on the end use and function (separation, filtration, etc.) of the fabric. These characteristics are determined by some of the test methods listed in **Table 3**. Some tests (referred to as H tests in the standards) are mandatory, i.e., required by Mandate M/107 issued by the European Community to the European standardisation committee for geotextiles, CEN 189. Some (referred to as A tests in the standards) are relevant to all conditions of use. The A tests are not legally required but are commercially important. A third test category (S) is relevant to specific conditions of use.

For more information and detail on the CPD and CE marking of geotextiles, the reader is recommended to consult a relevant and useful web site on quality and standardisation of geotextiles (www.vinci-quality.com).

It should be noted that ‘geotextile related products’ includes all other categories of geosynthetic except geomembranes and GCLs. The latter two groups are now known as ‘geosynthetic barriers’ and are in a separate CPD sector from geotextiles and related products.

The application standards for geosynthetic barriers are, at the time of writing, still at the provisional stage (prENs). There are five provisional geosynthetic barrier standards (www.vinci-quality.com):

- prEN 13361 reservoir and dam construction
- prEN 13362 canal construction
- prEN 13491 fluid barrier in tunnel and underground structures
- prEN 13492 liquid waste containment
- prEN 13493 solid and hazardous waste storage

4 Geomembranes

4.1 Description and Manufacturing

Geomembranes are thin, flexible sheets of material with very low permeability. They are manufactured from synthetic or bituminous products and may be

strengthened with a fabric or film. Geomembranes are employed invariably as a barrier to prevent the passage of gases and fluids.

A commonly accepted definition of a geomembrane from the ASTM is:

A geomembrane is a continuous membrane liner or barrier having sufficiently low permeability to control migration of fluids in a constructed project, structure or dam.

The original geomembranes were made of butyl rubber. Nowadays, polyvinyl chloride (PVC) or some form of polyethylene account for a high proportion of the total geomembrane useage. The different types of geomembrane polymer will be discussed in more detail later (Section 4.2).

The manufacture of geomembranes commences with the production of the raw materials, i.e., the polymer resin and a wide range of additives such as stabilisers, plasticisers, softeners, fillers, processing aids etc.

The raw materials can then be processed in three ways:

The first method produces the simplest type of geomembrane; single ply and non-reinforced. In this method the raw materials are blended and compounded before being extruded as sheets or cylinders. The extruder produces sheet material 0.1 to 5 mm thick and 1 to 5 m wide. Calendering, or pressing the sheets between counter rotating rollers, works them into uniform thickness and improves the mechanical properties. Full thickness sheets can also be blown into a large long bubble which is cut and opened into the final sheet form.

In the second method, multi-ply geomembranes are made by laminating several layers together. Laminated geomembranes can be non-reinforced or reinforced by inclusion of a fabric scrim between the layers. The scrim improves the mechanical properties (e.g., tensile and tear strength) of the composite material but does not reinforce the soil on which the geomembrane is placed.

The third production method is known as spread coating. In this method, a geotextile, usually a needle-punched non-woven, is used as a substrate on which the molten polymer is spread into its final thickness.

Geomembrane manufacturing methods are described in more detail in (a.9).

The surface of some geomembranes is roughened or textured, by spraying or embossing, to increase the soil/polymer friction (a.19). Three common techniques are coextrusion, lamination and impingement (a.20):

- Coextrusion uses a blowing agent in the extrudate which expands on cooling to cause a roughened surface.
- In lamination, a sheet containing a foam is adhered to a conventional sheet. The foaming agent provides a froth that produces a rough, textured laminate stuck to the smooth, solid sheet.
- Impingement is the projection of hot particles on to the smooth sheet.

A review of these texturing techniques and a discussion on characterisation of the surface topography has been presented by Zettler and co-workers (7).

4.2 Polymers

The vast majority of geomembranes are thin sheets of flexible thermoplastic or thermosetting polymeric materials. The main polymers used are (a.9):

Thermoplastic polymers

- Polyethylene (PE) – high density (HDPE), medium density (MDPE), linear low density (LLDPE), low density (LDPE), very low density (VLDPE)
- Polypropylene (PP)
- Polyvinyl chloride (PVC)
- Chlorinated polyethylene (CPE)
- Polyamide (PA)

Thermoset polymers

- Isoprene-isobutylene (IIR) or butyl rubber
- Epichlorohydrin rubber
- Ethylene-propylene-diene terpolymer (EDPM)
- Polychloroprene (Neoprene)
- Ethylene-propylene terpolymer (EPT)

- Ethylene-vinyl acetate (EVA)
- Ethylene interpolymer alloy (EIA)

Combination polymers

- PVC-nitrile rubber
- PE-EPDM
- PVC-EVA
- Crosslinked CPE
- Chlorosulfonated polyethylene (CSPE) ‘Hypalon’

Others

- Bitumen impregnated geotextiles
- Elastomer impregnated geotextiles
- Aluminium foil coated geomembranes for protection against hydrocarbons (329).

Which geomembrane polymer to choose depends, as usual, on its properties, availability and, of course, on the application and structure in which the geomembrane is to be incorporated. The properties will be discussed in more detail in Section 4.4 (Testing).

No study appears to have been performed which compares the properties of all these polymers as geomembrane sheets. However, **Table 6** (a.21) provides a useful qualitative comparison of geomembrane liners made from eight polymers and, another type of geosynthetic barrier known as a geosynthetic clay liner (GCL). The attributes considered are physical properties, durability, installation damage, seaming and cost.

Table 6 refers to seaming methods, that is to say the techniques used to join together the membrane sheets either at the factory or on site. Production of a leak tight and durable seam is clearly vital otherwise the geomembrane’s function as a fluid barrier is lost. There are several general seaming methods. The choice is governed by the polymer and site requirements.

- Solvent: A liquid solvent is brushed between two sheets of membrane followed by pressure to form the seal.
- Adhesive: A glue dissolved in a solvent is applied by brush or roller to the membrane sheets which are brought together after the surface becomes tacky.

Table 6 Geomembranes; comparison of polymers

Attribute	HDPE	LLDPE	PVC	EPDM	EIA	Reinforced CSPE	Flexible PP	GCL
Chemical resistance	Excellent	Good	Fair	Good	Excellent (when cured)	Excellent	Excellent	Fair
Hydrocarbon resistance	Good	Good	Fair	Good	Excellent (when cured)	Good	Good	Poor
Weathering; UV resistance	Excellent	Fair	Poor	Excellent	Excellent (when cured)	Excellent	Excellent	Poor
Thermal stability	Poor	Poor	Good	Excellent	Good	Excellent	Good – excellent when reinforced	Good
Tensile properties	Good	Good	Good	Good	Excellent	Excellent	Good – excellent when reinforced	Good
Elongation; uniaxial	Excellent	Excellent	Good	Good	Fair	Good	Excellent	Fair
Elongation; multiaxial	Poor	Excellent	Excellent	Good	Fair	Good	Excellent	Fair
Puncture resistance	Fair	Excellent	Excellent	Good	Excellent	Good	Good	Good
Stress cracking	Fair	Good	Does not occur	Does not occur	Does not occur	Does not occur	Does not occur	Does not occur
Resistance to installation damage	Fair	Fair	Excellent	Excellent	Good	Good	Excellent	Good
Seaming methods	Thermal/ excellent	Thermal/ excellent	Thermal or solvent bonding/ good	Tape seams/good	Thermal/ excellent	Thermal or solvent bonding good	Thermal/ excellent	Laps only
Ease of repair in service	Good	Good	Good	Good	Good	Poor needs adhesives	Excellent	NA
Flexibility	Fair	Excellent	Good	Good	Good	Good	Excellent	NA
Roll cost	Low	Low/ medium	medium	Medium/ high	High	High	Medium	Medium

Reproduced with permission from M. Sadlier (a.21)

- Thermal: There is a range of thermal methods in which the opposing surfaces are melted by some means such as hot air or electrically heated knife or wedge. The melted surfaces are pressed together by rollers.

- Mechanical: The usual mechanical joining methods are utilisation of sticky tapes or sewing. The sewn seams are subsequently waterproofed.

The solvent and adhesive methods tend to be used for thermoplastic polymers and elastomers such as PVC,

CPE, EDPM, and CSPE. Thermal methods can be employed for most thermoplastic polymers but are especially important for semi-crystalline polyethylene materials like HDPE and LDPE. A more detailed description of geomembrane seaming is given in Chapter 5 of Designing with Geosynthetics by Koerner (a.9).

Some further advantages and disadvantages of specific polymers are now given.

PVC (polyvinyl chloride)

- Tough without reinforcement
- Good seams by dielectric, solvent or heat
- Elasticised for flexibility, plasticiser leaches with time

CPE (chlorinated polyethylene)

- Seams easy to make by dielectric or solvent
- Plasticised with PVC

CSPE (chlorosulfonated polyethylene thermoplastic rubber)

- Good seams by heat or adhesive

Butyl

- Poor field seams

- Low gas permeability

CP (chloroprene/neoprene cured rubber)

- Fair field seams using solvent and tape

HDPE

- Good seams by thermal or extrusion methods
- Low friction surfaces
- High thermal expansion and contraction

MDPE, LDPE and VLDPE

- Good seams by thermal or extrusion methods
- Moderate thermal expansion and contraction

LLDPE

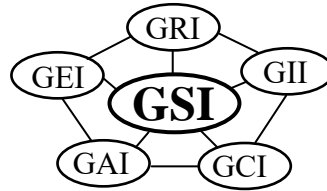
- High friction surface
- Good seams by thermal or extrusion methods
- Large variation in thickness

Further comparative properties are given in **Table 7** which has been generated from information in references a.9 and a.22.

Table 7 Geomembrane polymers: advantages and disadvantages								
Factor	PVC	CPE	CSPE	Butyl	CP	HDPE	MDPE LDPE VLDPE	LLDPE
Cost	low	medium	medium	medium to high	high	low	low	medium
Chemical resistance	-	good	good	-	good	excellent	good	very good
Weathering	poor	good	excellent	fair to good	good	-	-	-
High temperature	poor	-	fair	good	good	-	-	-
Cold crack	poor	good	good	-	-	-	-	-
Stress crack	-	-	-	-	-	sensitive	none	none
<i>CP = chloroprene rubber</i>								

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GRI White Paper #6

- on -

Geomembrane Lifetime Prediction: Unexposed and Exposed Conditions

by

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Original: June 7, 2005

Updated: February 8, 2011

Geomembrane Lifetime Prediction: Unexposed and Exposed Conditions

1.0 Introduction

Without any hesitation the most frequently asked question we have had over the past thirty years' is "how long will a particular geomembrane last".* The two-part answer to the question, largely depends on whether the geomembrane is covered in a timely manner or left exposed to the site-specific environment. Before starting, however, recognize that the answer to either covered or exposed geomembrane lifetime prediction is neither easy, nor quick, to obtain. Further complicating the answer is the fact that all geomembranes are formulated materials consisting of (at the minimum), (i) the resin from which the name derives, (ii) carbon black or colorants, (iii) short-term processing stabilizers, and (iv) long-term antioxidants. If the formulation changes (particularly the additives), the predicted lifetime will also change. See Table 1 for the most common types of geomembranes and their approximate formulations.

Table 1 - Types of commonly used geomembranes and their approximate formulations
(based on weight percentage)

Type	Resin	Plasticizer	Fillers	Carbon Black	Additives
HDPE	95-98	0	0	2-3	0.25-1
LLDPE	94-96	0	0	2-3	0.25-3
fPP	85-98	0	0-13	2-4	0.25-2
PVC	50-70	25-35	0-10	2-5	2-5
CSPE	40-60	0	40-50	5-10	5-15
EPDM	25-30	0	20-40	20-40	1-5

HDPE = high density polyethylene PVC = polyvinyl chloride (plasticized)
LLDPE = linear low density polyethylene CSPE = chlorsulfonated polyethylene
fPP = flexible polypropylene EPDM = ethylene propylene diene terpolymer

* More recently, the same question has arisen but focused on geotextiles, geogrids, geopipe, turf reinforcement mats, fibers of GCLs, etc. This White Paper, however, is focused completely on geomembranes due to the tremendous time and expense of providing such information for all types of geosynthetics.

The possible variations being obvious, one must also address the degradation mechanisms which might occur. They are as follows accompanied by some generalized commentary.

- Ultraviolet Light - This occurs only when the geosynthetic is exposed; it will be the focus of the second part of this communication.
- Oxidation - This occurs in all polymers and is the major mechanism in polyolefins (polyethylene and polypropylene) under all conditions.
- Ozone - This occurs in all polymers that are exposed to the environment. The site-specific environment is critical in this regard.
- Hydrolysis - This is the primary mechanism in polyesters and polyamides.
- Chemical - Can occur in all polymers and can vary from water (least aggressive) to organic solvents (most aggressive).
- Radioactivity - This is not a factor unless the geomembrane is exposed to radioactive materials of sufficiently high intensity to cause chain scission, e.g., high level radioactive waste materials.
- Biological - This is generally not a factor unless biologically sensitive additives (such as low molecular weight plasticizers) are included in the formulation.
- Stress State – This is a complicating factor which is site-specific and should be appropriately modeled in the incubation process but, for long-term testing, is very difficult and expensive to achieve.
- Temperature - Clearly, the higher the temperature the more rapid the degradation of all of the above mechanisms; temperature is critical to lifetime and furthermore is the key to

time-temperature-superposition which is the basis of the laboratory incubation methods which will be followed.

2.0 Lifetime Prediction: Unexposed Conditions

Lifetime prediction studies at GRI began at Drexel University under U. S. EPA contract from 1991 to 1997 and was continued under GSI consortium funding until ca. 2002. Focus to date has been on HDPE geomembranes placed beneath solid waste landfills due to its common use in this particular challenging application. Incubation of the coupons has been in landfill simulation cells (see Figure 1) maintained at 85, 75, 65 and 55°C. The specific conditions within these cells are oxidation beneath, chemical (water) from above, and the equivalent of 50 m of solid waste mobilizing compressive stress. Results have been forthcoming over the years insofar as three distinct lifetime stages; see Figure 2.

Stage A - Antioxidant Depletion Time

Stage B - Induction Time to the Onset of Degradation

Stage C - Time to Reach 50% Degradation (i.e., the Halflife)

2.1 Stage A - Antioxidant Depletion Time

The dual purposes of antioxidants are to (i) prevent polymer degradation during processing, and (ii) prevent oxidation reactions from taking place during Stage A of service life, respectively. Obviously, there can only be a given amount of antioxidants in any formulation. Once the antioxidants are depleted, additional oxygen diffusing into the geomembrane will begin to attack the polymer chains, leading to subsequent stages as shown in Figure 2. The duration of the antioxidant depletion stage depends on both the type and amount of the various antioxidants, i.e., the precise formulation.

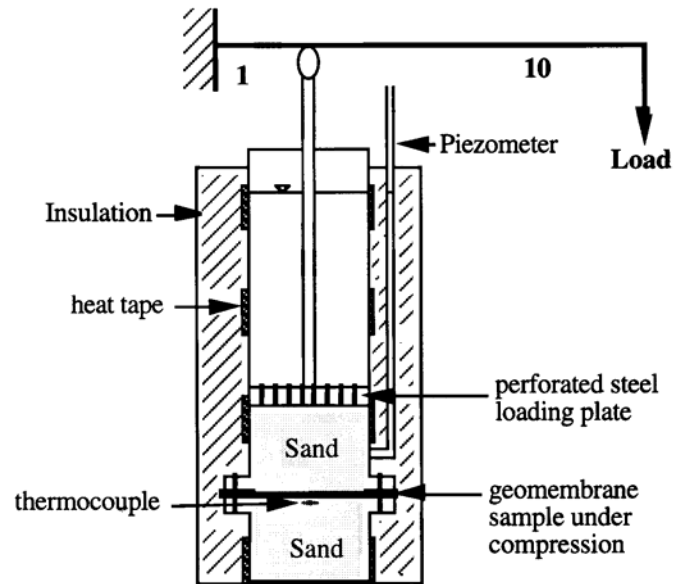


Figure 1. Incubation schematic and photograph of multiple cells maintained at various constant temperatures.

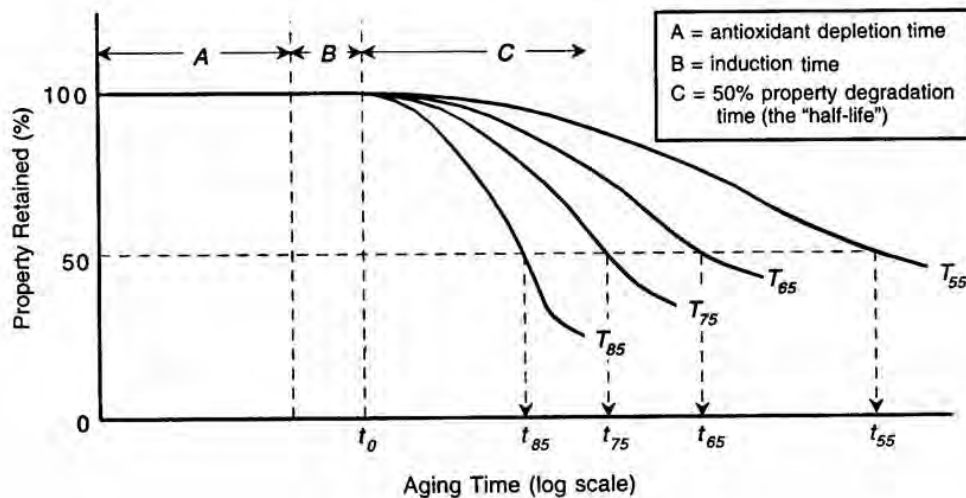


Figure 2. Three individual stages in the aging of most geomembranes.

The depletion of antioxidants is the consequence of two processes: (i) chemical reactions with the oxygen diffusing into the geomembrane, and (ii) physical loss of antioxidants from the geomembrane. The chemical process involves two main functions; the scavenging of free radicals converting them into stable molecules, and the reaction with unstable hydroperoxide (ROOH) forming a more stable substance. Regarding physical loss, the process involves the distribution of antioxidants in the geomembrane and their volatility and extractability to the site-specific environment.

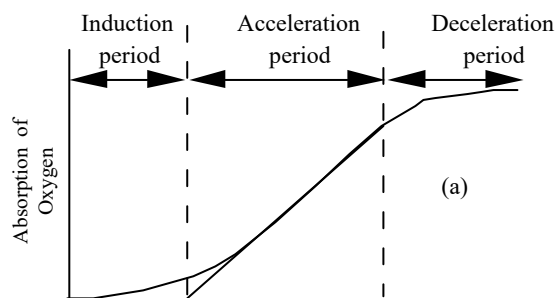
Hence, the rate of depletion of antioxidants is related to the type and amount of antioxidants, the service temperature, and the nature of the site-specific environment. See Hsuan and Koerner (1998) for additional details.

2.2 Stage B - Induction Time to Onset of Degradation

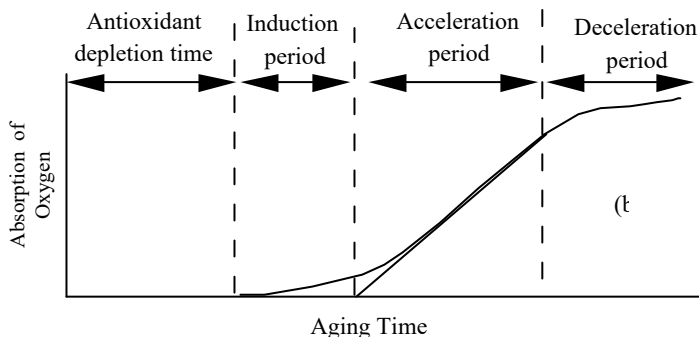
In a pure polyolefin resin, i.e., one without carbon black and antioxidants, oxidation occurs extremely slowly at the beginning, often at an immeasurable rate. Eventually, oxidation occurs more rapidly. The reaction eventually decelerates and once again becomes very slow.

This progression is illustrated by the S-shaped curve of Figure 3(a). The initial portion of the curve (before measurable degradation takes place) is called the induction period (or induction time) of the polymer. In the induction period, the polymer reacts with oxygen forming hydroperoxide (ROOH), as indicated in Equations (1)-(3). However, the amount of ROOH in this stage is very small and the hydroperoxide does not further decompose into other free radicals which inhibits the onset of the acceleration stage.

In a stabilized polymer such as one with antioxidants, the accelerated oxidation stage takes an even longer time to be reached. The antioxidants create an additional depletion time stage prior to the onset of the induction time, as shown in Figure 3(b).



(a) Pure unstabilized polyethylene



(b) Stabilized polyethylene

Figure 3. Curves illustrating various stages of oxidation.



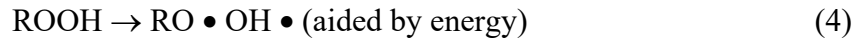
(aided by energy or catalyst residues in the polymer)



In the above, RH represents the polyethylene polymer chains; and the symbol “•” represents free radicals, which are highly reactive molecules.

2.3 Stage C - Time to Reach 50% Degradation (Halflife)

As oxidation continues, additional ROOH molecules are being formed. Once the concentration of ROOH reaches a critical level, decomposition of ROOH begins, leading to a substantial increase in the amount of free radicals, as indicated in Equations (4) to (6). The additional free radicals rapidly attack other polymer chains, resulting in an accelerated chain reaction, signifying the end of the induction period, Rapoport and Zaikov (1986). This indicates that the concentration of ROOH has a critical control on the duration of the induction period.



A series of oxidation reactions produces a substantial amount of free radical polymer chains ($R\bullet$), called alkyl radicals, which can proceed to further reactions leading to either cross-linking or chain scission in the polymer. As the degradation of polymer continues, the physical and mechanical properties of the polymer start to change. The most noticeable change in physical properties is the melt index, since it relates to the molecular weight of the polymer. As for mechanical properties, both tensile break stress (strength) and break strain (elongation) decrease.

Ultimately, the degradation becomes so severe that all tensile properties start to change (tear, puncture, burst, etc.) and the engineering performance is jeopardized. This signifies the end of the so-called “service life” of the geomembrane.

Although quite arbitrary, the limit of service life of polymeric materials is often selected as a 50% reduction in a specific design property. This is commonly referred to as the halflife time, or simply the “halflife”. It should be noted that even at halflife, the material still exists and can function, albeit at a decreased performance level with a factor-of-safety lower than the initial design value.

2.4 Summary of Lifetime Research-to-Date

Stage A, that of antioxidant depletion for HDPE geomembranes as required in the GRI-GM13 Specification, has been well established by our own research and corroborated by others, e.g., Sangram and Rowe (2004). The GRI data for standard and high pressure Oxidative Induction Time (OIT) is given in Table 2. The values are quite close to one another. Also, as expected, the lifetime is strongly dependent on the service temperature; with the higher the temperature the shorter the lifetime.

Table 2 - Lifetime prediction of HDPE (nonexposed) at various field temperatures

In Service Temperature (°C)	Stage “A” (years)			Stage “B” (years)	Stage “C” (years)	Total Prediction* (years)
	Standard OIT	High Press. OIT	Average OIT			
20	200	215	208	30	208	446
25	135	144	140	25	100	265
30	95	98	97	20	49	166
35	65	67	66	15	25	106
40	45	47	46	10	13	69

*Total = Stage A (average) + Stage B + Stage C

Stage “B”, that of induction time, has been obtained by comparing 30-year old polyethylene water and milk containers (containing no long-term antioxidants) with currently

produced containers. The data shows that degradation is just beginning to occur as evidenced by slight changes in break strength and elongation, but not in yield strength and elongation. The lifetime for this stage is also given in Table 2.

Stage “C”, the time for 50% change of mechanical properties is given in Table 2 as well. The data depends on the activation energy, or slope of the Arrhenius curve, which is very sensitive to material and experimental techniques. The data is from Gedde, et al. (1994) which is typical of the HDPE resin used for gas pipelines and is similar to Martin and Gardner (1983).

Summarizing Stages A, B, and C, it is seen in Table 2 that the half-life of covered HDPE geomembranes (formulated according to the current GRI-GM13 Specification) is estimated to be 449-years at 20°C. This, of course, brings into question the actual temperature for a covered geomembrane such as beneath a solid waste landfill. Figure 4 presents multiple thermocouple monitoring data of a municipal waste landfill liner in Pennsylvania for over 10-years, Koerner and Koerner (2005). Note that for 6-years the temperature was approximately 20°C. At that time and for the subsequent 4-years the temperature increased to approximately 30°C. Thus, the half-life of this geomembrane is predicted to be from 166 to 446 years within this temperature range. The site is still being monitored, see Koerner and Koerner (2005).

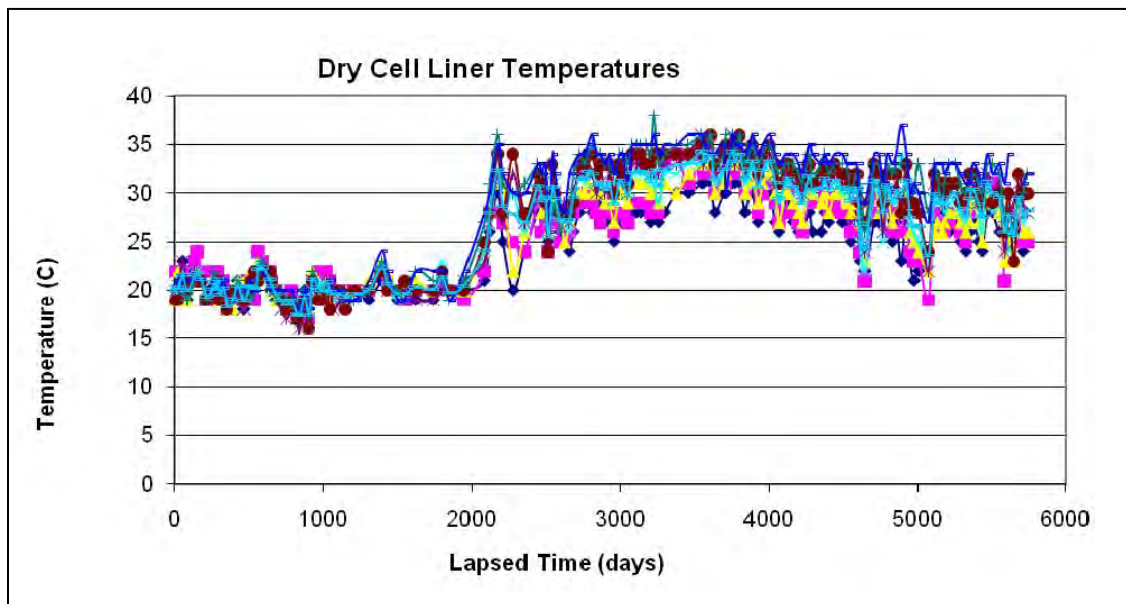


Figure 4. Long-term monitoring of an HDPE liner beneath a municipal solid waste landfill in Pennsylvania.

2.5 Lifetime of Other Covered Geomembranes

By virtue of its widespread use as liners for solid waste landfills, HDPE is by far the widest studied type of geomembrane. Note that in most countries (other than the U.S.), HDPE is the required geomembrane type for solid waste containment. Some commentary on other-than HDPE geomembranes (recall Table 1) follows:

2.5.1 Linear Low Density Polyethylene (LLDPE) geomembranes

The nature of the LLDPE resin and its formulation is very similar to HDPE. The fundamental difference is that LLDPE is a lower density, hence lower crystallinity, than HDPE; e.g., 10% versus 50%. This has the effect of allowing oxygen to diffuse into the polymer structure quicker, and likely decreases Stages A and C. How much is uncertain since no data is available, but it is felt that the lifetime of LLDPE will be somewhat reduced with respect to HDPE.

2.5.2 Plasticizer migration in PVC geomembranes

Since PVC geomembranes necessarily have plasticizers in their formulations so as to provide flexibility, the migration behavior must be addressed for this material. In PVC the plasticizer bonds to the resin and the strength of this bonding versus liquid-to-resin bonding is significant. One of the key parameters of a stable long-lasting plasticizer is its molecular weight. The higher the molecular weight of the plasticizer in a PVC formulation, the more durable will be the material. Conversely, low molecular weight plasticizers have resulted in field failures even under covered conditions. See Miller, et al. (1991), Hammon, et al. (1993), and Giroud and Tisinger (1994) for more detail in this regard. At present there is a considerable difference (and cost) between PVC geomembranes made in North America versus Europe. This will be apparent in the exposed study of durability in the second part of this White Paper.

2.5.3 Crosslinking in EPDM and CSPE geomembranes

The EPDM geomembranes mentioned in Table 1 are crosslinked thermoset materials. The oxidation degradation of EPDM takes place in either ethylene or propylene fraction of the co-polymer via free radical reactions, as expressed in Figure 5, which are described similarly by Equations (4) to (6).

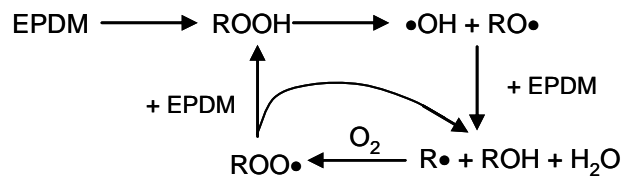


Figure 5. Oxidative degradation of crosslinked EPDM geomembranes, (Wang and Qu, 2003).

For CSPE geomembranes, the degradation mechanism is dehydrochlorination by losing chlorine and generating carbon-carbon double bonds in the main polymer chain, as shown in Figure 6.

The carbon-carbon double bonds become the preferred sites for further thermodegradation or cross-linking in the polymer, leading to eventual brittleness of the geomembrane.

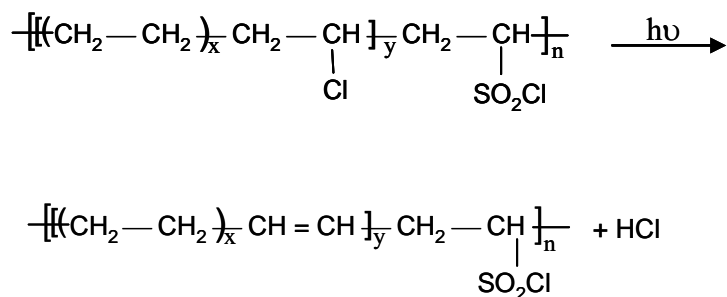


Figure 6. Dechlorination degradation of crosslinked CSPE geomembranes (Chailan, et al., 1995).

Neither EPDM nor CSPE has had a focused laboratory study of the type described for HDPE reported in the open literature. Most of lifetime data for these geomembranes is antidotal by virtue of actual field performance. Under covered conditions, as being considered in this section, there have been no reported failures by either of these thermoset polymers to our knowledge.

3.0 Lifetime Prediction: Exposed Conditions

Lifetime prediction of exposed geomembranes have taken two very different pathways; (i) prediction from anecdotal feedback and field performance, and (ii) from laboratory weathering device predictions.

3.1 Field Performance

There is a large body of anecdotal information available on field feedback of exposed geomembranes. It comes from two quite different sources, i.e., dams in Europe and flat roofs in the USA.

Regarding exposed geomembranes in dams in Europe, the original trials were using 2.0 mm thick polyisobutylene bonded directly to the face of the dam. There were numerous problems encountered as described by Scuero (1990). Similar experiences followed using PVC

geomembranes. In 1980, a geocomposite was first used at Lago Nero which had a 200 g/m² nonwoven geotextile bonded to the PVC geomembrane. This proved quite successful and led to the now-accepted strategy of requiring drainage behind the geomembrane. In addition to thick nonwoven geotextiles, geonets, and geonet composites have been successful. Currently over 50 concrete and masonry dams have been rehabilitated in this manner and are proving successful for over 30-years of service life. The particular type of PVC plasticized geomembranes used for these dams is proving to be quite durable. Tests by the dam owners on residual properties show only nominal changes in properties, Cazzuffi (1998). As indicated in Miller, et al. (1991) and Hammond, et al. (1993), however, different PVC materials and formulations result in very different behavior; the choice of plasticizer and the material's thickness both being of paramount importance. An excellent overview of field performance is recently available in which 250 dams which have been waterproofed by geomembranes is available from ICOLD (2010).

Regarding exposed geomembranes in flat roofs, past practice in the USA is almost all with EPDM and CSPE and, more recently, with fPP. Manufacturers of these geomembranes regularly warranty their products for 20-years and such warrants appear to be justified. EPDM and CSPE, being thermoset or elastomeric polymers, can be used in dams without the necessity of having seams by using vertical attachments spaced at 2 to 4 m centers, see Scuero and Vaschetti (1996). Conversely, fPP can be seamed by a number of thermal fusion methods. All of these geomembrane types have good conformability to rough substrates as is typical of concrete and masonry dam rehabilitation. It appears as though experiences (both positive and negative) with geomembranes in flat roofs should be transferred to all types of waterproofing in civil engineering applications.

3.2 Laboratory Weatherometer Predictions

For an accelerated simulation of direct ultraviolet light, high temperature, and moisture using a laboratory weatherometer one usually considers a worst-case situation which is the solar maximum condition. This condition consists of global, noon sunlight, on the summer solstice, at normal incidence. It should be recognized that the UV-A range is the target spectrum for a laboratory device to simulate the naturally occurring phenomenon, see Hsuan and Koerner (1993), and Suits and Hsuan (2001).

The Xenon Arc weathering device (ASTM D4355) was introduced in Germany in 1954. There are two important features; the type of filters and the irradiance settings. Using a quartz inner and borosilicate outer filter (quartz/boro) results in excessive low frequency wavelength degradation. The more common borosilicate inner and outer filters (boro/boro) shows a good correlation with solar maximum conditions, although there is an excess of energy below 300 nm wavelength. Irradiance settings are important adjustments in shifting the response although they do not eliminate the portion of the spectrum below 300 nm frequency. Nevertheless, the Xenon Arc device is commonly used method for exposed lifetime prediction of all types of geosynthetics.

UV Fluorescent devices (ASTM D7238) are an alternative type of accelerated laboratory test device which became available in the early 1970's. They reproduce the ultraviolet portion of the sunlight spectrum but not the full spectrum as in Xenon Arc weatherometers. Earlier FS-40 and UVB-313 lamps give reasonable short wavelength output in comparison to solar maximum. The UVA-340 lamp was introduced in 1987 and its response is seen to reproduce ultraviolet light quite well. This device (as well as other types of weatherometers) can handle elevated temperature and programmed moisture on the test specimens.

Research at the Geosynthetic Institute (GSI) has actively pursued both Xenon and UV Fluorescent devices on a wide range of geomembranes. Table 3 gives the geomembranes that were incubated and the number of hours of exposure as of 12 July 2005.

Table 5 - Details of the GSI laboratory exposed weatherometer study on various types of geomembranes

Geomembrane Type	Thickness (mm)	UV Fluorescent Exposure*	Xenon Exposure*	Comment
1. HDPE (GM13)	1.50	8000 hrs.	6600 hrs.	Basis of GRI-GM13 Spec
2. LLDPE (GM17)	1.00	8000	6600	Basis of GRI-GM-17 Spec
3. PVC (No. Amer.)	0.75	8000	6600	Low Mol. Wt. Plasticizer
4. PVC (Europe)	2.50	7500	6600	High Mol. Wt. Plasticizer
5. fPP (BuRec)	1.00	2745**	4416**	Field Failure at 26 mos.
6. fPP-R (Texas)	0.91	100	100	Field Failure at 8 years
7. fPP (No. Amer.)	1.00	7500	6600	Expected Good Performance

*As of 12 July 2005 exposure is ongoing

**Light time to reach halflife of break and elongation

3.3 Laboratory Weatherometer Acceleration Factors

The key to validation of any laboratory study is to correlate results to actual field performance. For the nonexposed geomembranes of Section 2 such correlations will take hundreds of years for properly formulated products. For the exposed geomembranes of Section 3, however, the lifetimes are significantly shorter and such correlations are possible. In particular, Geomembrane #5 (flexible polypropylene) of Table 3 was an admittedly poor geomembrane formulation which failed in 26 months of exposure at El Paso, Texas, USA. The reporting of this failure is available in the literature, Comer, et al. (1998). Note that for both UV Fluorescent and Xenon Arc laboratory incubation of this material, failure (halflife to 50% reduction in strength and elongation) occurred at 2745 and 4416 hours, respectively. The comparative analysis of laboratory and field for this case history allows for the obtaining of acceleration factors for the two incubation devices.

3.3.1 Comparison between field and UV Fluorescent weathering

The light source used in the UV fluorescent weathering device is UVA with wavelengths from 295-400 nm. In addition, the intensity of the radiation is controlled by the Solar Eye irradiance control system. The UV energy output throughout the test is 68.25 W/m².

The time of exposure to reach 50% elongation at break was as follows:

$$\begin{aligned} &= 2745 \text{ hr. of light} \\ &= 9,882,000 \text{ seconds} \end{aligned}$$

$$\begin{aligned} \text{Total energy in MJ/m}^2 &= 68.25 \text{ W/m}^2 \times 9,882,000 \\ &= 674.4 \text{ MJ/m}^2 \end{aligned}$$

The field site was located at El Paso, Texas. The UVA radiation energy (295-400 nm) at this site is estimated based on data collected by the South Florida Testing Lab in Arizona (which is a similar atmospheric location). For 26 months of exposure, the accumulated UV radiation energy is 724 MJ/m² which is very close to that generated from the UV fluorescent weatherometer. Therefore, direct comparison of the exposure time between field and UV fluorescent is acceptable.

Field time	vs.	Fluorescent UV light time:	Thus, the acceleration factor is 6.8.
= 26 Months		= 3.8 Months	

3.3.2 Comparison between field and Xenon Arc weathering

The light source of the Xenon Arc weathering device simulates almost the entire sunlight spectrum from 250 to 800 nm. Depending of the age of the light source and filter, the solar energy ranges from 340.2 to 695.4 W/m², with the average value being 517.8 W/m².

The time of exposure to reach 50% elongation at break

$$\begin{aligned} &= 4416 \text{ hr. of light} \\ &= 15,897,600 \text{ seconds} \end{aligned}$$

$$\begin{aligned} \text{Total energy in MJ/m}^2 &= 517.8 \text{ W/m}^2 \times 15,897,600 \\ &= 8232 \text{ MJ/m}^2 \end{aligned}$$

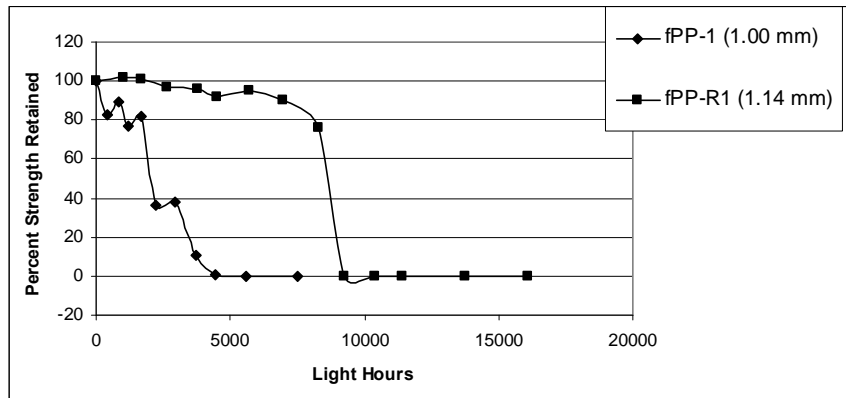
The solar energy in the field is again estimated based on data collected by the South Florida Testing Lab in Arizona. For 26 months of exposure, the accumulated solar energy (295-800 nm) is $15,800 \text{ MJ/m}^2$, which is much higher than that from the UV Fluorescent device. Therefore, direct comparison of half-lives obtained from the field and Xenon Arc device is not anticipated to be very accurate. However, for illustration purposes the acceleration factor based on Xenon Arc device would be as follows:

Field	vs.	Xenon Arc	:	Thus, the acceleration factor is 4.3.
= 26 Months		= 6.1 Months		

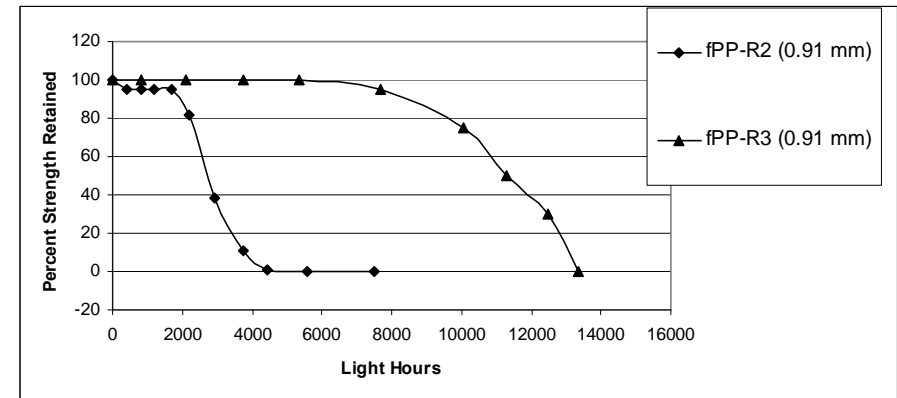
The resulting conclusion of this comparison of weathering devices is that the UV Fluorescent device is certainly reasonable to use for long-term incubations. When considering the low cost of the device, its low maintenance, its inexpensive bulbs, and ease of repair it (the UV Fluorescent device) will be used exclusively by GSI for long-term incubation studies.

3.3.3 Update of exposed lifetime predictions

There are presently (2011) four field failures of flexible polypropylene geomembranes and using unexposed archived samples from these sites their responses in laboratory UV Fluorescent devices per ASTM D7328 at 70°C are shown in Figure 5. From this information we deduce that the average correlation factor is approximately *1200 light hours \simeq one-year in a hot climate.* This value will be used accordingly for other geomembranes.



(a) Two Sites in West Texas



(b) Two Sites in So. Calif.

Lab-to-Field Correlation Factors
(ASTM D7238 @ 70°C)

Method	Thickness (mm)	Field (yrs.)	Location	Lab (lt. hr.)	Factor (lt. hrs./1.0 yr.)
fPP-1	1.00	≈ 2	W. Texas	1800	900
fPP-R1	1.14	≈ 8	W. Texas	8200	1025
fPP-R2	0.91	≈ 2	So. Calif.	2500	1250
fPP-R3	0.91	≈ 8	So. Calif.	11200	<u>1400</u>
					1140*

*Use 1200 lt. hr. = 1.0 year in hot climates

Figure 5. Four field failures of fPP and fPP-R exposed geomembranes.

Exposure of a number of different types of geomembranes in laboratory UV Fluorescent devices per ASTM D7238 at 70°C has been ongoing for the six years (between 2005 and 2011) since this White Paper was first released. Included are the following geomembranes:

- Two black 1.0 mm (4.0 mil) unreinforced flexible polypropylene geomembranes formulated per GRI-GM18 Specification; see Figure 6a.
- Two black unreinforced polyethylene geomembranes, one 1.5 mm (60 mil) high density per GRI-GM13 Specification and the other 1.0 mm (40 mil) linear low density per GRI-GM17 Specification; see Figure 6b.
- One 1.0 (40 mil) black ethylene polypropylene diene terpolymer geomembrane per GRI-GM21 Specification; see Figure 6c.
- Two polyvinyl chloride geomembranes, one black 1.0 mm (40 mil) formulated in North America and the other grey 1.5 mm (60 mil) formulated in Europe; see Figure 6d.

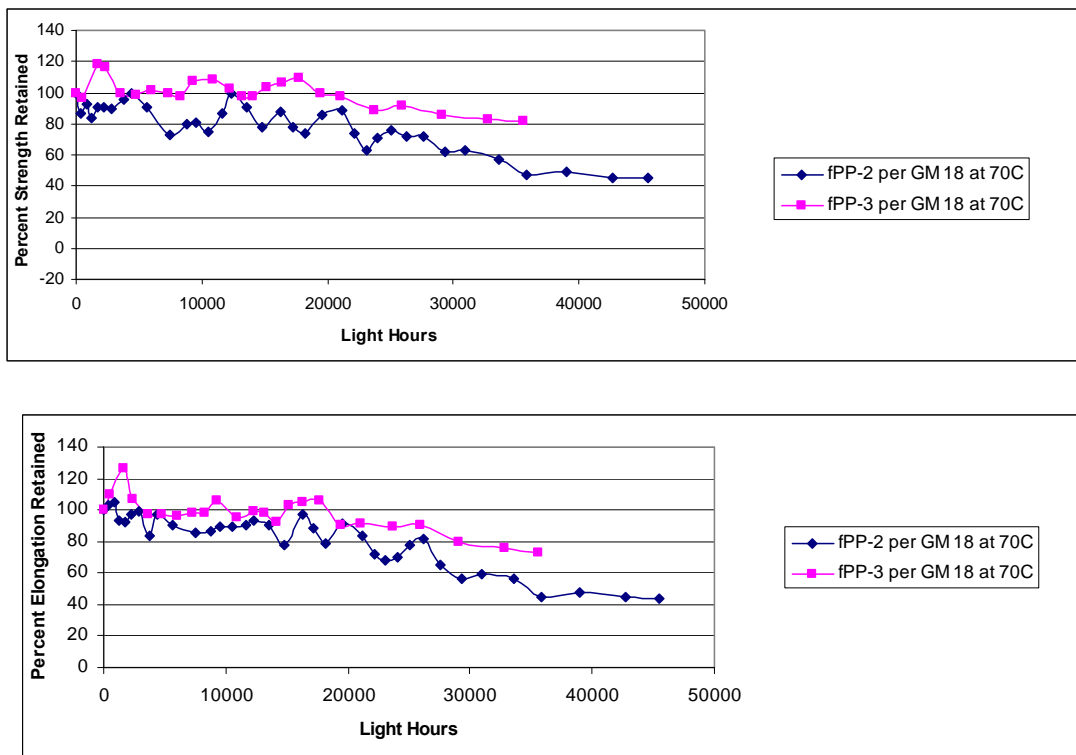


Figure 6a. Flexible polyethylene (fPP) geomembrane behavior.

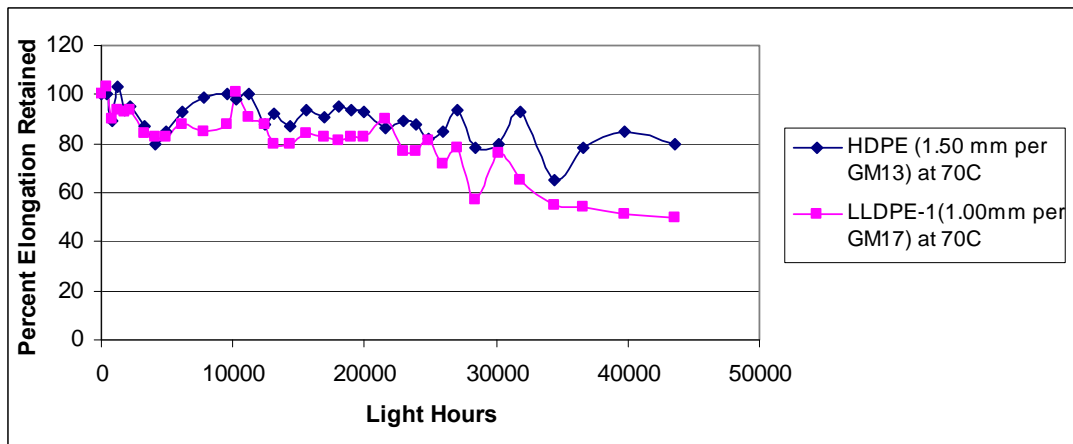
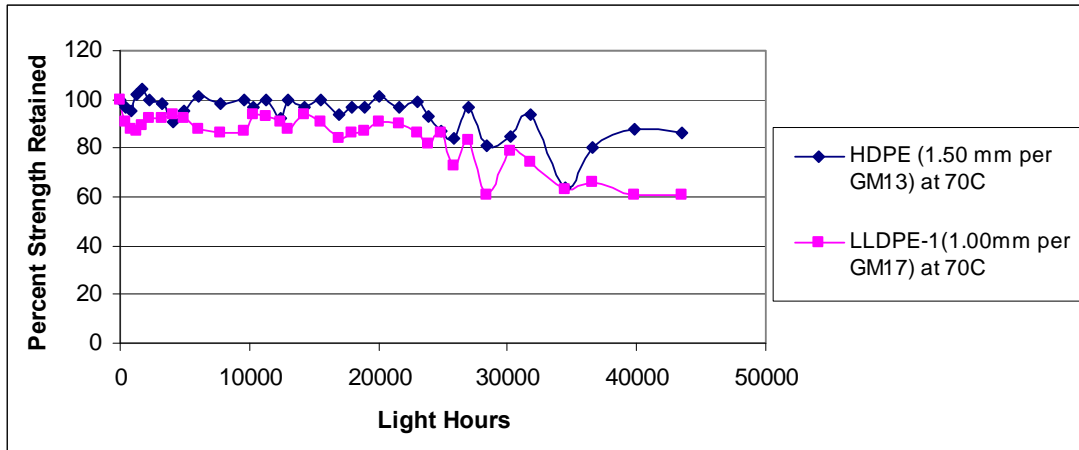


Figure 6b. Polyethylene (HDPE and LLDPE) geomembrane behavior.

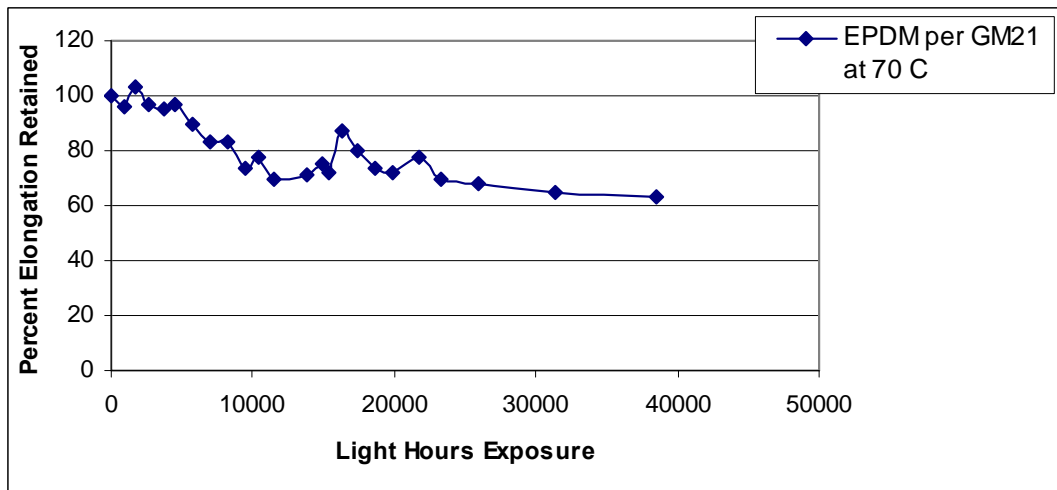
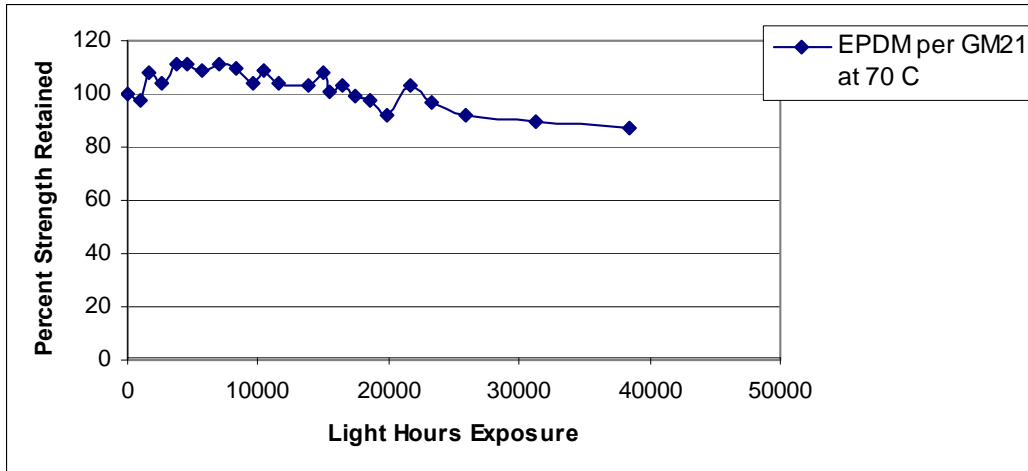


Figure 6c. Ethylene polypropylene diene terpolymer (EPDM) geomembrane.

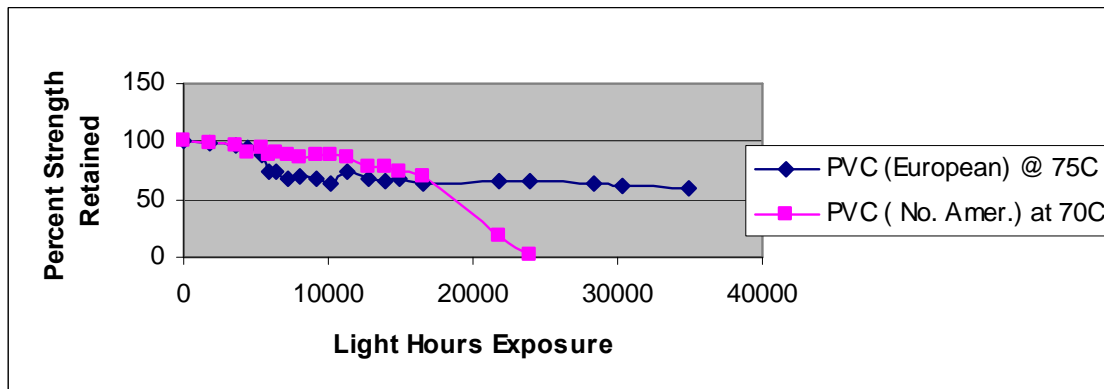


Figure 6d. Polyvinyl chloride (PVC) geomembranes.

From the response curves of the various geomembranes shown in Figure 6a-d, the 50% reduction value in strength or elongation (usually elongation) was taken as being the “halflife”. This value is customarily used by the polymer industry as being the materials lifetime prediction value. We have done likewise to develop Table 6 which is our predicted values for the designated exposed geomembrane lifetimes to date.

Table 6 – Exposed lifetime prediction results of selected geomembranes to date

Type	Specification	Prediction Lifetime in a Dry and Arid Climate
HDPE	GRI-GM13	> 36 years (ongoing)
LLDPE	GRI-GM17	≈ 36 years (halflife)
EPDM	GRI-GM21	> 27 years (ongoing)
fPP-2	GRI-GM18	≈ 30 years (halflife)
fPP-3	GRI-GM18	> 27 years (ongoing)
PVC-N.A.	(see FGI)	≈ 18 years (halflife)
PVC-Eur.	proprietary	> 32 years (ongoing)

4.0 Conclusions and Recommendations

This White Paper is bifurcated into two very different parts; covered (or buried) lifetime prediction of HDPE geomembranes and exposed (to the atmosphere) lifetime prediction of a number of geomembrane types. In the covered geomembrane study we chose the geomembrane type which has had the majority of usage, that being HDPE as typically used in waste containment applications. Invariably whether used in landfill liner or cover applications *the geomembrane is covered*. After ten-years of research Table 2 (repeated here) was developed which is the conclusion of the covered geomembrane research program. Here it is seen that HDPE decreases its predicted lifetime (as measured by its halflife) from 446-years at 20°C, to 69-years at 40°C. Other geomembrane types (LLDPE, fPP, EPDM and PVC) have had

essentially no focused effort on their covered lifetime prediction of the type described herein. That said, all are candidates for additional research in this regard.

Table 2 - Lifetime prediction of HDPE (nonexposed) at various field temperatures

In Service Temperature (°C)	Stage “A” (years)			Stage “B” (years)	Stage “C” (years)	Total Prediction* (years)
	Standard OIT	High Press. OIT	Average OIT			
20	200	215	208	30	208	446
25	135	144	140	25	100	265
30	95	98	97	20	49	166
35	65	67	66	15	25	106
40	45	47	46	10	13	69

*Total = Stage A (average) + Stage B + Stage C

Exposed geomembrane lifetime was addressed from the perspective of field performance which is very unequivocal. Experience in Europe, mainly with relatively thick PVC containing high molecular weight plasticizers, has given 25-years of service and the geomembranes are still in use. Experience in the USA with exposed geomembranes on flat roofs, mainly with EPDM and CSPE, has given 20⁺-years of service. The newest geomembrane type in such applications is fPP which currently carries similar warranties.

Rather than using the intricate laboratory setups of Figure 1 which are necessary for covered geomembranes, exposed geomembrane lifetime can be addressed by using accelerating laboratory weathering devices. Here it was shown that the UV fluorescent device (per ASTM D7238 settings) versus the Xenon Arc device (per ASTM D 4355) is equally if not slightly more intense in its degradation capabilities. As a result, all further incubation has been using the UV fluorescent devices per D7238 at 70°C.

Archived flexible polypropylene geomembranes at four field failure sites resulted in a correlation factor of 1200 light hours equaling one-year performance in a hot climate. Using this

value on the incubation behavior of seven commonly used geomembranes has resulted in the following conclusions (recall Figure 6 and Table 6);

- HDPE geomembranes (per GRI-GM13) are predicted to have lifetimes greater than 36-years; testing is ongoing.
- LLDPE geomembranes (per GRI-GM17) are predicted to have lifetimes of approximately 36-years.
- EPDM geomembranes (per GRI-GM21) are predicted to have lifetimes of greater than 27-years; testing is ongoing.
- fPP geomembranes (per GRI-GM18) are predicted to have lifetimes of approximately 30-years.
- PVC geomembranes are very dependent on their plasticizer types and amounts, and probably thicknesses as well. The North American formulation has a lifetime of approximately 18-years, while the European formulation is still ongoing after 32-years.

Regarding continued and future recommendations with respect to lifetime prediction, GSI is currently providing the following:

- (i) Continuing the exposed lifetime incubations of HDPE, EPDM and PVC (European) geomembranes at 70°C.
- (ii) Beginning the exposed lifetime incubations of HDPE, LLDPE, fPP, EPDM and both PVC's at 60°C and 80°C incubations.
- (iii) With data from these three incubation temperatures (60, 70 and 80°C), time-temperature-superposition plots followed by Arrhenius modeling will eventually provide information such as Table 2 for covered geomembranes. This is our ultimate goal.

- (iv) Parallel lifetime studies are ongoing at GSI for four types of geogrids and three types of turf reinforcement mats at 60, 70 and 80°C.
- (v) GSI does not plan to duplicate the covered geomembrane study to other than the HDPE provided herein. In this regard, the time and expense that would be necessary is prohibitive.
- (vi) The above said, GSI is always interested in field lifetime behavior of geomembranes (and other geosynthetics as well) whether covered or exposed.

Acknowledgements

The financial assistance of the U. S. Environmental Protection Agency for the covered HDPE lifetime study and the member organizations of the Geosynthetic Institute and its related institutes for research, information, education, accreditation and certification is sincerely appreciated. Their identification and contact member information is available on the Institute's web site at <<geosynthetic-institute.org>>.

References

- Cazzuffi, D., "Long-Term Performance of Exposed Geomembranes on Dams in the Italian Alps," Proc. 6th Intl. Conf. on Geosynthetics, IFAI, 1998, pp. 1107-1114.
- Chailan, J.-F., Boiteux, C., Chauchard, J., Pinel, B. and Seytre, G., "Effect of Thermal Degradation on the Viscoelastic and Dielectric Properties of Chlorosulfonated Polyethylene (CSPE) Compounds," Journal of Polymer Degradation and Stability, Vol. 48, 1995, pp. 61-65.
- Comer, A. I., Hsuan, Y. G. and Konrath, L., "The Performance of Flexible Polypropylene Geomembranes in Covered and Exposed Environments," 6th International Conference on Geosynthetics, Atlanta, Georgia, USA, March, 1998, pp. 359-364.
- Gedde, U. W., Viebke, J., Leijstrom, H. and Ifwarson, M., "Long-Term Properties of Hot-Water Polyolefin Pipes - A Review," Polymer Engineering and Science, Vol. 34, No. 24, 1994, pp. 1773-1787.
- Giroud, J.-P. and Tisinger, L. G., "The Influence of Plasticizers on the Performance of PVC Geomembranes," PVC GRI-17 Conference, IFAI, Roseville, MN, 1994, pp. 169-196.
- Hammon, M., Hsuan, G., Levin, S. B. and Mackey, R. E., "The Re-examination of a Nine-Year-Old PVC Geomembrane Used in a Top Cap Application," 31st Annual SWANA Conference, San Jose, CA, 1993, pp. 93-108.

- Hsuan, Y. G. and Guan, Z., "Evaluation of the Oxidation Behavior of Polyethylene Geomembranes Using Oxidative Induction Time Tests," ASTM STP 1326, Riga and Patterson, Eds., ASTM, 1997, pp. 138-149.
- Hsuan, Y. G. and Koerner, R. M., "Can Outdoor Degradation be Predicted by Laboratory Acceleration Weathering?," GFR, November, 1993, pp. 12-16.
- Hsuan, Y. G. and Koerner, R. M., "Antioxidant Depletion Lifetime in High Density Polyethylene Geomembranes," Jour. Geotech. and Geoenviron. Engr., ASCE, Vol. 124, No. 6, 1998, pp. 532-541.
- ICOLD (2010), "Geomembrane Sealing Systems for Dams: Design Principles and Return of Experience," Intl. Committee on Large Dams, Bulletin 135, Paris, France.
- Koerner, G. R. and Koerner, R. M., "In-Situ Temperature Monitoring of Geomembranes," Proc. GRI-18 Conf. at GeoFrontiers, Austin, TX, 2005, 6 pgs.
- Martin, J. R. and Gardner, R. J. (1983), "Use of Plastics in Corrosion Resistant Instrumentation," 1983 Plastics Seminar, NACE, October 24-27.
- Miller, L. V., Koerner, R. M., Dewyea, J. and Mackey, R. E., "Evaluation of a 30 mil PVC Liner and Leachate Collection System," Proc. 29th Annual GRCDA/SWANA Conf., Cincinnati, OH, 1991.
- Müller, W. and Jakob, I., "Oxidative Resistance of High-Density Polyethylene Geomembranes," Jour. Polymer Degradation and Stability," Elsevier Publ. Co., No. 79, 2003, pp. 161-172.
- Rapoport, N. Y. and Zaikov, G. E., "Kinetics and Mechanisms of the Oxidation of Stressed Polymer," Developments in Polymer Stabilization—4, G. Scott, Ed., Applied Science Publishers Ltd., London, U.K., 1986, pp. 207-258.
- Sangam, H. P. and Rowe, R. K., "Effects of Exposure Conditions on the Depletion of Antioxidants from HDPE Geomembranes," Canadian Geotechnical Journal, Vol. 39, 2002, pp. 1221-1230.
- Scuero, A., "The Use of Geocomposites for the Rehabilitation of Concrete Dams," Proc. 4th Intl. Conf. on Geosynthetics, The Hague, Balkema Publ. Co., 1990, pg. 474.
- Scuero, A. M. and Vaschetti, G. L., "Geomembranes for Masonry and Concrete Dams: State-of-the-Art Report," Proc. Geosynthetics Applications, Design and Construction, M. B. deGroot, et al., Eds., A. A. Balkema, 1996, pp. 889-898.
- Suits, L. D. and Hsuan, Y. G., "Assessing the Photo Degradation of Geosynthetics by Outdoor Exposure and Laboratory Weatherometers," Proc. GRI-15 Conference, Hot Topics in Geosynthetics II, GII Publ., Folsom, PA, 2001, pp. 267-279.
- Wang, W. and Qu, B., "Photo and Thermo-Oxidative Degradation of Photocrosslinked Ethylene-Propylene-Diene Terpolymer," Journal of Polymer Degradation and Stability, Vol. 81, 2003, pp. 531-537.

C-147 (revised 2/12/18)

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Albuquerque, NM 87104

District I
1625 N. French Dr., Hobbs, NM 88240
District II
811 S. First St., Artesia, NM 88210
District III
1000 Rio Brazos Road, Aztec, NM 87410
District IV
1220 S. St. Francis Dr., Santa Fe, NM 87505

State of New Mexico
Energy Minerals and Natural Resources
Department
Oil Conservation Division
1220 South St. Francis Dr.
Santa Fe, NM 87505

Form C-147
Revised April 3, 2017

Recycling Facility and/or Recycling Containment

Type of Facility: ☒ Recycling Facility ☒ Recycling Containment*
Type of action: ☒ Permit ☒ Registration
☐ Modification ☐ Extension
☐ Closure ☐ Other (explain) _____

*** At the time C-147 is submitted to the division for a Recycling Containment, a copy shall be provided to the surface owner.**

Be advised that approval of this request does not relieve the operator of liability should operations result in pollution of surface water, ground water or the environment. Nor does approval relieve the operator of its responsibility to comply with any other applicable governmental authority's rules, regulations or ordinances.

1.

Operator: : ADVANCE ENERGY PARTNERS HAT MESA, LLC (For multiple operators attach page with information) OGRID #: 372417
Address: 11490 Westheimer Rd. STE 950, Houston, TX 77077
Facility or well name (include API# if associated with a well): Merchant Containment
OCD Permit Number: (For new facilities the permit number will be assigned by the district office)
U/L or Qtr/Qtr Section 35 Township 21S Range 33E County: Lea
Surface Owner: ☐ Federal ☐ State ☒ Private ☐ Tribal Trust or Indian Allotment

2.

☒ **Recycling Facility:**

Location of (if applicable): Latitude 32.441240 Longitude -103.546175 NAD83

Proposed Use: ☒ Drilling* ☒ Completion* ☒ Production* ☒ Plugging *

**The re-use of produced water may NOT be used until fresh water zones are cased and cemented*

☐ Other, requires permit for other uses. Describe use, process, testing, volume of produced water and ensure there will be no adverse impact on groundwater or surface water.

☒ Fluid Storage

☐ Above ground tanks ☒ Recycling containment ☐ Activity permitted under 19.15.17 NMAC explain type _____

☐ Activity permitted under 19.15.36 NMAC explain type: _____ ☐ Other explain _____

☐ For multiple or additional recycling containments, attach design and location information of each containment

☐ **Closure Report (required within 60 days of closure completion):** ☐ Recycling Facility Closure Completion Date: _____

3.

☒ **Recycling Containment:**

☐ Annual Extension after initial 5 years (attach summary of monthly leak detection inspections for previous year)

Center of Recycling Containment (if applicable Latitude 32.440492 Longitude -103.546077 NAD83

☐ For multiple or additional recycling containments, attach design and location information of each containment

☒ Lined ☐ Liner type: Thickness Secondary 40 mil Primary 60 mil ☐ LLDPE ☒ HDPE ☐ PVC ☐ Other _____

☐ String-Reinforced

Liner Seams: ☒ Welded ☐ Factory ☐ Other _____ Overflow Volume: 432,000 bbl Dimensions: L 500 x W 450 x D 23

☐ Recycling Containment Closure Completion Date: _____

4.

Bonding:

- ☒ Covered under bonding pursuant to 19.15.8 NMAC per 19.15.34.15(A)(2) NMAC (These containments are limited to only the wells owned or operated by the owners of the containment.)
- ☐ Bonding in accordance with 19.15.34.15(A)(1). Amount of bond \$ _____ (work on these facilities cannot commence until bonding amounts are approved)
- ☐ Attach closure cost estimate and documentation on how the closure cost was calculated.

5.

Fencing:

- ☐ Four foot height, four strands of barbed wire evenly spaced between one and four feet
- ☒ Alternate. Please specify __ 8-foot high Game Fence _____

6.

Signs:

- ☒ 12"x 24", 2" lettering, providing Operator's name, site location, and emergency telephone numbers
- ☐ Signed in compliance with 19.15.16.8 NMAC

7.

Variances:

Justifications and/or demonstrations that the proposed variance will afford reasonable protection against contamination of fresh water, human health, and the environment.

Check the below box only if a variance is requested:

☒ **Variance(s):** Requests must be submitted to the appropriate division district for consideration of approval. If a Variance is requested, include the variance information on a separate page and attach it to the C-147 as part of the application.

If a Variance is requested, it must be approved prior to implementation.

8.

Siting Criteria for Recycling Containment

Instructions: The applicant must provide attachments that demonstrate compliance for each siting criteria below as part of the application. Potential examples of the siting attachment source material are provided below under each criteria.

General siting

Ground water is less than 50 feet below the bottom of the Recycling Containment.

NM Office of the State Engineer - iWATERS database search; USGS; Data obtained from nearby wells

☐ Yes ☒ No
☐ NA

Within incorporated municipal boundaries or within a defined municipal fresh water well field covered under a municipal ordinance adopted pursuant to NMSA 1978, Section 3-27-3, as amended.

☐ Yes ☒ No
☐ NA

- Written confirmation or verification from the municipality; written approval obtained from the municipality

Within the area overlying a subsurface mine.

☐ Yes ☒ No

- Written confirmation or verification or map from the NM EMNRD-Mining and Minerals Division

Within an unstable area.

☐ Yes ☒ No

- Engineering measures incorporated into the design; NM Bureau of Geology & Mineral Resources; USGS; NM Geological Society; topographic map

Within a 100-year floodplain. FEMA map

☐ Yes ☒ No

Within 300 feet of a continuously flowing watercourse, or 200 feet of any other significant watercourse, or lakebed, sinkhole, or playa lake (measured from the ordinary high-water mark).

☐ Yes ☒ No

- Topographic map; visual inspection (certification) of the proposed site

Within 1000 feet from a permanent residence, school, hospital, institution, or church in existence at the time of initial application.

☐ Yes ☒ No

- Visual inspection (certification) of the proposed site; aerial photo; satellite image

Within 500 horizontal feet of a spring or a fresh water well used for domestic or stock watering purposes, in existence at the time of initial application.

☐ Yes ☒ No

- NM Office of the State Engineer - iWATERS database search; visual inspection (certification) of the proposed site

Within 500 feet of a wetland.

☐ Yes ☒ No

- US Fish and Wildlife Wetland Identification map; topographic map; visual inspection (certification) of the proposed site

9.

Recycling Facility and/or Containment Checklist:*Instructions: Each of the following items must be attached to the application. Indicate, by a check mark in the box, that the documents are attached.*

- ☒ Design Plan - based upon the appropriate requirements.
- ☒ Operating and Maintenance Plan - based upon the appropriate requirements.
- ☒ Closure Plan - based upon the appropriate requirements.
- ☒ Site Specific Groundwater Data -
- ☒ Siting Criteria Compliance Demonstrations -
- ☒ Certify that notice of the C-147 (only) has been sent to the surface owner(s)

10.

Operator Application Certification:

I hereby certify that the information and attachments submitted with this application are true, accurate and complete to the best of my knowledge and belief.

Name (Print): David Harwell Title: Vice President Engineering/OperationsSignature: David Harwell Date: 11-30-17e-mail address: DHarwell@advanceenergypartners.com Telephone: 832-672-4604

11.

OCD Representative Signature:  Approval Date: March 12, 2018Title: Environmental Specialist OCD Permit Number: 1RF-29☐ OCD Conditions☐ Additional OCD Conditions on AttachmentpOY1807154524fOY1807136904

Site Specific Information

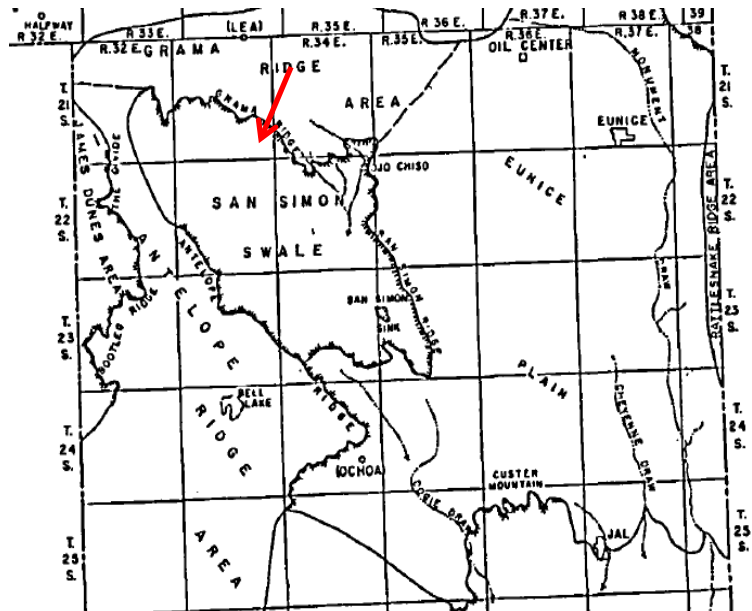
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Geologic Setting of the Regional Fresh-Water Bearing Formations

The recycling containment site is located within the northwest portion of the San Simon Swale (see inset below), which is on the eastern edge of the Pecos Valley Physiographic Province. Regionally, the San Simon Swale drains from the northwest to the southeast, toward the San Simon Sink, located approximately fifteen miles to the southeast of the site. The San Simon Swale in this area is approximately six miles wide extending from Grama Ridge on the northeast to Antelope Ridge on the southwest.

Groundwater in the area within and surrounding the San Simon Swale is found only in Mesozoic and Cenozoic Era rocks. The oldest of these are the Triassic age Dockum Group. They consist of conglomerates, cross-bedded sandstones, claystones, and siltstones that were deposited in a continental fluvial environment over the evaporites of the late Permian Ochoan Series, which had filled the Delaware Basin by that time.



Any Jurassic or Cretaceous age rocks that were deposited above the Triassic have subsequently been removed by erosion leaving an irregular surface on the Triassic rocks. Cenozoic Era rocks in the area consist of the Tertiary age Ogallala Formation and Quaternary age eolian and piedmont deposits. The Ogallala Formation consists of terrestrial sediments (sand with some clay, silt and gravel) that were deposited on the Triassic age rocks.

The Ogallala and associated alluvium aquifers are the primary groundwater source where they are present at the higher elevations outside of the San Simon Swale. Water wells drilled within the San Simon Swale target only the Triassic age rocks and are designated by the USGS as producing from either Chinle or Santa Rosa aquifers.

Distance to Groundwater

Figure 1, Figure 2, and the discussion presented below demonstrates that groundwater (fresh water as defined by NMOCD Rules) at the location is greater than 100 feet beneath the containment.

Figure 1 is an area geologic base map that depicts regional topography (metric contour units) and includes the water wells located nearest to the containment site for which information is available, regardless of how comprehensive or useful. It also shows:

1. The location of the recycling containment as a blue rectangle.

Siting Criteria (19.15.34.11 NMAC) Advance Energy - Merchant Containment

2. Water wells from the USGS database as color-coded triangles that indicate the producing aquifer (see Legend).
3. Water wells from the New Mexico Office of the State Engineer (OSE) database as a small blue triangle inside a colored circle that indicates the well depth (see Legend). Please note, OSE wells are often miss-located in the WATERS database as older wells are plotted in the center of the quarter, quarter, quarter, of the Section Township and Range. Topographic maps and/or aerial photographs verified all of the OSE well locations included on this map.
4. Water wells, which are not documented in the public databases but were identified by field inspection or other published reports are shown as a dot inside a color-coded (depth) square.
5. Depth to water and gauging dates from the most recent and reliable measurement for each well is provided adjacent to the well symbol. It should be noted that in most cases the depth to water provided by the OSE database are from drillers log notes estimated at the time of completion, rather than actual field measurements.

Figure 2 is a regional geologic base map (metric contour units) that depicts the potentiometric surface contours of the shallow-most aquifer surrounding the site. The potentiometric contours are labeled in feet above sea level (ASL). The water wells plotted include only the USGS database and published report water wells from Figure 1 for which a reliable depth to water measurement has been recorded. Figure 2 also shows:

1. The location of the containment as a blue rectangle.
2. Groundwater elevations and gauging dates from the most recent available static water level measurement for each well.

Site Geology

The proposed containment is located on an outcrop of Quaternary Age eolian and piedmont deposits (Qe/Qp on Figure 1). These fine-grained sands and clays, along with the Quaternary piedmont deposits and Quaternary lacustrine/playa deposits (Qp and Qpl on Figure 1), are present as a thin covering of the underlying ero Triassic age rocks.

The surface drainage is generally to the northeast, however the area is covered by low stabilized sand dunes and no continuously flowing or any other significant watercourses are present.

Water Table Elevation and Depth to Groundwater

Twelve measurements of water wells in the area surrounding the containment site provide a good estimate of the groundwater elevation in the area (see Figure 2). Figure 2 uses only data from the USGS, published, and field verified well information, which is generally considered reliable. As stated earlier, the groundwater elevations provided for these OSE wells are likely based on driller log notes after well completion rather than measurements made under static conditions.

Based on the potentiometric surface contours created using the available measurements from surrounding wells (Figure 2), we conclude that the groundwater elevation at the containment site is approximately 3,490 feet ASL. With a surface elevation of 3,672 feet ASL and a maximum depth of the containment of 25 feet, the depth to groundwater below the containment floor should be approximately 157 feet.

Distance to Surface Water

Figure 3 and the site visit demonstrates that the location is not within 300 feet of a continuously flowing watercourse, or any other significant watercourse, or within 200 feet of any lakebed, sinkhole, or playa lake (measured from the ordinary high-water mark).

No continuously flowing watercourses exist within 300 feet of the location. The nearest surface drainage feature (un-named intermittent streams identified on the USGS quadrangle map) are located about 1 mile to the southwest and 1 mile east, as identified in Figure 3. Stabilized dune fields, like that which characterizes the location, are seldom characterized by well-defined drainage patterns.

Distance to Permanent Residence or Structures

Figure 4 and the site visit demonstrates that the location is not within 300 feet from a permanent residence, school, hospital, institution, church, or other structure in existence at the time of initial application.

Distance to Non-Public Water Supply

Figures 1, 2, and 3 demonstrate that the location is not within 500 horizontal feet of a private, domestic fresh water well or spring that less than five households use for domestic or stock watering purposes, or within 1000 horizontal feet of any other fresh water well or spring, in existence at the time of initial application.

- Figure 1 shows the locations of all area water wells. The nearest fresh water well is a well drilled relatively recently (2014) labeled CP-1114 Pod 2. This is a deep well with a reported location about ½ mile to the east. This well would be used for oil field water supply.
- There are no known domestic water wells located within the mapping area.
- Figure 3 shows that no springs are identified within the mapping area.

Distance to Municipal Boundaries and Fresh Water Fields

Figure 5 demonstrates that the location is not within incorporated municipal boundaries or defined municipal fresh water well fields covered under a municipal ordinance adopted pursuant to NMSA 1978, Section 3-27-3, as amended.

- The closest municipality is Eunice, NM approximately 30 miles to the east.
- The closest public well field is located approximately 27 miles to the north.

Distance to Wetlands

Figure 6 demonstrates the location is not within 500 feet of wetlands.

- The nearest designated wetlands are east of Grama Ridge and considerably higher in elevation than the containment
- Dagger Lake; a “Freshwater Pond” located 2.5 miles to the south-southeast.

Distance to Subsurface Mines

Figure 7 and our general reconnaissance of the area demonstrate that the nearest mines are caliche pits.

- Figures 3 and 4 show a caliche pit about ½ mile northwest
- The nearest registered caliche pit is located approximately 1 mile to the northwest – and is probably the same pit shown in Figures 3 and 4.

Distance to High or Critical Karst Areas

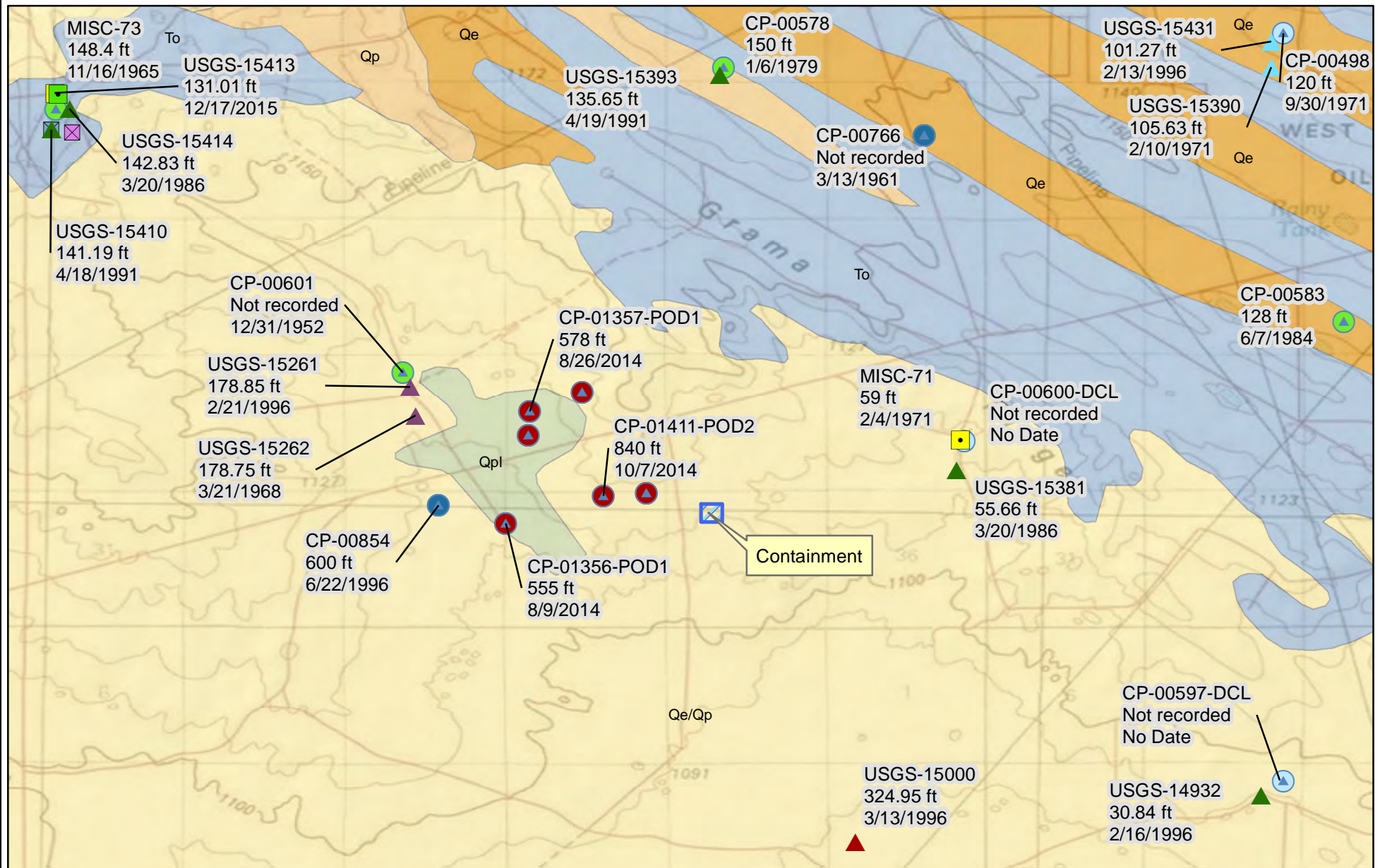
Figure 8 shows the location of the recycling containment with respect BLM Karst areas

- The proposed recycling containment is located within a “low” potential karst area.
- The nearest “high” or “critical” potential karst area is located approximately 15 miles west of the site.
- We saw no evidence of unstable ground near the containment location during the site inspection.

Distance to 100-Year Floodplain

Figure 9 demonstrates that the location is within an area that has not yet been mapped by the Federal Emergency Management Agency with respect to the Flood Insurance Rate 100-Year Floodplain.

- Areas that are not mapped are designated as “Undetermined Flood Hazard” and are generally considered minimal flood risk.
- Our field inspection and examination of the topography permit a conclusion that the location is not within any floodplain.

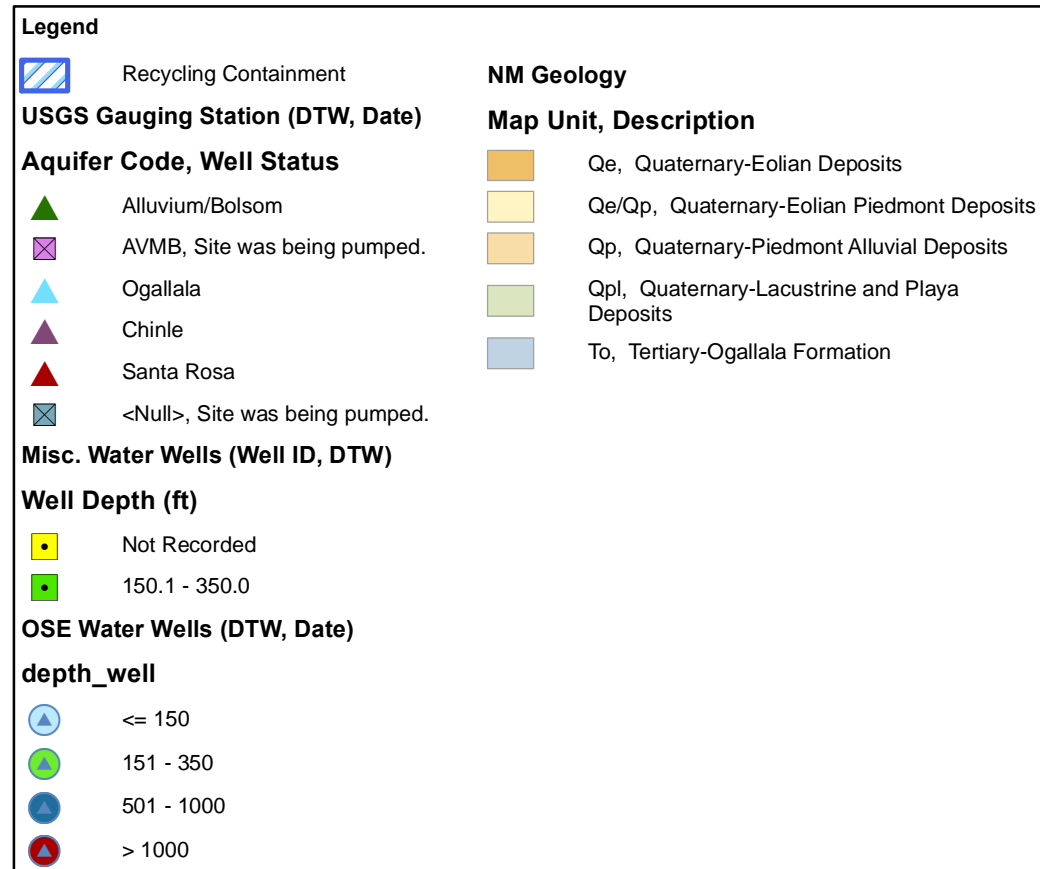


0 0.5 1
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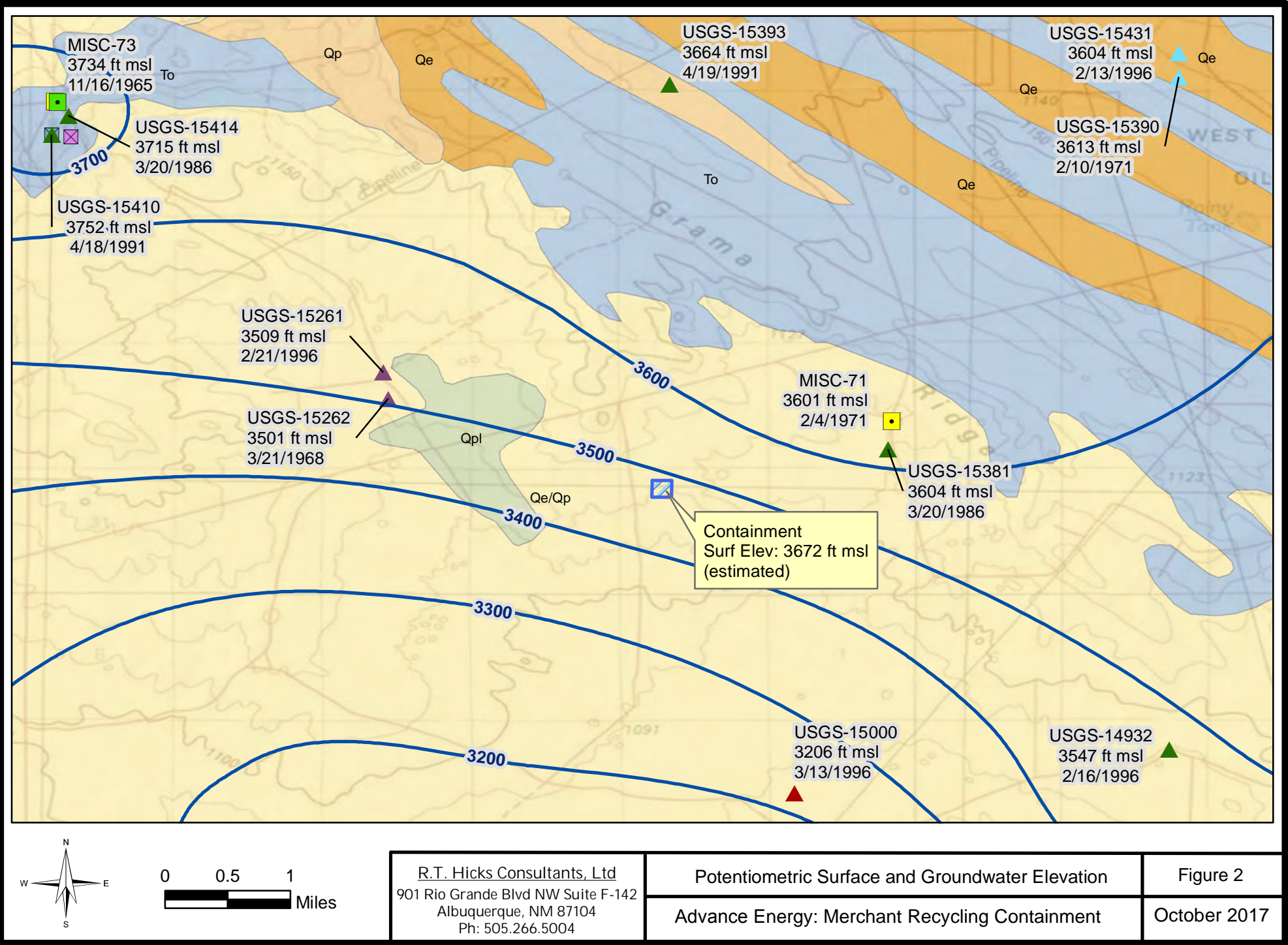
R.T. Hicks Consultants, Ltd
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Depth To Water and Geology
Advance Energy: Merchant Recycling Containment

Figure 1
October 2017



R.T. Hicks Consultants, Ltd 901 Rio Grande Blvd NW Suite F-142 Albuquerque, NM 87104 Ph: 505.266.5004	Depth To Water and Geology	Figure 1 LEGEND
	Advance Energy: Merchant Recycling Containment	October 2017



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Potentiometric Surface and Groundwater Elevation

Advance Energy: Merchant Recycling Containment

Figure 2

October 2017

Legend



Recycling Containment

Potentiometric Surface (ft msl)

Isocontours



Isocontour

Misc. Water Wells (GW Elev, Date)

Well Depth (ft)



No Data



151 - 350

USGS Gauging Station (GW Elev, Date)

Aquifer Code, Well Status



Alluvium/Bolsom



AVMB, Site was being pumped.



Ogallala



Chinle



Santa Rosa



<Null>, Site was being pumped.

NM Geology

Map Unit, Description



Qe, Quaternary-Eolian Deposits



Qe/Qp, Quaternary-Eolian Piedmont Deposits



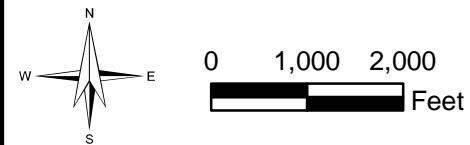
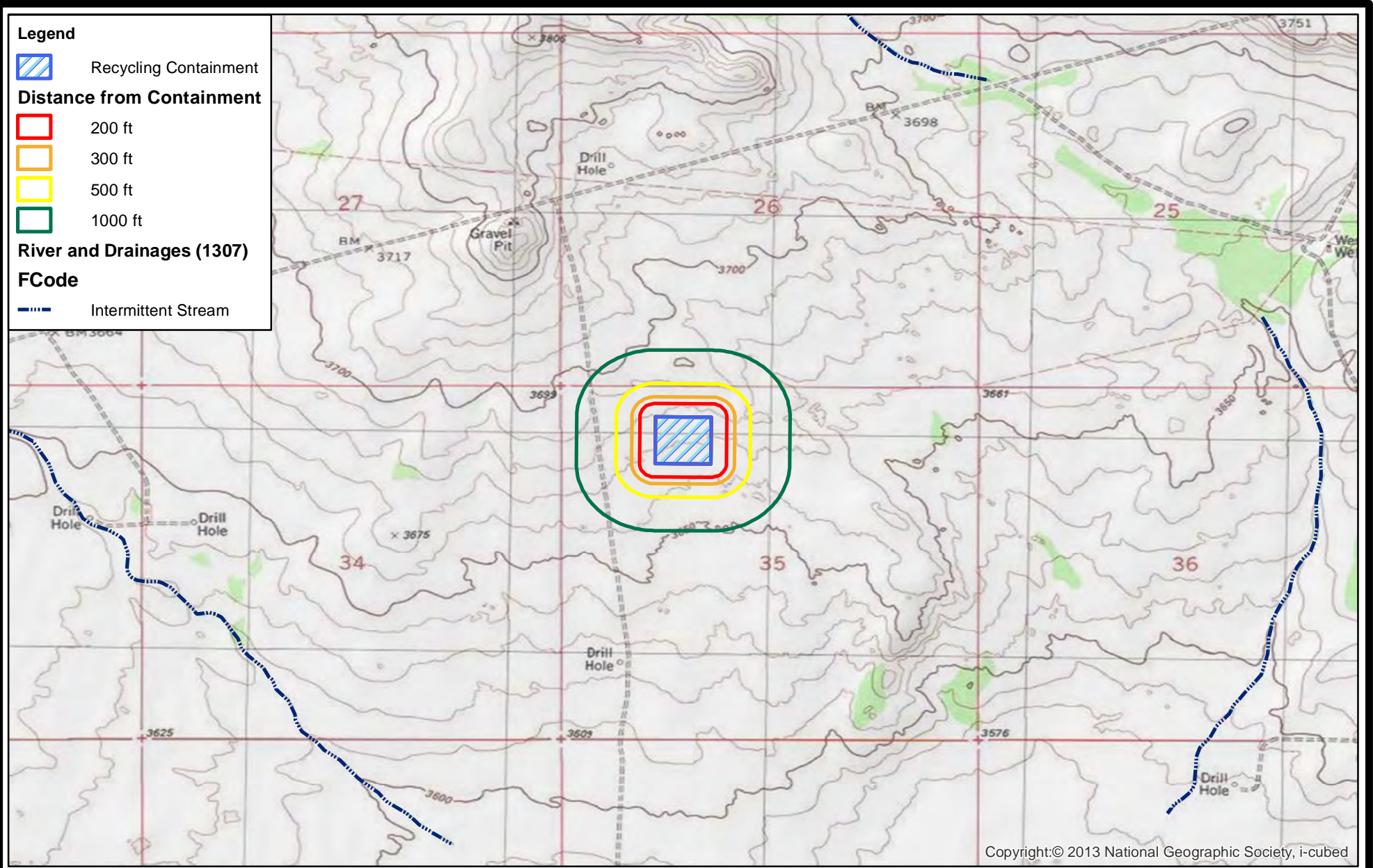
Qp, Quaternary-Piedmont Alluvial Deposits



Qpl, Quaternary-Lacustrine and Playa Deposits



To, Tertiary-Ogallala Formation



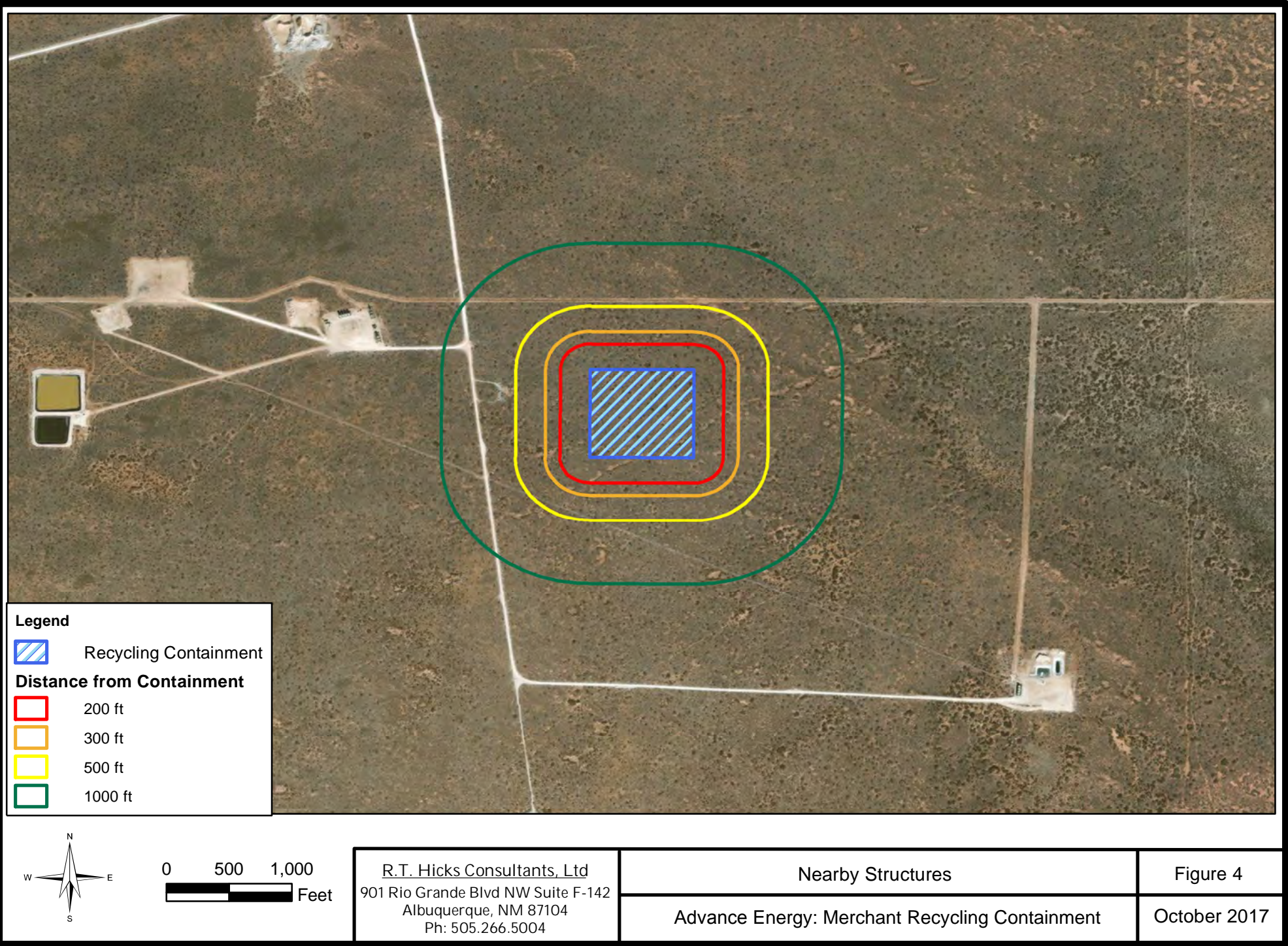
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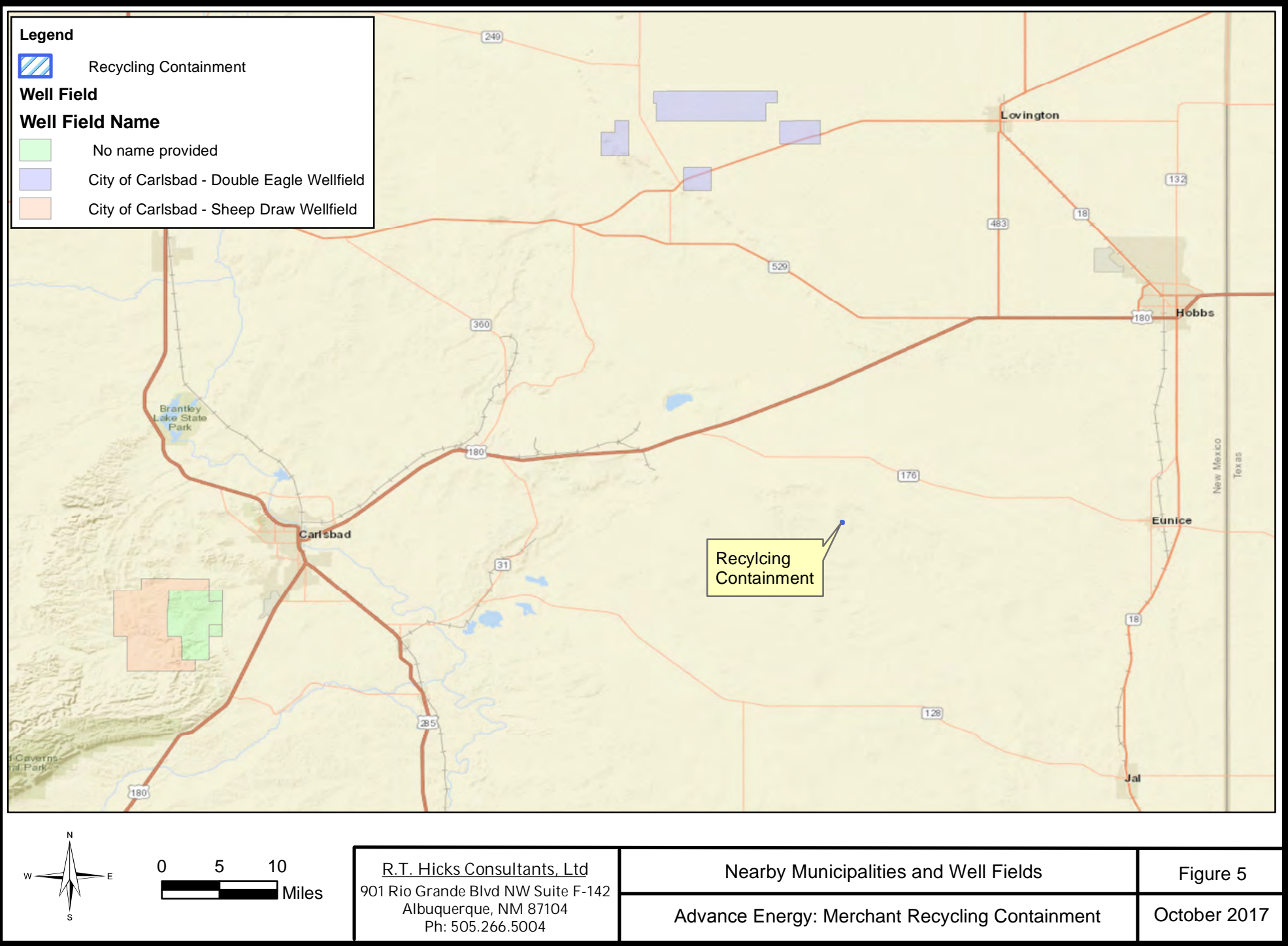
Surface Water and Topography

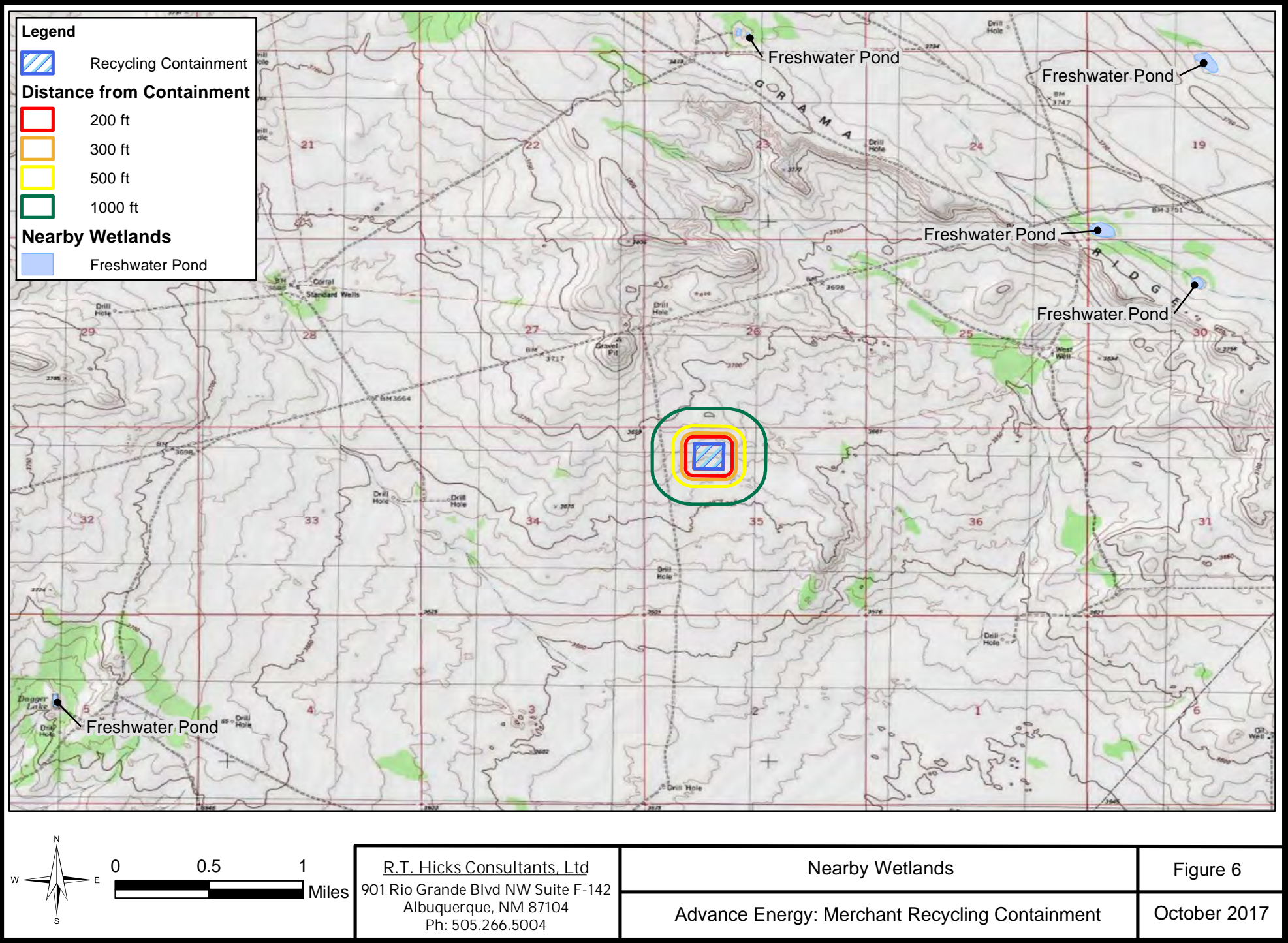
Advance Energy: Merchant Recycling Containment

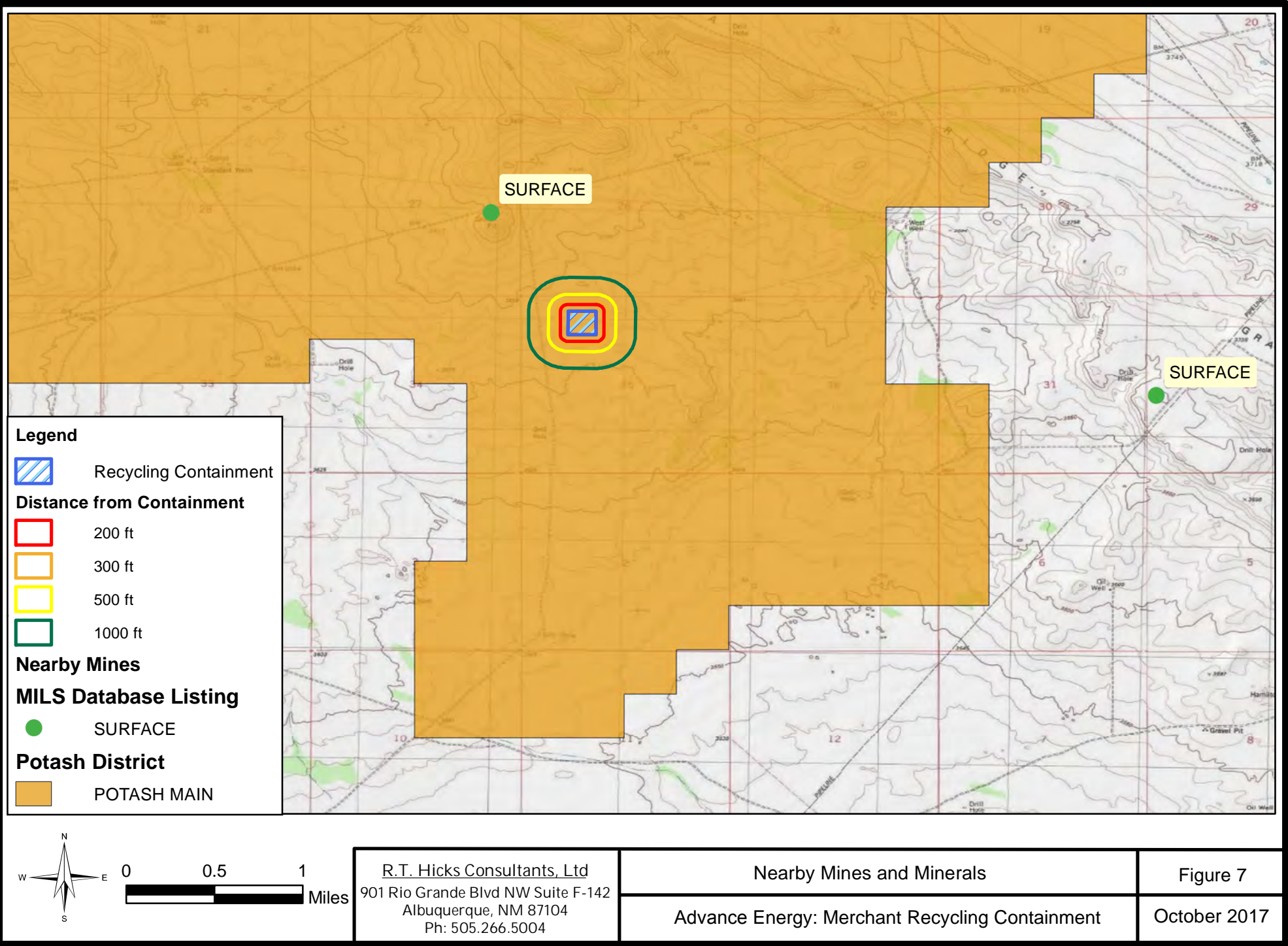
Figure 3

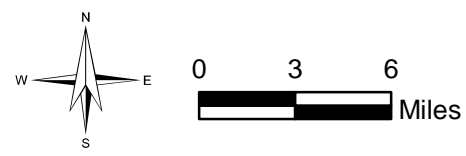
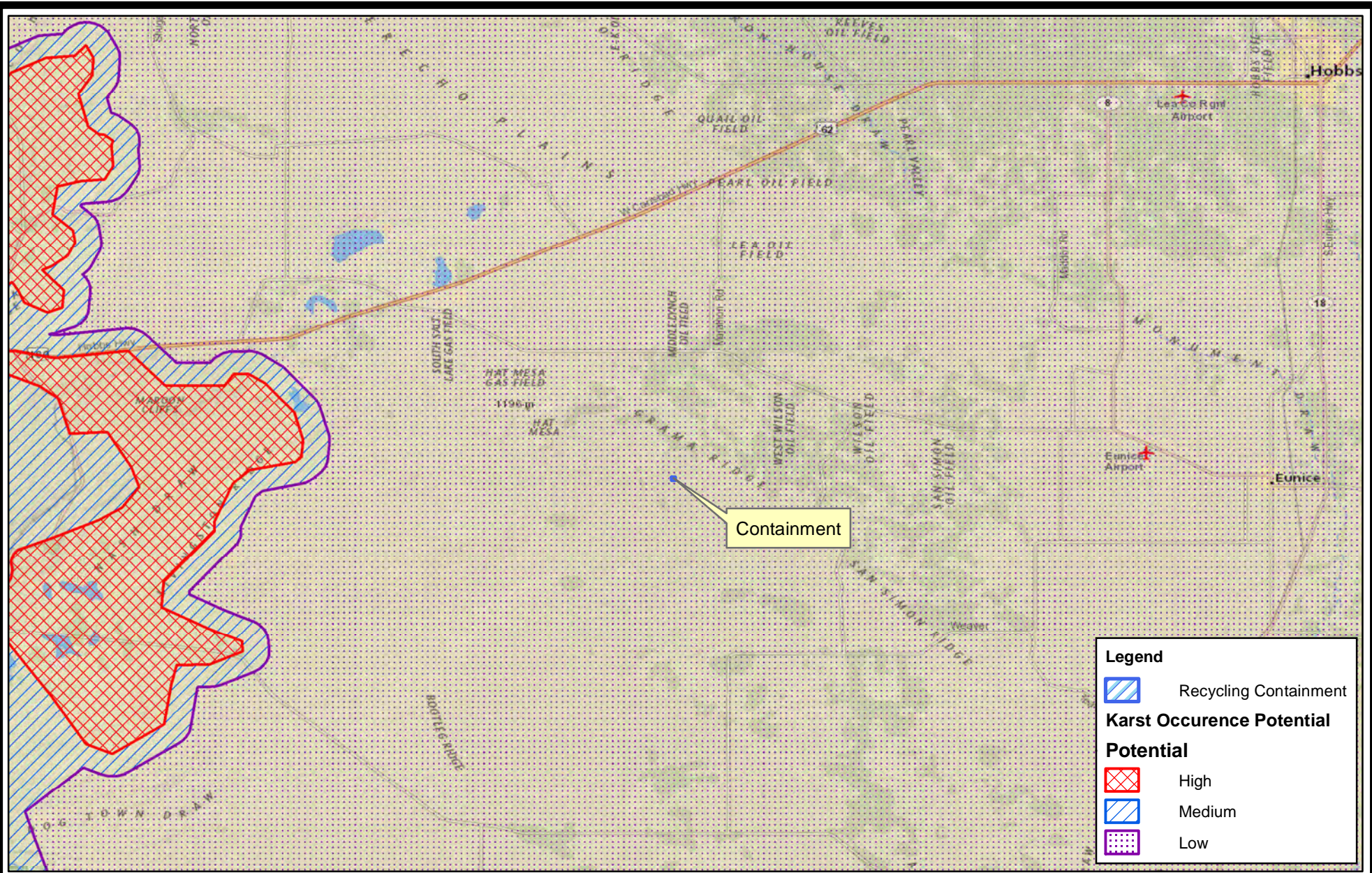
October 2017

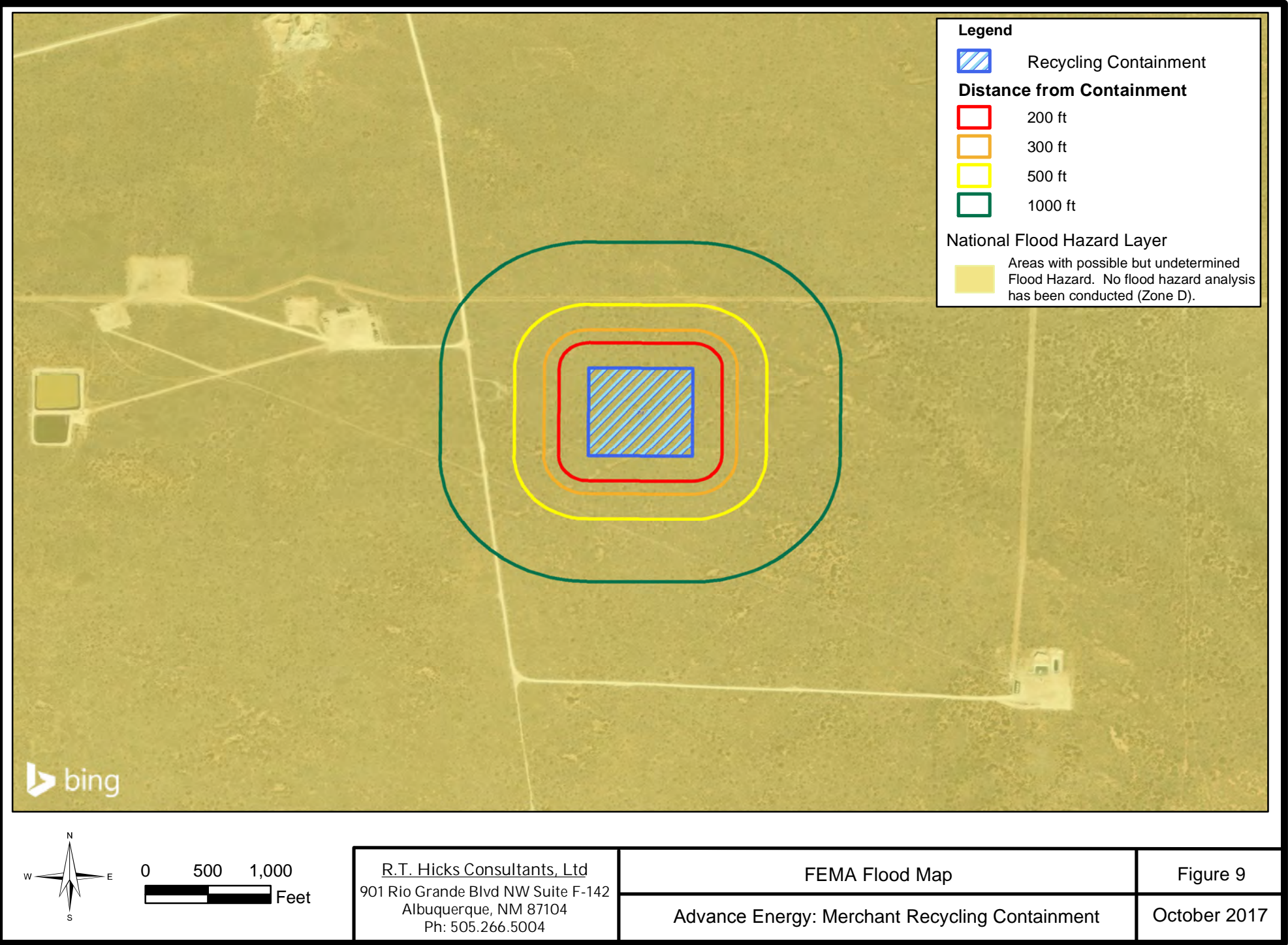












Appendix A

Containment Design

Primary Liner Data Sheet

Secondary Liner Data Sheet

Geonet Data Sheet

Liner Installation Manual

Avian Protection Program

R.T. Hicks Consultants, Ltd.

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Albuquerque, NM 87104

MERCHANT RECYCLING CONTAINMENT AND RECYCLING FACILITY

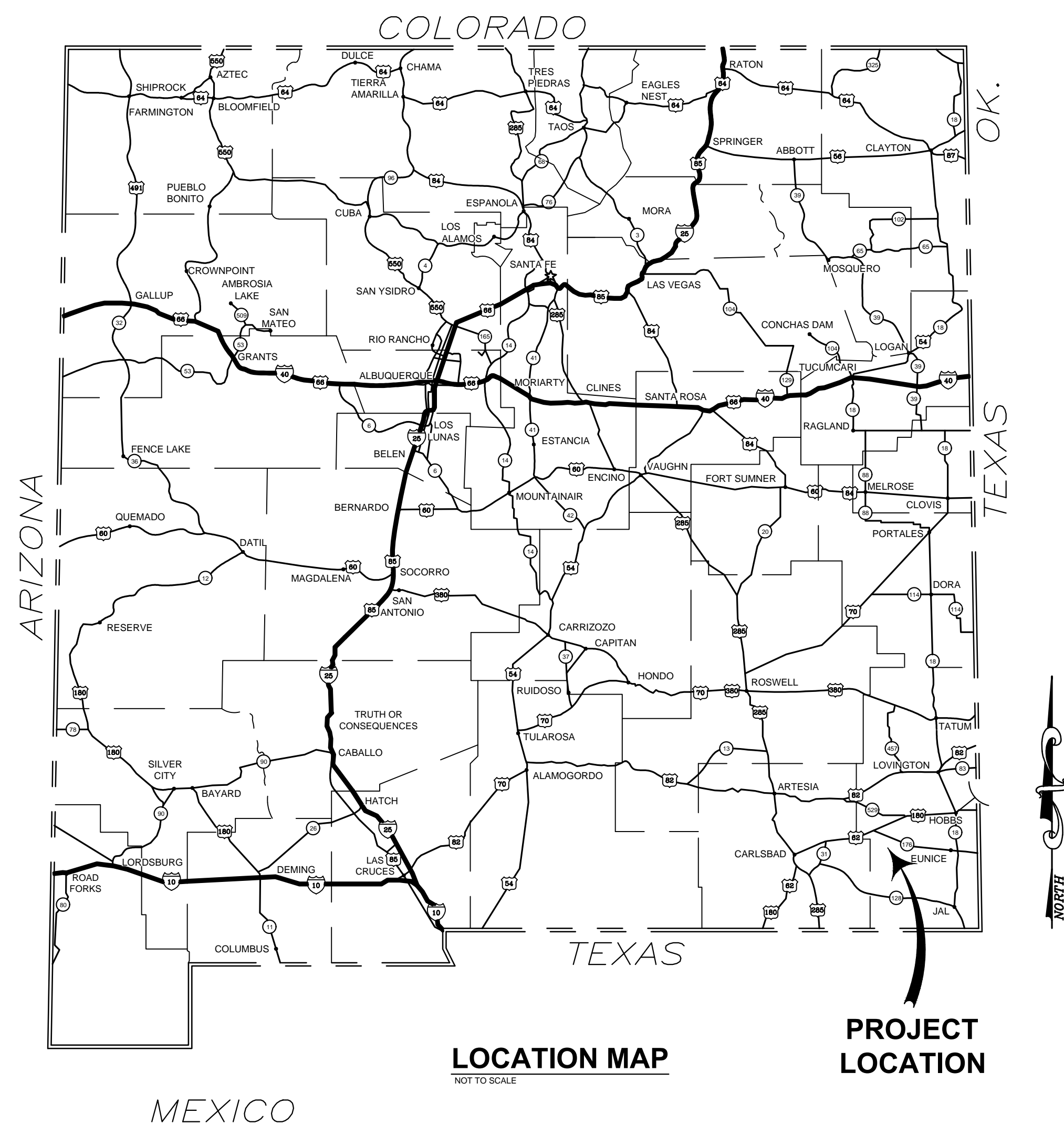
ADVANCE ENERGY PARTNERS HAT MESA, LLC

LEA COUNTY, NEW MEXICO



Jonathan Arcey 12/1/17

PROJECT ENGINEER:	JMC
PROJECT DESIGNER:	JMC
DRAWN BY:	JMC



VICINITY MAP

REVISIONS

[illegible]

COVER SHEET

MERCHANT RECYCLING
CONTAINMENT AND
RECYCLING FACILITY

ADVANCE ENERGY PARTNERS
HAT MESA, LLC

PROJECT NUMBER:
2017.1238

SHEET:

C-001



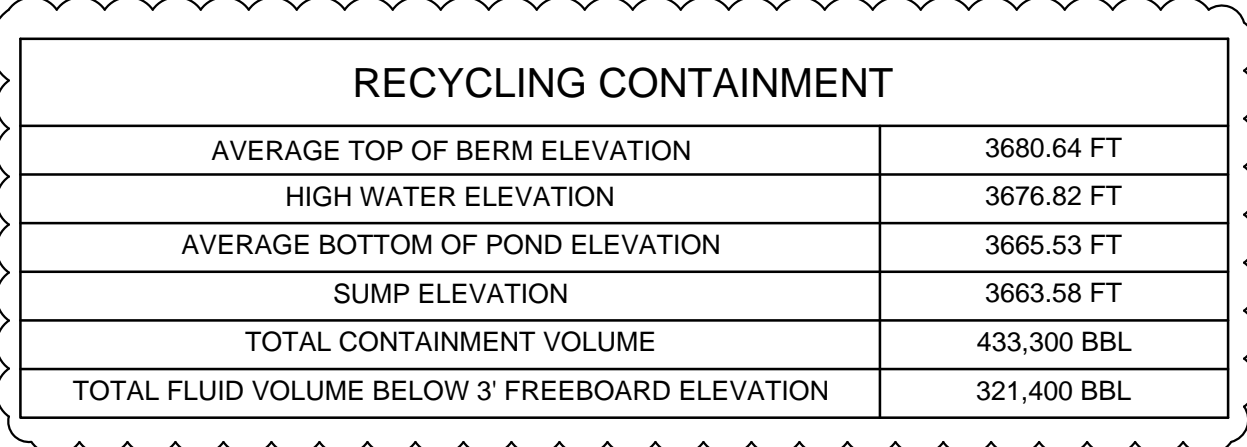
THE LOCATIONS OF EXISTING UNDERGROUND UTILITIES ARE SHOWN IN AN APPROXIMATE WAY ONLY AND HAVE NOT BEEN INDEPENDENTLY VERIFIED BY THE OWNER OR ITS REPRESENTATIVE. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION OF ALL EXISTING UTILITIES BEFORE COMMENCING WORK, AND AGREES TO BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE OCCASIONED BY THE CONTRACTOR'S FAILURE TO EXACTLY LOCATE AND PRESERVE ANY AND ALL UNDERGROUND UTILITIES.

FLOOD NOTE

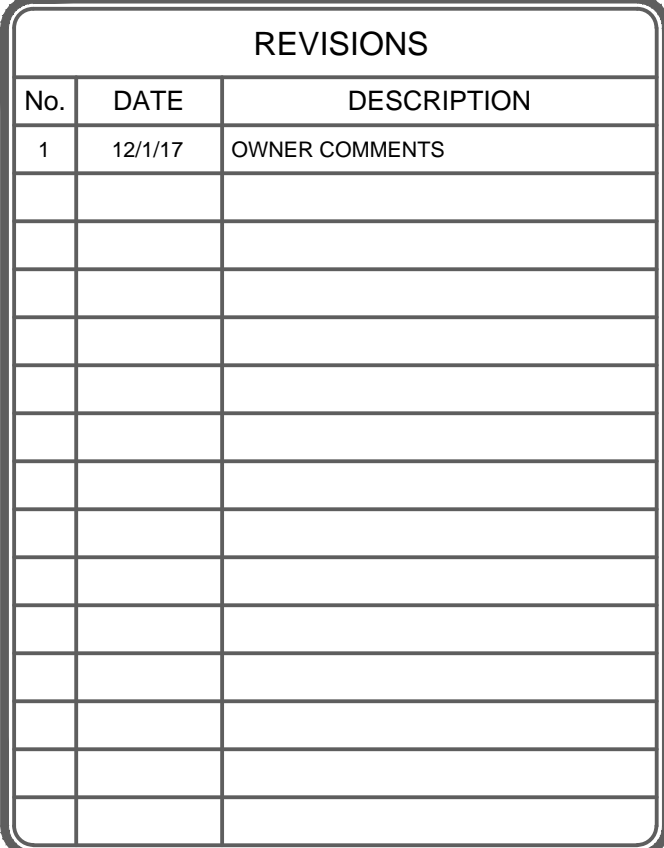
THIS PROJECT IS LOCATED IN AN AREA WITHOUT A PRINTED FLOOD MAP CORRESPONDING TO FEMA FIRM PANEL 35025C1600D

INDEX OF DRAWINGS

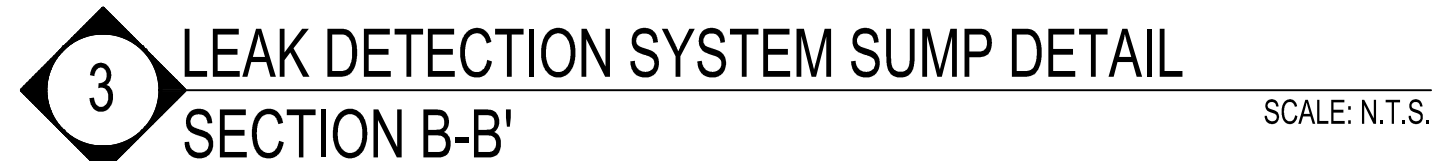
C-001 - COVER SHEET
CS-101 - SITE GRADING PLAN AND GENERAL NOTES
CS-501 - DETAILS



- ## GENERAL NOTES
1. THE CONTOURS SHOWN REPRESENT AS-BUILT CONDITIONS AS OF NOV. 29, 2017. STORM CONSTRUCTION SHALL INSURE THAT THE CONSTRUCTION OF THE POND BOTTOM (DENOTED BY KEYED NOTE 8) ARE FILLED AND THE POND BOTTOM SLOPES CONSISTENTLY TOWARD THE SUMP TO THE SATISFACTION OF THE OWNER.
 2. ALL CONSTRUCTION SHALL BE PERFORMED IN ACCORDANCE WITH THE PROJECT CONSTRUCTION PLANS OF THE REGISTRATION/PERMIT. NEW MEXICO STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION LATEST EDITION SHALL APPLY TO THIS PROJECT.
 3. IT WILL BE THE CONTRACTOR'S RESPONSIBILITY TO SECURE AND SUPPLY WATER FOR THE PROJECT.
 4. THE PERIMETER OF THE PIT SHALL BE ENCLOSED BY A GAME FENCE AT THE TOP OF THE LEVEE AS PRESCRIBED BY THE DESIGN/CONSTRUCTION PLAN IN THE REGISTRATION/PERMIT APPLICATION. THE FENCE SHALL BE INSTALLED APPROXIMATELY 1 FT INSIDE THE OUTSIDE EDGE OF THE LEVEE.
 5. STRIP AND STOCKPILE TOP SOIL FOR FUTURE CLOSURE USE. NEW MEXICO ACT CODE SPECIFICATIONS, 19.15.34 NMWAC (DESIGN AND CONSTRUCTION SPECIFICATIONS FOR A RECYCLING CONTAINMENT IN REGISTRATION/PERMIT) SHALL APPLY TO THIS PROJECT.
 6. THE RECYCLING CONTAINMENT SHALL HAVE A PROPERLY CONSTRUCTED FOUNDATION AND INTERIOR SLOPES CONSISTING OF A FIRM, UNYIELDING BASE, SMOOTH AND FREE OF ROCKS, DEBRIS, SHARP EDGES OR IRREGULARITIES TO PREVENT THE LINER'S RUPTURE OR TEAR. GEOTEXTILE IS REQUIRED UNDER ALL EXPOSED LINER AREAS. LINER AREAS NEEDED TO REDUCE LOCALIZED STRESS-STRAIN OR PROTRUSIONS THAT OTHERWISE MAY COMPROMISE THE LINER'S INTEGRITY.
 7. AS DESCRIBED IN THE DESIGN/CONSTRUCTION PLANS OF THE REGISTRATION/PERMIT, ALL PRIMARY (UPPER) LINERS IN A RECYCLING CONTAINMENT SHALL BE GEOMEMBRANE LINERS COMPOSED OF AN IMPERVIOUS, SYNTHETIC MATERIAL THAT IS RESISTANT TO ULTRAVIOLET RADIATION, PETROLEUM HYDROCARBONS, ACIDIC AND ALKALINE WASTES. SECONDARY, ALL PRIMARY LINERS SHALL BE 30-MIL THICKNESS PVC, 45-MIL LDPE STRING REINFORCED OR 60-MIL HDPE LINERS. SECONDARY LINERS SHALL BE 30-MIL LDPE STRING REINFORCED OR EQUIVALENT WITH A HYDRAULIC CONDUCTIVITY NO GREATER THAN 1×10^{-9} CM/SEC. LINER IMPERMEABILITY SHALL MEET OR EXCEED THE EPA SW-846 METHOD 9050A OR SUBSEQUENT RELEVANT PUBLICATIONS.
 8. LINER SEAMS SHALL BE MINIMIZED AND ORIENTED UP AND DOWN, NOT ACROSS, A SLOPE OF THE LEVEE. FACTORY WELDED SEAMS SHALL BE USED WHERE POSSIBLE.
 9. UNLESS DIFFERENTLY STATED IN THE CONSTRUCTION PLAN OF THE REGISTRATION/PERMIT, THE OPERATOR SHALL INSURE FIELD SEAMS IN GEOSYNTHETIC MATERIAL ARE THERMALLY SEALED, PRIOR TO FIELD SEAMING, THE OPERATOR SHALL OVERLAP LINERS FOUR TO SIX INCHES. THE OPERATOR SHALL MINIMIZE THE NUMBER OF FIELD SEAMS AND JOINTS AND IRREGULARITIES. THE SEAMS SHALL BE NO MORE THAN 24 INCHES WITHIN FIVE FEET OF THE SLOPE'S TOE. QUALIFIED PERSONNEL SHALL PERFORM FIELD WELDING AND TESTING.
 10. AT A POINT OF DISCHARGE INTO OR SECTION FROM THE RECYCLING CONTAINMENT, THE OPERATOR SHALL INSURE THAT THE LINER IS PROTECTED FROM EXCESSIVE HYDROSTATIC FORCE OR MECHANICAL DAMAGE. EXTERNAL DISCHARGE OR SUCCTION LINES SHALL NOT PENETRATE THE LINER.
 11. THE OPERATOR SHALL POST AN UPRIGHT SIGN NO LESS THAN 12 INCHES BY 24 INCHES WITH LETTERING NOT LESS THAN TWO INCHES IN HEIGHT IN A CONSPICUOUS PLACE ON THE LEVEE SURROUNDING THE CONTAINMENT. THE OPERATOR SHALL POST THE SIGN IN A MANNER AND LOCATION SUCH THAT A PERSON CAN EASILY READ THE LEGEND. THE SIGN SHALL PROVIDE THE FOLLOWING INFORMATION: OPERATOR'S NAME, THE LOCATION OF THE SITE BY QUARTER SECTION, TOWNSHIP, RANGE, SECTION, TOWNSHIP AND RANGE, AND EMERGENCY TELEPHONE NUMBERS.
 12. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO CONTACT NEW MEXICO 811 (FORMERLY NEW MEXICO ONE CALL) A MINIMUM OF 48 HOURS BEFORE EXCAVATION. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL EXCAVATE AND VERIFY THE HORIZONTAL AND VERTICAL LOCATIONS OF ALL DESIGNATED UNDERGROUND UTILITIES. SHOULD A CONFLICT EXIST, THE CONTRACTOR SHALL NOTIFY AN ENGINEER SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY.
 13. DURING CONSTRUCTION OF THE CONTAINMENT, THE CONTRACTOR WILL REPORT AND RESPOND TO ANY SPILLS OF HAZARDOUS MATERIALS SUCH AS GASOLINE, DIESEL, MOTOR OILS, SOLVENTS, CHEMICALS, TOXIC OR CORROSIVE SUBSTANCES, ETC. A SPILL IS DEFINED AS ANY RELEASE OF A CORROSIVE, HAZARDOUS, TOXIC OR RADIOACTIVE SUBSTANCE THAT MAY BE A THREAT TO PUBLIC HEALTH OR THE ENVIRONMENT. REPORTS OF SPILLS WILL BE MADE IMMEDIATELY TO BOTH THE NEW MEXICO ENVIRONMENT DEPARTMENT EMERGENCY RESPONSE TEAM (505-827-9329 OR 866-428-6535) AND THE LEA COUNTY EMERGENCY DISPATCHER (575-663-1101) AND TO THE LEA COUNTY ENVIRONMENTAL PROTECTION AND CLEANUP OF ANY SPILL ASSOCIATED WITH PROJECT CONSTRUCTION. THE CONTRACTOR WILL BE RESPONSIBLE FOR REPORTING ANY DISCOVERIES OF PAST SPILLS OR CURRENT SPILLS NOT ASSOCIATED WITH CONSTRUCTION.
 14. CONTRACTOR MUST OBTAIN CLIENT PERMISSION BEFORE SALVAGING ANY ITEMS SPECIFIED FOR REMOVAL AND DISPOSAL AFTER COMPLETION OF CONSTRUCTION OF THE CONTAINMENT.
 15. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING DISPOSAL SITES THAT ARE ENVIRONMENTALLY SUITABLE FOR DISPOSAL OF ITEMS NOT SPECIFIED TO BE SALVAGED. IF THE CONTRACTOR IS EXPECTED TO ABIDE BY ALL FEDERAL, STATE, AND LOCAL LAWS AND REGULATIONS IN OBTAINING THE NECESSARY PERMITS FROM ALL APPLICABLE AGENCIES AND/OR PRIVATE PROPERTY OWNERS, ALL COSTS ASSOCIATED WITH OBTAINING THESE PERMITS SHALL BE THE CONTRACTOR'S RESPONSIBILITY. THE CONTRACTOR SHALL OBTAIN MEASUREMENT OR PAYMENT SHALL BE MADE THEREFORE. THE CONTRACTOR SHALL PROVIDE THE ENGINEER WITH COPIES OF ALL PERTINENT INFORMATION, AGREEMENTS, AND PERMITS RELATED TO DISPOSAL SITES UTILIZED. BORROW MATERIAL, ROCK WASTE, AND QUANTITATIVE DEBRIS SHALL NOT BE USED IN WETLAND AREAS THAT MAY BE DIRECTLY IMPACT THREATENED OR ENDANGERED SPECIES. ARCHEOLOGICAL AND ENVIRONMENTAL CLEARANCES MUST BE OBTAINED BEFORE DISPOSAL.



SHEET: CS-101



GSE HD White/Gray Smooth Geomembrane

Nominal

GSE HD White/Gray is a smooth high density polyethylene (HDPE) geomembrane with a white/gray upper surface. It is manufactured with the highest quality resin specifically formulated for flexible geomembranes. GSE White/Gray has a UV stabilized upper white surface that reflects light, improves damage detection, and reduces wrinkles and sub-grade desiccation. This product is used in applications that require excellent chemical resistance and endurance properties.

Primary Liner 60 mil HDPE

(*)

AT THE CORE:

An HDPE geomembrane used in applications that require excellent chemical resistance and endurance properties.

Product Specifications

Tested Property	Test Method	Frequency	Nominal Value				
			30 mil	40 mil	60 mil	80 mil	100 mil
Thickness (minimum average), mil	ASTM D 5199	every roll	27	36	54	72	90
Density, g/cm ³	ASTM D 1505	200,000 lb	0.940	0.940	0.940	0.940	0.94
Tensile Properties (each direction)	ASTM D 6693, Type IV Dumbbell, 2 ipm G.L. 2.0 in G.L. 1.3 in	20,000 lb	114	152	228	304	380
Strength at Break, lb/in-width			63	84	126	168	210
Strength at Yield, lb/in-width			700	700	700	700	700
Elongation at Break, %			12	12	12	12	12
Elongation at Yield, %							
Tear Resistance, lb	ASTM D 1004	45,000 lb	21	28	42	56	70
Puncture Resistance, lb	ASTM D 4833	45,000 lb	54	72	108	144	180
Carbon Black Content, % (Range)	ASTM D 1603*/4218	20,000 lb	2.0 - 3.0	2.0 - 3.0	2.0 - 3.0	2.0 - 3.0	2.0 - 3.0
Carbon Black Dispersion	ASTM D 5596	45,000 lb	Note ⁽¹⁾	Note ⁽¹⁾	Note ⁽¹⁾	Note ⁽¹⁾	Note ⁽¹⁾
Notched Constant Tensile Load, hr	ASTM D 5397, Appendix	200,000 lb	300	300	300	300	300
Oxidative Induction Time, mins	ASTM D 3895, 200°C; O ₂ , 1 atm	200,000 lb	>100	>100	>100	>100	>100
TYPICAL ROLL DIMENSIONS							
Roll Length ⁽²⁾ , ft			1,240	940	630	470	370
Roll Width ⁽²⁾ , ft			22.5	22.5	22.5	22.5	22.5
Roll Area, ft ²			27,900	21,150	14,175	10,575	8,325

NOTES:

- ⁽¹⁾Dispersion only applies to near spherical agglomerates. 9 of 10 views shall be Category 1 or 2. No more than 1 view from Category 3.
- ⁽²⁾Roll lengths and widths have a tolerance of ±1%.
- GSE HD is available in rolls weighing approximately 3,900 lb.
- All GSE geomembranes have dimensional stability of ±2% when tested according to ASTM D 1204 and LTB of <-77°C when tested according to ASTM D 746.
- *Modified.

GSE is a leading manufacturer and marketer of geosynthetic lining products and services. We've built a reputation of reliability through our dedication to providing consistency of product, price and protection to our global customers.

Our commitment to innovation, our focus on quality and our industry expertise allow us the flexibility to collaborate with our clients to develop a custom, purpose-fit solution.

(DURABILITY RUNS DEEP)

For more information on this product and others, please visit us at GSEworld.com, call 800.435.2008 or contact your local sales office.

GSE
ENVIRONMENTAL™

GSE HD Smooth Geomembrane

Nominal

GSE HD is a smooth high density polyethylene (HDPE) geomembrane manufactured with the highest quality resin specifically formulated for flexible geomembranes. This product is used in applications that require excellent chemical resistance and endurance properties.



AT THE CORE:

An HDPE geomembrane used in applications that require excellent chemical resistance and endurance properties.

Secondary liner 40-mil HDPE

Product Specifications

Tested Property	Test Method	Frequency	Nominal Value					
			30 mil	40 mil	60 mil	80 mil	100 mil	120 mil
Thickness (minimum average), mil	ASTM D 5199	every roll	27	36	54	72	90	108
Density, g/cm ³	ASTM D 1505	200,000 lb	0.940	0.940	0.940	0.940	0.94	0.94
Tensile Properties (each direction)	ASTM D 6693, Type IV Dumbbell, 2 ipm G.L. 2.0 in G.L. 1.3 in	20,000 lb	114	152	228	304	380	456
Strength at Break, lb/in-width			63	84	126	168	210	252
Strength at Yield, lb/in-width			700	700	700	700	700	700
Elongation at Break, %			12	12	12	12	12	12
Elongation at Yield, %								
Tear Resistance, lb	ASTM D 1004	45,000 lb	21	28	42	56	70	84
Puncture Resistance, lb	ASTM D 4833	45,000 lb	54	72	108	144	180	216
Carbon Black Content, % (Range)	ASTM D 1603*/4218	20,000 lb	2.0 - 3.0	2.0 - 3.0	2.0 - 3.0	2.0 - 3.0	2.0 - 3.0	2.0-3.0
Carbon Black Dispersion	ASTM D 5596	45,000 lb	Note ⁽¹⁾	Note ⁽¹⁾	Note ⁽¹⁾	Note ⁽¹⁾	Note ⁽¹⁾	Note ⁽¹⁾
Notched Constant Tensile Load, hr	ASTM D 5397, Appendix	200,000 lb	500	500	500	500	500	500
Oxidative Induction Time, mins	ASTM D 3895, 200°C; O ₂ , 1 atm	200,000 lb	>100	>100	>100	>100	>100	>100
TYPICAL ROLL DIMENSIONS								
Roll Length ⁽²⁾ , ft			1,240	940	630	470	370	280
Roll Width ⁽²⁾ , ft			22.5	22.5	22.5	22.5	22.5	22.5
Roll Area, ft ²			27,900	21,150	14,175	10,575	8,325	6,300

NOTES:

- ⁽¹⁾Dispersion only applies to near spherical agglomerates. 9 of 10 views shall be Category 1 or 2. No more than 1 view from Category 3.
- ⁽²⁾Roll lengths and widths have a tolerance of ±1%.
- GSE HD is available in rolls weighing approximately 3,900 lb.
- All GSE geomembranes have dimensional stability of ±2% when tested according to ASTM D 1204 and LTB of <-77°C when tested according to ASTM D 746.
- *Modified.

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Our commitment to innovation, our focus on quality and our industry expertise allow us the flexibility to collaborate with our clients to develop a custom, purpose-fit solution.

[DURABILITY RUNS DEEP]

For more information on this product and others, please visit us at GSEworld.com, call 800.435.2008 or contact your local sales office.

GSE
ENVIRONMENTAL™

GSE HyperNet Geonet 150 - 225 mil

GSE HyperNet geonet is a synthetic drainage material manufactured from a premium grade high density polyethylene (HDPE) resin. The structure of the GSE HyperNet geonet is formed specifically to transmit fluids uniformly under a variety of field conditions. The geonet is formulated to be resistant to ultraviolet light for a period of time necessary to complete the installation.



AT THE CORE:

A synthetic geonet engineered specifically to transmit fluids consistently under a variety of field conditions.

Product Specifications

Tested Property	Test Method	Frequency	Minimum Average Roll Value			
			150 mil	175 mil	200 mil	225 mil
Geonet Thickness, mil	ASTM D 5199	1/50,000 ft ²	150	175	200	225
Transmissivity, gal/min/ft (m ² /sec)	ASTM D 4716	1/540,000 ft ²	15 (3.1 x 10 ⁻³) ⁽¹⁾	20 (4.1 x 10 ⁻³) ⁽¹⁾	9.6 (2 x 10 ⁻³) ⁽²⁾	12 (2.5x10 ⁻³) ⁽²⁾
Density, g/cm ³	ASTM D 1505	1/50,000 ft ²	0.94	0.94	0.94	0.94
Tensile Strength (MD), lb/in	ASTM D 5035/7179	1/50,000 ft ²	35	40	45	50
Carbon Black Content, %	ASTM D 4218	1/50,000 ft ²	2.0	2.0	2.0	2.0
NOMINAL ROLL DIMENSIONS						
Roll Width ⁽³⁾ , ft			14.5	15	15	15
Roll Length ⁽³⁾ , ft			400	350	330	290
Roll Area, ft ²			5,800	5,250	4,950	4,350

NOTES:

- ⁽¹⁾Gradient of 0.1, normal load of 1,000 psf, water at 70° F, between steel plates for 15 minutes. Contact GSE for performance transmissivity value for use in design.
- ⁽²⁾Gradient of 0.1, normal load of 10,000 psf, water at 70° F, between steel plates for 15 minutes. Contact GSE for performance transmissivity value for use in design.
- ⁽³⁾Roll widths and lengths have a tolerance of ±1%

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[DURABILITY RUNS DEEP] For more information on this product and others, please visit us at GSEworld.com, call 800.435.2008 or contact your local sales office.



Appendix A

Liner Installation Manual

R.T. Hicks Consultants, Ltd.

901 Rio Grande Blvd. NW, Suite F-142
Albuquerque, NM 87104

Installation Quality Assurance Manual



Geomembrane Products





Table of Contents

1.0	Introduction.....	1
2.0	Standard Test Methods.....	1
3.0	Material Delivery.....	1
4.0	Earthwork.....	2
5.0	Panel Placement.....	2
6.0	Trial Welds.....	3
7.0	Geomembrane Field Seaming.....	5
8.0	Field Destructive Testing.....	6
9.0	Non-Destructive Testing.....	7
10.0	Defects & Repairs.....	8
11.0	Repair Procedures.....	8
12.0	As-Built Drawings.....	9
	Appendix A: Inventory Check List Form.....	10
	Appendix B: Subgrade Surface Acceptance Form.....	11
	Appendix C: Panel Placement Log Form.....	12
	Appendix D: HDPE & LLDPE Seam Strength Properties.....	13
	Appendix E: Trial Weld Log Form.....	14
	Appendix F: Seam Log Form.....	15
	Appendix G: Destructive Test Log Form.....	16
	Appendix H: Repair Log - Vacuum Test Form.....	17
	Appendix I: Non-Destructive Log - Air Test Form.....	18



1.0 INTRODUCTION

This manual provides an overview of the GSE Installation Quality Assurance procedures consistent with industry accepted practices to ensure that the geomembrane products installed will perform for its intended purpose. In addition, all installation work will be performed in strict accordance per the customer's specifications. Please read the procedures below completely before you begin. If you need further clarification, contact GSE Engineering Support Staff for assistance. Remember safety first and use safe practices always on every project.

2.0 STANDARD TEST METHODS

ASTM D 6392: Standard Test Methods For Determining The Integrity Of Non-Reinforced Geomembrane Seams Produced Using Thermo Fusion Methods

ASTM D 5820: Standard Practice For Pressurized Air Channel Evaluation of Dual Seamed Geomembranes

ASTM D 5641: Standard Practice For Geomembrane Seam Evaluation By Vacuum Chamber

ASTM D 6497: Standard Guide For Mechanical Attachment of Geomembrane to Penetrations or Structures

ASTM D 7240: Standard Practice for Leak Location using Geomembranes with an Insulating Layer in Intimate Contact with a Conductive Layer via Electrical Capacitance Technique (Conductive Geomembrane Spark Test)

GRI Standard GM13: Test Properties, Testing Frequency and Recommended Warranty for High Density Polyethylene (HDPE) Smooth and Textured Geomembranes

GRI Standard GM14: Selecting Variable Intervals for Taking Geomembrane Destructive Seam Samples Using the Method of Attributes

GRI Standard GM17: Test Properties, Testing Frequency and Recommended Warranty for Linear Low Density Polyethylene (LLDPE) Smooth and Textured Geomembranes

GRI Standard GM19: Standard Specification for Seam Strength and Related Properties of Thermally Bonded Polyolefin Geomembranes

3.0 MATERIAL DELIVERY

- A. Upon arrival on site, the QA personnel will inventory all materials on the job site.
- B. Roll numbers of geomembrane will be logged on the Inventory Check List (Appendix A) and cross-referenced with the Bill of Lading for materials supplied by GSE.
- C. Copies of the Inventory Check List and signed Bill of Lading should be sent to the GSE's corporate headquarters while the QA personnel retains the original copies.



- D. Any visible damage to roll materials should be noted on the roll and Inventory Check List.

4.0 EARTHWORK

- A. The general contractor is responsible for preparing and maintaining the subgrade. The subgrade should be prepared and maintained per the job specifications.
- B. The site manager shall be responsible for assuring that the subgrade surface has been properly prepared for deployment of geosynthetics. After each day's deployment the Subgrade Surface Acceptance form (Appendix B) will be signed by all parties.

5.0 PANEL PLACEMENT

- A. Each panel will be assigned a number as described below.
1. When there is one layer, panels may be designated with only a number, i.e... 1, 2, 3, 4 etc.
 2. When two or more layers are required, use a letter and number, i.e....
Primary Liner P1, P2, P3, P4 etc...
Secondary Liner S1, S2, S3, S4 etc...
Tertiary Liner T1, T2, T3, T4 etc...
- B. This numbering system should be used whenever possible. Agreement to a panel numbering system should be made at the pre-construction meeting. However, it is essential that the installer, the owner representative and third party QA inspector agree.
- C. Panel numbers shall be written in large block letters in the center of each deployed panel. The roll number, date of deployment and length (gross) should be noted below the panel number. All notes should be made, so that they are easily visible from a distance. On long panels it is beneficial to write information on both ends.
- D. Panel numbers shall be logged on the Panel Placement Log (Appendix C) along with the roll number and other information necessary to complete the form.
- E. If there is a partial roll left after deployment, it is important to write the last four digits of the roll number in several locations on the roll along with the estimated length for future identification.
- F. Deployment of geomembrane panels shall be performed in a manner that will comply with the following guidelines:
1. Unroll geomembrane using methods that will not damage geomembrane and will protect underlying surface from damage. GSE Conductive should be installed with Conductive layer facing down.
 2. Place temporary ballast, such as sangbags, on geomembrane that will not damage the geomembrane and to prevent wind uplift.
 3. Personnel walking on geomembrane shall not engage in activities or wear shoes that could

damage it. Smoking is not permitted on the geomembrane.

4. Do not allow heavy vehicular traffic directly on geomembrane. Rubber tired and tracked ATV's and equipment are acceptable if contact pressure is less than 8 psi.
 - a. Protect geomembrane in areas of heavy traffic by placing protective cover over the geomembrane.
 - b. Prior to driving on any geomembrane layer, please check for sharp edges, embedded rocks, or other foreign objects that may protrude in the tires and tracks.
 - c. Path driven on geomembranes shall be as straight as possible with no sharp turns, sudden stops or quick starts.
 - d. Areas where driving occurs shall be continuously and thoroughly inspected throughout the deployment process by the contractor and the third party CQA.

6.0 TRIAL WELDS

- A. Seaming apparatus shall be allowed to warm up a minimum of 10 minutes before performing trial welds.
- B. Each seaming apparatus along with a welding technician will pass a trial weld prior to use. Trial welds to be performed in the morning and afternoon, as a minimum, as well as whenever there is a power shutdown.
- C. Fusion or wedge welds will always be performed or conducted on samples at least 6.0 ft long. Extrusion welds will be done on samples at least 3.0 ft long.

Note: Always perform trial welds in the same conditions that exist on the job. Run the trial welds on the ground, not the installed liner. Do not use a wind break unless you are using one on the job.

- D. Operating temperatures should be monitored while welding. The welding technician should verify that the equipment is capable of maintaining temperature while welding.
- E. Sampling Procedure
 1. Cut five 1.0 in wide specimens from the trial weld sample. Specimens will always be cut using a 1.0 in die cutter, so the peel values may be used for qualitative analysis.
 2. When cutting coupons from the trial weld samples, the inside and outside tracks on the coupon should be identified to assist in troubleshooting problems in case the weld fails. The outside track will be defined as the track, which would be peeled if pulling the overlap exposed in a typical installation, or the seam that is closest to the edge of the top sheet. The inside track is the seam closest to the edge of the bottom sheet.

F. Cutter

1. Only cut one sample at a time to avoid damaging the die cutter.
2. Samples should be free of sand and grit prior to cutting sample.
3. Inspect the die edge weekly for nicks, dents or signs of dullness. Dullness of the cutting edge may damage the units.
4. Remove die when edge has been dulled and lightly reshape it with a medium hand file. When wear is excessive return it for a replacement die.
5. When the cutting board becomes deeply scored and/or interferes with coupon cutting it should be replaced.
6. To adjust the depth of the die cut into the cutting board, after replacing the cutting board or sharpening the die, 0.015 in washer shims can be added or removed between the cutting ram and the ram extension. Only add shims when cutting is difficult due to lack of depth of cut.

G. Trial Weld Testing

1. Allow coupons to cool prior to testing. Avoid separating the coupons while hot as failure of the sheet may be initiated and false readings indicated.
2. In extreme heat the coupons may need to be cooled, using water or an insulated cooler prior to peel testing. Lab conditions specify 70 degrees (plus or minus 4 degrees) Fahrenheit. Coupon temperatures greater than 70 degrees may result in lowered strengths.
3. Visually inspect the coupons for squeeze-out, footprint, pressure and general appearance.
4. Each of the five coupons will be tested in peel on the field tensiometer at a separation rate of 2 in per minute (for HDPE). Shear tests, in addition to the peel tests, will be performed.

H. Pass/Fail Criteria

1. Criteria for passing trial welds will be as follows:
 - a. Seam must exhibit film tear bond (FTB). Trial welds should have no incursion into the weld.
 - b. Peel and shear values shall meet or exceed the values as listed in Appendix D, Table 1 for HDPE smooth or textured sheet (@ 2 in/min).
 - c. Peel and shear values shall meet or exceed the values as listed in Appendix D, Table 2 for LLDPE smooth or textured sheet (@ 20 in/min).
 - d. Both tracks of fusion welded samples must pass for the trial weld to be considered acceptable. If any of the five coupons fail due to seam incursion (no FTB) or low strength values, the trial weld must be performed again.



- e. The QA personnel will give approval to proceed with welding after observing and recording all trial welds.
2. All trial weld data will be logged on the Trial Weld Log (Appendix E).
3. When logging fusion welded peel values on the Trial Weld Log indicate the values. for the outside track first, followed by the inside track.
4. Speed and temperature settings will be recorded for each machine trial weld as appropriate.

7.0 GEOMEMBRANE FIELD SEAMING

- A. The seam number takes the identity of the panels on each side. The seam between panels 1 & 2 becomes seam 1/2.
- B. Welding technicians will record their initials, machine number, date and time at the start of every seam and on the Seam Log (Appendix F). The technician should also periodically mark temperatures along the seam and at the end of the seam.
- C. Approved processes for field seaming and repairing are fusion welding and extrusion welding. All welding equipment shall have accurate temperature monitoring devices installed and working to ensure proper measurement.
- D. Fusion welding shall be used for seaming panels together and is not used for patching or detail work. The site manager shall verify that:
 1. The equipment used is functioning properly.
 2. All work is performed on clean surfaces and done in a professional manner. No seaming will be performed in adverse weather conditions.
- E. Extrusion welding shall be used primarily for repairs, patching and special detail fabricating and may be used for seaming. The site manager shall verify that:
 1. Equipment used is functioning properly.
 2. Welding personnel are purging the extrusion welders of heat degraded extrudate prior to actual use.
 3. All work is performed on clean surfaces and done in a professional manner. No seaming will be performed in adverse weather conditions.
- F. For seam preparation, the welding technician shall verify that:
 1. Prior to seaming, the seaming area is free of moisture, dust, dirt, sand or debris of any nature.
 2. The seam is overlapped properly for fusion welding.
 3. The seam is overlapped or extended beyond damaged areas at least 4.0 in when extrusion welding.



4. The seam is properly heat tacked and abraded prior to extrusion welding.
5. Seams are welded with fewest number of unmatched wrinkles or "fishmouths".
- G. No seaming will be performed in ambient air temperatures or adverse weather conditions that would jeopardize the integrity of the liner installation.

8.0 FIELD DESTRUCTIVE TESTING

- A. Destructive seam tests shall be performed to evaluate bonded seam strength. The frequency of sample removal shall be one sample per 500 ft of seam, unless site specifications differ. Location of the destructive samples will be selected and marked by the QA technician or third party QA inspector. Field testing should take place as soon as possible after seam is completed.
- B. Samples should be labeled in numerical order, i.e. DS-1, DS-2 etc....This should carry thru any layer and or multiple ponds, do not start numbering from 1 again. The size of samples and distribution should be approximately 12 in x 39 in (Size may vary depending on job requirements) and distributed as follows:
 1. 12 in x 12 in piece given to QA technician for field testing.
 2. 12 in x 12 in piece sent to the GSE's corporate headquarters for testing, if required.
 3. 12 in x 12 in piece given to third party for independent testing or to archive.

NOTE: All samples will be labeled showing test number, seam number, machine number, job number, date welded and welding tech number.

- C. The sample given to the QA technician in the field shall have ten coupons cut and be tested with a tensiometer adjusted to a pull rate as shown below. The strength of four out of five specimens should meet or exceed the values below, and the fifth specimen must meet or exceed 80% of the value below.
 1. Seam must exhibit film tear bond (FTB). Welds should have $\leq 25\%$ incursion into the weld.
 2. Peel and shear values shall meet or exceed the values as listed in Appendix D, Table 1 for HDPE smooth or textured sheet (@ 2 in/min).
 3. Peel and shear values shall meet or exceed the values as listed in Appendix D, Table 2 for LLDPE smooth or textured sheet (@ 20 in/min).
- D. All weld destructive test data will be logged on the Destructive Test Log (Appendix G).
- E. When logging fusion welded peel values on the Destructive Test Log, indicate the values for the outside track first, followed by the inside track.
- F. Test results will be noted in the Destructive Test Log as Pass (P) or Fail (F).

- G. If a test fails, additional samples will be cut, approximately 10 ft on each side of the failed test, and retested. These will be labeled A (After) & B (Before). This procedure will repeat itself until a sample passes. Then the area of failed seam between the two tests that pass will be capped or reconstructed.

9.0 NON-DESTRUCTIVE TESTING

- A. All seams shall be non-destructively tested over their full length using an air pressure or vacuum test. The purpose of this test is to check the continuity of the seam.
- B. For air pressure testing, the following procedures are applicable to those seams welded with a double seam fusion welder.
1. The equipment used shall consist of an air tank or pump capable of producing a minimum 35 psi and a sharp needle with a pressure gauge attached to insert into the air chamber.
 2. Seal both ends of the seam by heating and squeezing them together. Insert the needle with the gauge into the air channel. Pressurize the air channel to 30 psi. Note time test starts and wait a minimum of 5 minutes to check. If pressure after five minutes has dropped less than 2 psi then the test is successful (Thickness of material may cause variance).
 3. Cut opposite seam end and listen for pressure release to verify full seam has been tested.
 4. If the test fails, follow these procedures.
 - a. While channel is under pressure walk the length of the seam listening for a leak.
 - b. While channel is under pressure apply a soapy solution to the seam edge and look for bubbles formed by air escaping.
 - c. Re-test the seam in smaller increments until the leak is found.
 5. Once the leak is found using one of the procedures above, cut out the area and retest the portions of the seams between the leak areas per 4a to 4b above. Continue this procedure until all sections of the seam pass the pressure test.
 6. Repair the leak with a patch and vacuum test.
- C. For vacuum testing, the following procedures are applicable to those seams welded with an extrusion welder.
1. The equipment used shall consist of a vacuum pumping device, a vacuum box and a foaming agent in solution.
 2. Wet a section with the foaming agent, place vacuum box over wetted area. Evacuate air from the vacuum box to a pressure suitable to affect a seal between the box and geomembrane. Observe the seam through the viewing window for the presence of soap bubbles emitting from the seam.
 3. If no bubbles are observed, move box to the next area for testing. If bubbles are observed, mark the area of the leak for repair per section 11.0 and re-test per section 9.0.

Note: If vacuum testing fusion welded seams, the overlap flap must be cut off to perform the tests

4. All non-destructive tests will be noted in the Non-Destructive Logs (Appendixes H-I).
- D. For spark testing GSE Conductive geomembranes, ASTM D 7240 will be the procedure, unless otherwise instructed by the engineer client.

10.0 DEFECTS & REPAIRS

- A. All seams and non-seam areas of the geomembrane lining system shall be examined for defects.
- B. Identification of the defect should be made using the following procedures:
 1. For any defect in the seam or sheet that is an actual breach (hole) in the liner, installation personnel shall circle the defect and mark with the letter P along side the circle. The letter P indicates a patch is required.
 2. For any defect that is not an actual hole, installation personnel shall circle the defect indicating that the repair method may be only an extruded bead and that a patch is not required.
 3. Each suspect area that has been identified as repair shall be repaired in accordance with section 11.0 and in the non-destructively testing per section 9.0. After all work is completed, the site manager will conduct a final walk-through to confirm all repairs have been completed and debris removed. Only after this final evaluation by the site manager, the owner, and the agent shall any material be placed over the installed liner.

11.0 REPAIR PROCEDURES

- A. Any portion of the geomembrane lining system exhibiting a defect that has been marked for repair may be repaired with any one or combination of the following procedures:
 1. Patching - used to repair holes, tears, undispersed raw materials in the sheet.
 2. Grind and Reweld - used to repair small sections of extrusion welded seams.
 3. Spot Welding - Used to repair small minor, localized flaws.
 4. Flap Welding - Used to extrusion weld the flap of a fusion weld in lieu of a full cap.
 5. Capping - Used to repair failed seams.
- B. The following conditions shall apply to the above methods:
 1. Surfaces of the geomembrane which are to be repaired shall be prepared according to this section.
 2. All surfaces must be clean and dry at the time of the repair.
 3. All seaming equipment used in repairing procedures shall be qualified.



4. All patches and caps shall extend at least 4 in beyond the edge of the defect, and all patches must have rounded corners.
5. All cut out holes in liner must have rounded corners of 3.0 in minimum radius.
- C. Patches should be labeled in numerical order, i.e. RP-1, RP-2, etc... This should carry through any layer and/or multiple ponds, and do not start with the number 1 again.

12.0 AS-BUILT DRAWINGS

The installer shall provide the following:

- A. As-built drawings will be provided at the completion of the project.
- B. As-built drawings will include geomembrane panels and panel numbers with the last four digits of the roll number.
- C. Panel numbers and the full roll numbers will correspond with the Panel Placement Log(Appendix C).
- D. All destructive testing and repair locations will be placed on the as-built drawings.



Appendix A

[illegible]



Appendix B

Subgrade Surface Acceptance

Date: _____

Project: _____ Site Manager: _____

Project #: _____

Location: _____ Partial: _____ Final: _____

This document only applies to the acceptability of surface conditions for installation of geosynthetic products. GSE does not accept responsibility for compaction, elevation or moisture content, nor for the surface maintenance during deployment. Structural integrity of the subgrade and maintenance of these conditions are the responsibility of the owner or earthwork contractor.

For GSE Lining Technology, LLC:

For Owner / Contractor:

Acceptance Number: _____ Area Accepted: _____ s.f. Total Area Accepted to date: _____ s.f.



Appendix C

Panel Placement Log

Project Name: _____

Location: _____ Site Supervisor: _____

Job Number: _____ Type of Material: _____

Q.A. Tech.: _____ Sheet Thickness: _____

Panel Number	Roll Number	Deployment Date	Width (Feet)	Length (Feet)	Square Feet	Square Feet (Cumulative)	A/T 1	A/T 2



Appendix D

Table 1. HDPE Seam Strength Properties

Material (Mil)	Shear Strength (PPI)	Fusion Peel (PPI)	Extrusion Peel (PPI)
40	81	65	52
60	121	98	78
80	162	130	104
100	203	162	130

Table 2. LLDPE Seam Strength Properties

Material (Mil)	Shear Strength (PPI)	Fusion Peel (PPI)	Extrusion Peel (PPI)
40	60	50	48
60	90	75	72
80	120	100	96
100	150	125	120



Trial Weld Log

[illegible]



Appendix F

Seam Log

Project Name: _____

Location: _____ Site Supervisor: _____

Job Number: _____ Type of Material: _____

Q.A. Tech.: _____ Sheet Thickness: _____

Seam Number	Time of Weld	Date of Weld	Type of Weld	Length of Seam	Machine Number	Technician ID Number



Destructive Test Log

Project Name:	Site Supervisor:	Extrusion (psi)	
Location:	Type of Material:	Min. Peel	Min. Peel
Job Number:	Sheet Thickness:	Min. Shear	Min. Shear
Q.A. Tech.:			

[illegible]



Appendix H

Repair Log - Vacuum Test

Project Name: _____

Location: _____ Site Supervisor: _____

Job Number: _____ Type of Material: _____

Q.A. Tech.: _____ Sheet Thickness: _____

Repair Number	Weld Date	Machine Number	Tech ID	Location	Test Date	Tech ID	Pass/Fail

IQA GEO 803/22/11



Appendix I

Non-Destructive Log - Air Test

Project Name: _____

Location: _____ Site Supervisor: _____

Job Number: _____ Type of Material: _____

Q.A. Tech.: _____ Sheet Thickness: _____

Seam Number	Test Date	Technician ID Number	Air Pressure Test		Test Result (P or F)	Location
			psi start	psi finish		

Appendix A

Avian Protection Program **Variance**

R.T. Hicks Consultants, Ltd.

901 Rio Grande Blvd. NW, Suite F-142
Albuquerque, NM 87104

AVIAN PROTECTION PROGRAM FOR PRODUCED WATER CONTAINMENTS

Variance

The operator will install and use the Bird-X Mega Blaster Pro as a primary hazing program for avian species. In addition to this sonic device, staff will routinely inspect the containment at the start or end of the workday for the presence of avian species and, if detected, will use a blank cartridge or shell in a handgun, starter pistol or shotgun as additional hazing. Decoys of birds of prey are placed on the game fence and other roosts around the open water to provide additional hazing.

Wetlands and freshwater lakes are within 3-miles of the proposed containment (see Figure 6 of the application). The presence of these natural waterbodies (Dagger Lake to the southwest typically holds water) provides an better source of water, which may be the most effective decoy to cause avian species to avoid the containment.

EFFECTIVE WIDE-AREA BIRD CONTROL!

Mega Blaster PRO sonic bird repeller covers 30 acres!



Mega Blaster PRO uses intermittent distress calls to create a "danger zone" that frightens infesting birds away for good.

PREDATOR cries help scare all the birds.



- **NEMA Rated Case**
- **Crystal-Clear Digital Sounds**

- *Laughing Gull*
- *Ring-Billed Gull*
- *Herring Gull*
- *California Gull*
- *Black-Headed Gull*
- *Glaucous-Winged Gull*
- *Double Crested Cormorant*
- *Marsh Hawk*

**Perfect for Landfills, Airfields, Fish Farms,
Farm Fields or any multi-acre facility.**

Our most powerful system features two high-output amplifiers that drive our specially-designed 20 speaker tower. The intense sound output covers up to 30 acres (12 hectares).

It features solid-state electronics mounted inside a NEMA-type control box, suitable for most any application.

The generating unit mounts easily to a post or pole using the included hardware. The unit comes pre-recorded in four different configurations for the most common bird infestations.

Choose any or all of the 8 sounds, including predators to give the birds even more of a sense of danger. Customize by choosing volume and silent time between sounds.

Mega Blaster PRO

Complete system includes the generating unit with two built-in high-output amplifiers, 20-speaker tower with audio cables, 40 watt solar panel, battery clips and all mounting hardware.

CONFIGURATIONS AVAILABLE:

- Agricultural
MEGA-AG
- Crow / Raven
MEGA-CROW
- Woodpecker
MEGA-WP
- Marine / Gull
MEGA-MAR



The Bird Control 'X'-Perts

NOTE: This unit is capable of sound output up to 125 decibels. **HEARING PROTECTION IS RECOMMENDED.**

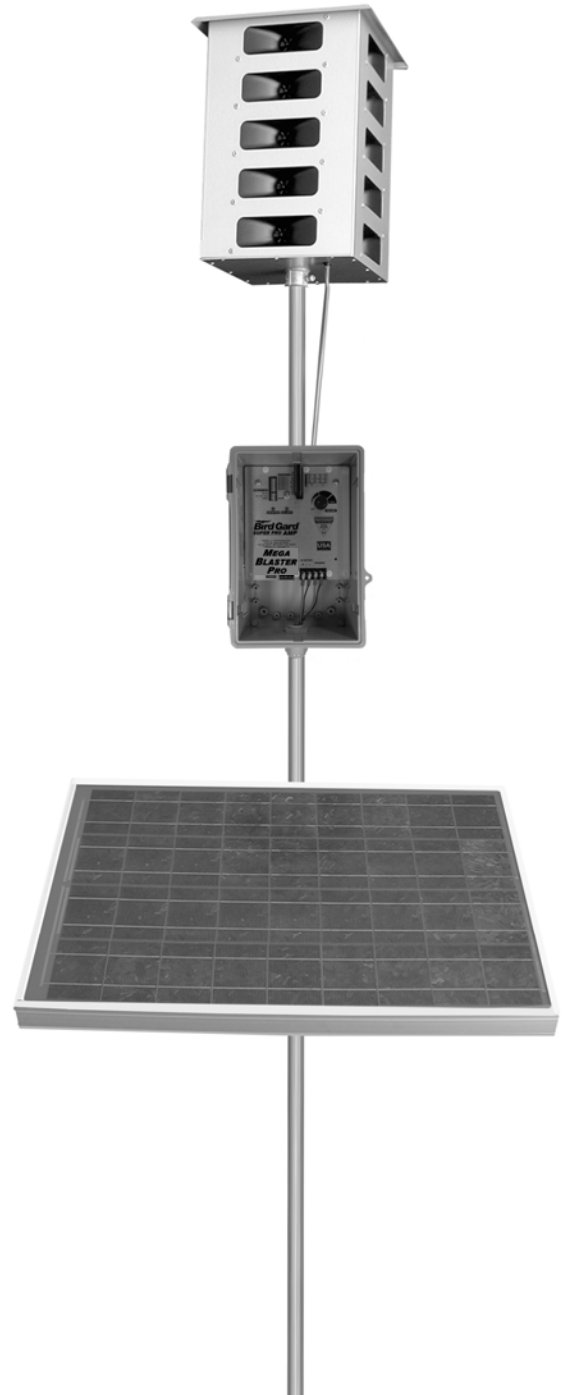


MEGA BLASTER PRO



User's Manual

Overview	2
Bird Control Management Guidelines	3
Materials List	4
Assembly	5
Control Unit	5
Solar Panel	5
Placement	6
Building a Mounting Pole or Mast	7
Installation	8
20-Speaker Tower	8
Solar Panel	8
Control Box	9
Solar Panel Connections	9
Settings	10
Recordings	10
Mode Settings	10
Warranty	12



Overview

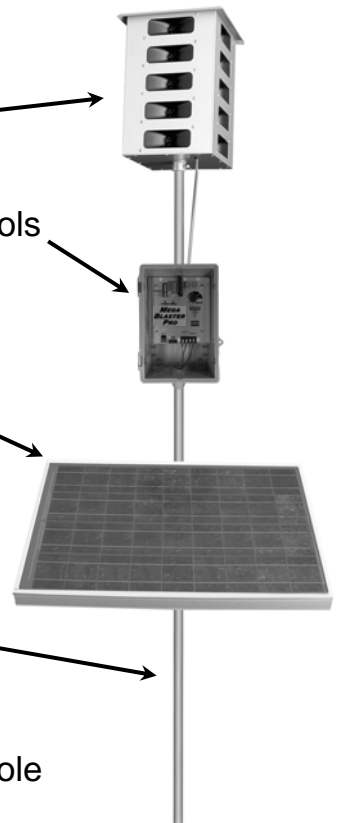
The Bird-X Mega Blaster Pro utilizes the innate power of the natural survival instincts of birds to effectively repel them. Digital recordings of distressed and alarmed birds, along with the sounds made by their natural predators are broadcast through high fidelity weather-resistant speakers over the top of areas. This action triggers a primal fear and flee response. Pest birds soon relocate to where they can feed without feeling threatened.

Your Bird-X Mega Blaster Pro system consists of:

20-Speaker Tower broadcasts the bird sounds

Control Unit produces the bird sounds and contains all operational controls

Solar Panel recharges the 12-volt deep cycle battery



Items needed but not included:

- (1) **Mounting Pole or Mast** tall enough to raise the 20-Speaker Tower at least 5 feet above the top of the areas, trees or other obstructions
- (1) **12-volt Deep Cycle Battery** (RV/Marine) Group 27 or larger wet cell
- (1) **T-Post** or similar (Optional) may be needed to support the mounting pole
- (1) **Bailing Wire or zip-tie** (Optional) to secure the Mounting Pole to the T-Post

CAUTION: THE MEGA BLASTER PRO IS CAPABLE OF PRODUCING SOUNDS UP TO 125 DECIBELS. PROPER HEARING PROTECTION MUST BE WORN ANYTIME THE UNIT IS TURNED ON.



Bird Control Management Guidelines







An active bird control management program is a key to successfully repelling pest birds. Bird feeding patterns may take several days or weeks to break. Follow all suggestions for maximum effectiveness. Read all instructions prior to installation.

For best results:

- **It is extremely important to fully protect your entire area from birds.** Any areas not fully protected will allow birds to begin feeding at the fringes of the sound coverage. They will soon become bolder and learn the sounds are nothing to fear. This will cause the effectiveness to diminish. Complete Bird-X product coverage forces birds to leave the area entirely.
- Install the Mega Blaster Pro unit at least two weeks before birds are attracted to your area. It is much easier to keep birds away before they have found a food source than it is to repel them once they have developed a feeding pattern.
- Most birds begin feeding from the perimeter of an area. Place Mega Blaster Pro units so the sound protection covers past the edges of the area.
- Birds will often use tall trees for roosting and observation. If birds are in bordering trees it is necessary to position the units so the sound protection covers the trees as well.
- Mount the 20-Speaker Tower at least five feet above trees, areas and structures for maximum coverage. The higher the better. Sound will disperse or reflect off structures or foliage. Mount control unit out of direct sun, if possible.
- When first installed, run Mega Blaster Pro units at FULL volume and on SHORT time off periods. This ensures maximum “bird stress” and creates a hostile environment.
- Watch for changes in bird activity and adjust the location of your Mega Blaster Pro unit if needed.
- **Check the battery and unit settings often to insure continuous bird control. Be certain that the system is not turned down or has a dead battery. Field hands or harvesters may turn down the volume.**
- Changing settings and switches often helps to prevent bird habituation. Periodically change the switch settings of the eight sounds (turning them ON or OFF). NEVER turn OFF the distress calls of the target birds you are trying to repel and always keep at least one predator bird sound turned ON.
- If different bird species enter the protected area and begin causing damage contact us immediately for an updated Sound Recording Card designed to repel the new invading birds.
- Remember that the Mega Blaster Pro system is a management tool, and should be used as part of your overall bird control strategy, sometimes in conjunction with other bird control techniques and devices.

Be aware that under extreme drought or other adverse conditions, birds will disregard all deterrents and risks in order to survive

Materials List

Item	Qty		Notes
Mega Blaster Pro Control Box	1		
Sound Recording Card	1		Pre-installed in control box
20-Speaker Tower	1		
Control Box Mounting U-Bolts	2		1/4" x 1" x 2"
Control Box Brackets	2		
40-Watt Solar Panel	1		
Solar Panel Mounting Bracket	1		
Solar Panel Mounting U-Bolts	2		1/4" x 1-1/8" x 2"
Control Box Connector Cable	1		2 Wire, 4 ft. Long
Battery Box	1		

Assembly

Note: You will find it easier to pre-assemble the following components prior to installation in the field.

Control Unit

1. Lay the Control Unit face down
2. Attach the two Control Box Mounting Brackets to the back with the included screws (Figure 1)

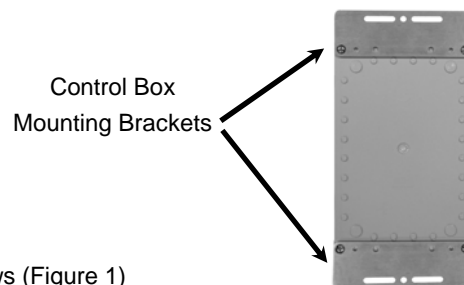


Figure 1

Solar Panel

3. Install the two Solar Panel Mounting U-Bolts in the Head of the Solar Panel Mounting Bracket (Figure 2)
4. Loosen, but do not remove the Carriage Bolts securing the movable Clamp Plates on the Solar Panel Mount Bracket
5. Lay the solar panel on a flat surface with the glass side down
6. Lay the Mounting Arm across the Solar Panel with the Clamp Plates down. Position the Mounting Arm at an angle so the Clamp Plates slide under the lip of the Solar Panel (Figure 3A)
7. Rotate the Mounting Arm and secure it to the Solar Panel by tightening the Carriage Bolts (Figure 3B)

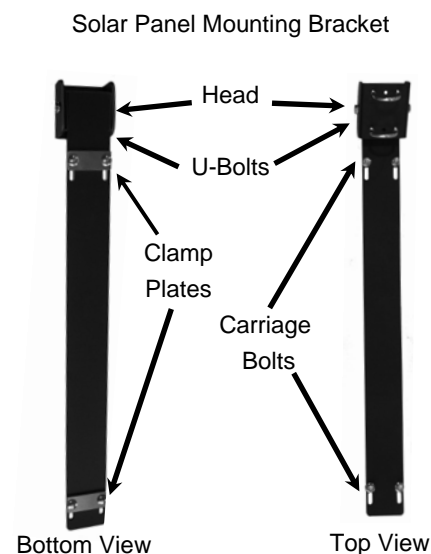


Figure 2

Clamp Plates slide under the lip of the Solar Panel

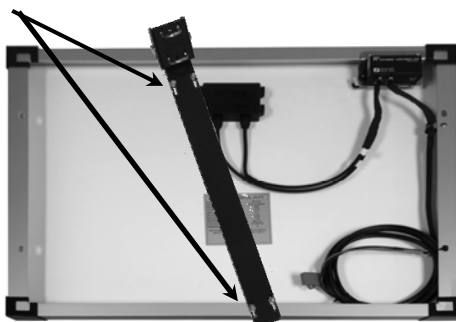


Figure 3A

Rotate Mounting Arm and tighten Carriage Bolts



Figure 3B

Placement

Your Mega Blaster Pro will protect an area up to approximately 600 feet in all directions.

Factors to consider when selecting the best location include:

- Birds typically feed from the perimeter of the area and work their way in. Place Mega Blaster Pro units so the sound protection covers all the way to the edges of the area. For larger areas Mega Blaster Pro units should be positioned 400-500 feet inside the area and spaced every 1,200 feet.
- Mount the 20-Speaker Tower at least 5 feet above terrain, areas, trees and other obstacles.
- Placing the Mega Blaster Pro on top of a hill or small rise will give you much better coverage than at the bottom of a valley. The greater the height the further the sounds will travel.
- Wind can blow the sound waves. If the area you need to protect has consistent wind coming from the same direction, position your Mega Blaster Pro more “upwind.”
- Trees surrounding areas provide birds with a safe perch that allows them to fly in, grab food and fly out. It is much more difficult to eliminate bird damage if the birds are able to use the surrounding trees as a staging area for attacks on your areas. Your Mega Blaster Pro unit should be positioned close to any trees bordering your areas. If birds are roosting in the trees at night the TIME OF OPERATION should be set to 24 HOUR.
- Lakes, rivers and wetlands are a favorite resting and hiding place for birds. Your Mega Blaster Pro unit should be placed so the sound thoroughly covers any areas where birds frequent.
- Neighbors, businesses and others may not appreciate hearing the bird sounds. At the limits of the effective range the sounds from your Mega Blaster Pro are at a level people may find annoying. Avoid placing the unit where it becomes a nuisance.

Building a Mounting Pole or Mast

CAUTION: TALL POLES AND MASTS CAN BE HEAVY AND POTENTIALLY DANGEROUS. USE EXTREME CAUTION WHEN CONSTRUCTING OR WORKING AROUND TALL POLES AND MASTS. BIRD-X, INC., ASSUMES NO RESPONSIBILITY FOR DAMAGES OR INJURIES.

Things to consider:

- The 20-Speaker Tower is designed to mount onto a 1 in. (outside diameter) pipe at least 14 in. long. 1 in. conduit works well as it is light, rigid, inexpensive and available in 10 ft. lengths making it ideal for low areas, vineyards and bushes.
- You will want to take down your Mega Blaster Pro unit after harvest and store it in a dry location until the next season.

A suggestion for masts up to 20 feet tall:

1. 3/4 inch Galvanized steel water pipe has a 1 inch outside diameter and is the correct size to fit inside the 20-Speaker Tower. It is often available in 20 ft. lengths from hardware and plumbing supply stores. If these are not available, 10 ft. lengths are common and can be fastened together with a threaded coupler. Assemble the poles on the ground.
2. Slide the 20-Speaker Tower over the pipe and tighten the set screw in the collar at the base.
3. Stand the pole assembly up just inside the drip line of a tree and securely tie the pole to a few heavy branches.
4. Drive a T-Post into the ground at the base of the pole and secure with wire.

For masts taller than 20 feet:

1. Use 20 ft. lengths of galvanized steel water pipe or similar, securely fastened together with threaded reducing couplers.
2. Starting with 3 in. pipe, step the size down with each length of pipe.
3. The last 10 ft. can be 1 in. (O.D.) conduit hose clamped to the final section of galvanized pipe.

A semi-permanent mast support can be made by digging a hole 4 ft. deep and 4 ft. round. In the middle of the hole sink a length of galvanized water pipe large enough that your mast will easily fit inside. Make sure at least 2 ft. of pipe is above ground level. Fill the area around the pipe with packed sand, leaving the last foot filled with concrete to form a cap over the hole. Your mast can be dropped into the galvanized water pipe “receiver” for support. At the end of harvest the mast can be lifted out and positioned on the ground for easy disassembly and storage.

Installation

Note: Foliage, trees, and other obstructions severely reduce the effective range of Mega Blaster Pro units. It is critical that the 20-Speaker Tower is mounted at least 5 feet above all obstructions to achieve the maximum protection.

Mounting Pole or Mast

1. The Mounting Pole or Mast will need to be supported by a T-Post, fence post, tree or other means. The Pole Support should be in place before proceeding.

20-Speaker Tower

2. Lay the 20-Speaker Tower on its side on the ground and cut the zip-tie securing the speaker cables.
3. Slide the 1 in. (outside diameter) Mounting Pole through the Collar at the bottom of the 20-Speaker Tower until it slides over the positioning bolt inside the top of the Tower (Figure 4).
4. Tighten the Set Screw in Collar securely.

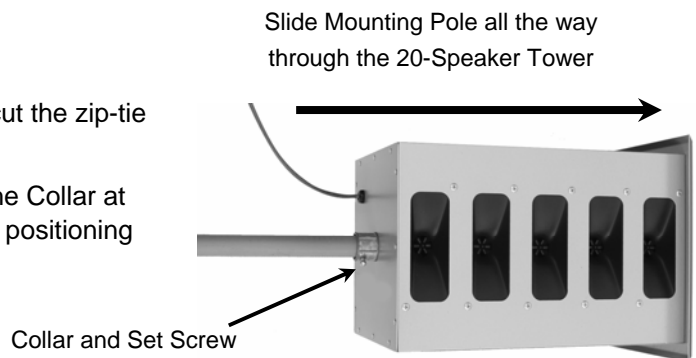


Figure 4

Solar Panel

5. Rest the lower end of the Mounting Pole on the Solar Panel Mounting Bracket approximately three feet from the bottom of the pole with the top of the solar panel facing the 20-Speaker Tower (Figure 5).
6. Lean up the Mounting Pole with the 20-Speaker Tower on top, against the Pole Support and fasten the Mounting Pole to the Pole Support securely with wire or other semi-permanent means.
7. Rotate the solar panel so it receives sunlight.

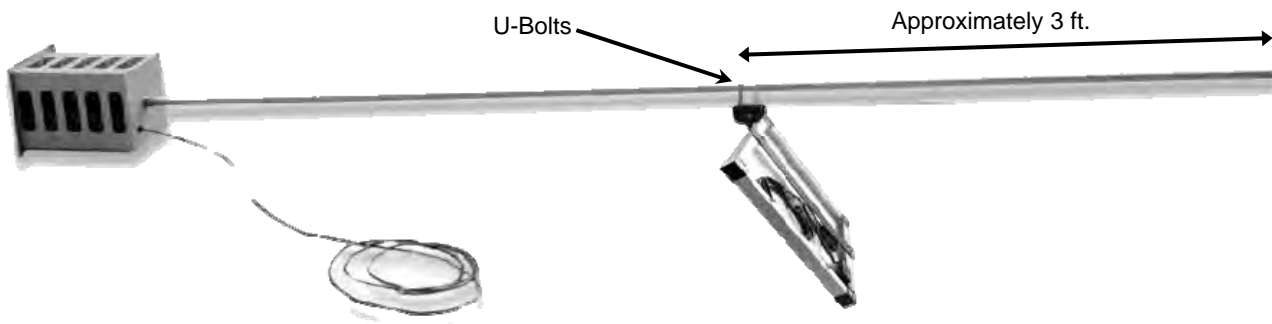
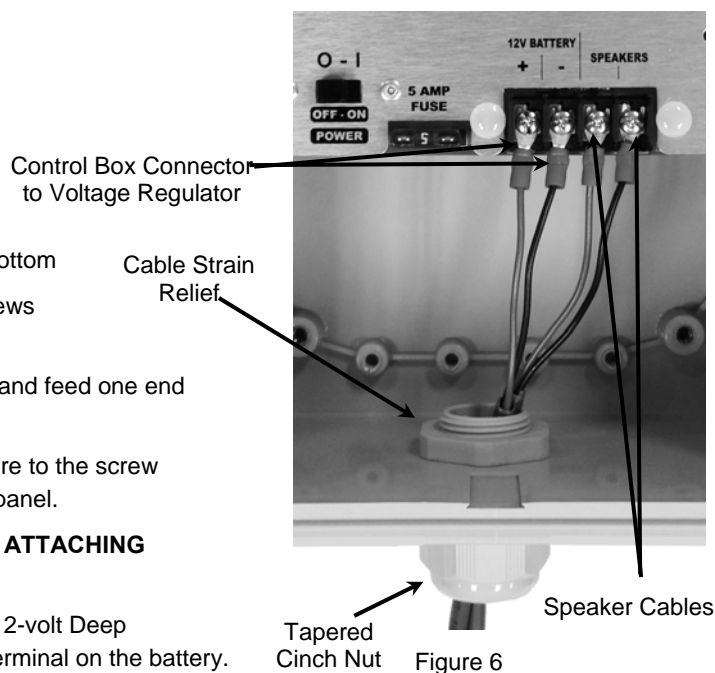


Figure 5

Control Box

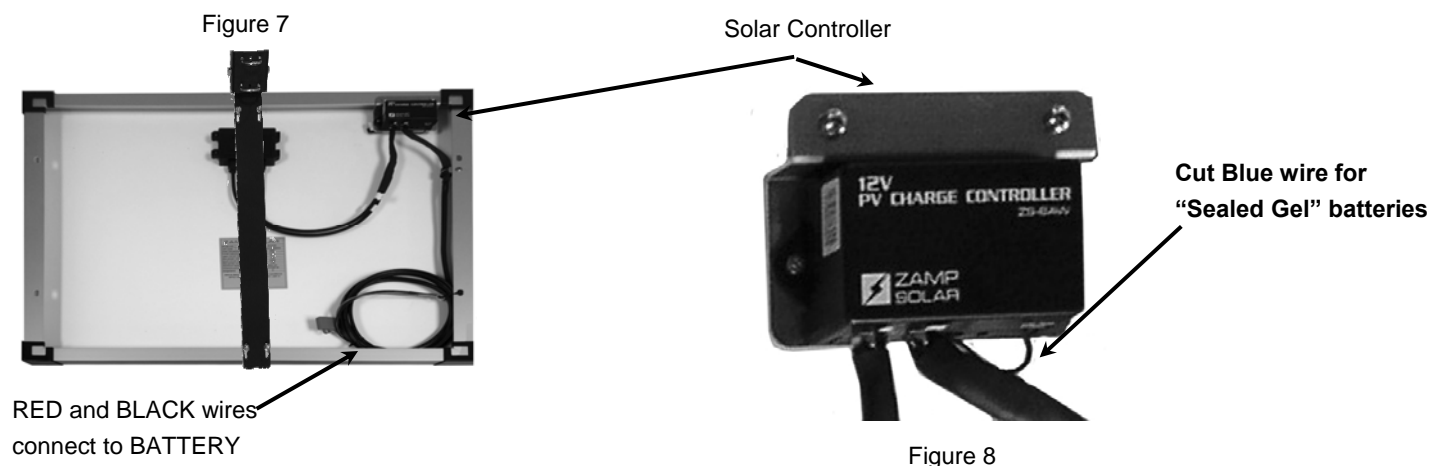
8. Attach the Control Box to the Mounting Pole with the U-Bolts.
9. Feed the Speaker Cables through the Cable Strain Relief at the bottom
10. Attach the Speaker Cables from the 20-Speaker Tower to the screws marked "SPEAKER" on the faceplate of the control panel.
11. Locate the Control Box Connector Cable (the grey 2 lead cables) and feed one end through the Cable Strain Relief.
12. Connect the RED wire to the screw marked "+" and the BLACK wire to the screw marked "-" under "12V BATTERY" on the faceplate of the control panel.
13. **MAKE SURE THE POWER SWITCH IS TURNED OFF BEFORE ATTACHING BATTERY.**
14. Connect the other end of the RED wire to the "+" terminal on the 12-volt Deep Cycle battery (not included). Connect the BLACK wire to the "-" terminal on the battery.
15. Hand tighten the Tapered Cinch Nut on the bottom of the Cable Strain Relief to help keep insects and moisture out.



Solar Panel Connections

16. Cut the black zip-ties securing the RED and BLACK wires on the underside of the solar panel. (Figure 7)
17. Connect the RED wire to the "+" terminal on the 12-volt battery and connect the BLACK wire to the "-" terminal on the battery.

NOTE: If you are using a "Sealed Gel" 12-volt battery (instead of a Lead Acid battery) you will need to cut the indicated small BLUE wire on the attached voltage regulator. This prevents Sealed Gel batteries from being overcharged. Failure to cut this wire can result in permanent battery damage. (Figure 8)



CAUTION: The Mega Blaster Pro is capable of producing sounds up to 125 decibels. Hearing protection must be worn anytime the unit is on!



Settings

Repelling birds requires regular monitoring and active management. Birds are intelligent and highly adaptable so it is important to create and maintain an environment the birds perceive as hostile and dangerous. This is achieved by playing the sounds frequently and at a high volume, otherwise the birds will not be fully repelled and will soon learn to adapt.

Below are the initial settings that should be used when your Mega Blaster Pro is first installed. Please see the “Bird Control Management Guidelines” section for more information.

Recordings

There are eight separate bird sounds contained on the Replaceable Sound Card. The label on the sound card lists each sound with a number corresponding to the eight “RECORDINGS” dip switches to the left of the Sound Card. Initially all RECORDING switches should be turned ON. If the target birds begin returning, periodically change the switch settings for the eight sounds (turning them ON or OFF). **NOTE: NEVER turn OFF the distress calls of the target birds you are trying to repel and always keep at least one predator bird sound turned ON.**

Mode Settings

TIME OFF INTERVAL controls the time off periods between each playing of the bird recordings.

Setting	Time Off Duration	Switch #1	Switch #2
SHORT	17-50 Seconds	ON	OFF
MEDIUM	1:00-4:15 Minutes	OFF	ON
LONG	5:00-10:00 Minutes	ON	ON
XLONG	10:00-30:00 Minutes	OFF	OFF

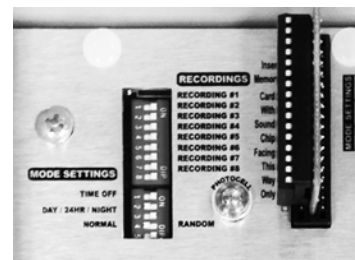
When the Mega Blaster Pro unit is first installed the **TIME OFF INTERVAL** should be set to **SHORT** to create the greatest sense of danger and move the birds out of the area the fastest. Once the birds have left the area completely for a week or more you may try increasing the **TIME OFF INTERVAL** gradually, but you must monitor the birds carefully. Switch back to **SHORT** at the first sign birds are returning.

TIME OF OPERATION controls when the bird recordings play.

Setting	Switch #3	Switch #4
DAY ONLY	ON	OFF
24-HOUR	OFF	ON
NIGHT ONLY	ON	ON

In most cases birds are only active during the day so the **DAY ONLY** is recommended. If birds are roosting in bordering trees at night you will need to set the **TIME OF OPERATION** for **24-HOUR**.

RANDOM OPERATION should always be turned **ON**. **VOLUME** should be set as high as possible.



Recommended Settings

Troubleshooting

Problem	Possible Cause	Solution
No Sound	Volume turned down	Turn volume up
	Dead battery	Charge or replace battery
	Loose battery connection	Verify all battery connections are tight
	All RECORDINGS are turned OFF	Verify all RECORDINGS are switched to ON
	Sound Card not fully seated	Remove sound card and reinstall, making sure it is fully inserted into the socket
	Sound Card is installed backward	Unplug the sound card and reinstall with the label facing to the left
	TIME OF OPERATION set to DAY ONLY without enough light	Change TIME OF OPERATION to 24-HOUR
	Unit was not shut down before the battery was disconnected causing the unit to go into "SAFE MODE"	<ol style="list-style-type: none"> 1. Turn the POWER switch OFF 2. Disconnect the battery 3. Remove the sound card 4. Wait 30 seconds 5. Reinstall sound card 6. Reconnect the battery 7. Turn the POWER switch ON
Was working but stopped	The battery is dead	Connect the battery to a battery charger and see if it will hold a charge. Replace if necessary
	Solar Panel is not getting enough sunlight	Reposition the Solar Panel

Limited Warranty

THIS MEGA BLASTER PRO UNIT IS WARRANTED AGAINST DEFECTS IN MATERIAL AND WORKMANSHIP FOR SIX MONTHS FROM DATE OF PURCHASE (EXTENDED WARRANTY AVAILABLE). BIRD-X WILL REPLACE OR REPAIR, PROVIDED DEFECT OCCURS UNDER NORMAL USE. *RETURNS ACCEPTED ONLY WITH AUTHORIZATION FROM OUR CHICAGO OFFICE.*



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EPA Establishment Number 075130-OR-001

Mega Blaster Pro P/N 655-0065-00 (Rev. 9/2013)



Appendix B

Design/Construction Plan

R.T. Hicks Consultants, Ltd.

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Albuquerque, NM 87104

C-147 Supplemental Information: Appendix B Design and Construction Plan – Merchant Containment

Applicable mandates in Rule 34 are underlined. This plan addresses construction of the Advance Energy Merchant Containment. Appendix A contains the surveyed “as built” drawing of the containment prior to lining and liner and geotextile specifications are attached to this document.

Pettigrew Engineers provided the design of the containment as well as a geotechnical evaluation of the liner foundation and levees. Their design is included in this plan.

Dike Protection and Structural Integrity

The design and operation provide for the confinement of produced water, to prevent releases and to prevent overtopping due to wave action or rainfall. Additionally, the design prevents run-on of surface water as the containment is surrounded by an above-grade levee (a berm) and/or diversion ditch (between the levee and the soil stockpile) to prevent run-on of surface water.

Stockpile Topsoil

Where topsoil was present, prior to constructing containment, the operator stripped and stockpiled the topsoil for use as the final cover or fill at the time of closure.

Signage

The operator will place an upright sign no less than 12 inches by 24 inches with lettering not less than two inches in height in a conspicuous place on the fence surrounding the containment. The sign is posted in a manner and location such that a person can easily read the legend. The sign will provide the following information:

- the operator's name,
- the location of the site by quarter-quarter or unit letter, section, township and range, and
- emergency telephone numbers

Fencing **Variance**

The operator will provide for a fence to enclose the recycling containment in a manner that deters unauthorized wildlife and human access. The perimeter fence is a game fence rather than a a four foot fence that has at least four strands evenly spaced in the interval between one foot and four feet above ground level. Because feral pigs, javelena and deer are present in the area, an 8-foot high game fence is required in order to comply with Section 19.15.34.12 D.1 of the Rule¹. The specification for fencing provided in 19.15.34.12 D.2 contradicts D.1 because pigs will move beneath the lower strand of a 4-strand, 4-foot high barbed wire fence and deer will jump over. Thus, compliance with D.2 results in a violation of D.1. ~~Compliance with D.1 is the critical component of the Rule and operators need not submit a variance request in order to follow Best Management Practices and comply with the Rule.~~ As stated in the O&M plan, the operator will ensure that all gates associated with the fence are closed and locked when responsible personnel are not onsite.

¹ The operator shall fence or enclose a recycling containment in a manner that deters unauthorized wildlife and human access and shall maintain the fences in good repair.

C-147 Supplemental Information: Appendix B Design and Construction Plan – Merchant Containment

Netting and Protection of Wildlife

The perimeter game fence will be effective in excluding stock and most terrestrial wildlife. If requested by the surface owner, the game fence can include a fine mesh from the base to 1 foot above the ground to exclude the small reptiles (e.g. dune sagebrush lizard).

The recycling containment will be protective of wildlife, including migratory birds through the implementation of an Avian Protection Plan (attached hereto), routine inspections and the perimeter fence. The O&M plan calls for the operator to inspect for and, within 30 days of discovery, report the discovery of dead migratory birds or other wildlife to the appropriate wildlife agency and to the division district office in order to facilitate assessment and implementation of measures to prevent incidents from reoccurring.

Earthwork

The containment will have a properly constructed foundation and interior slopes consisting of a firm, unyielding base, smooth and free of rocks, debris, sharp edges or irregularities to prevent the liner's rupture or tear. Geotextile may be placed under the liner when needed to reduce localized stress-strain or protuberances that otherwise may compromise the liner's integrity.

Appendix A shows the

- a) levee has inside grade no steeper than two horizontal feet to one vertical foot (2H: 1V).
- b) levee outside grade is no steeper than three horizontal feet to one vertical foot (3H: 1V)
- c) top of the levee is wide enough to install an anchor trench and provide adequate room for inspection and maintenance.
- d) The containment floor design calls for a slope toward the sump in the northeast corner.

Liner and Drainage Geotextile Installation

The containment has a primary (upper) liner and a secondary (lower) liner with a leak detection system appropriate to the site's conditions.

The primary (upper) liner is a geomembrane liner composed of an impervious, synthetic material that is resistant to ultraviolet light, petroleum hydrocarbons, salts and acidic and alkaline solutions. It is 60-mil HDPE. The secondary liner is 40-mil HDPE and is equivalent to 30-mil LLDPEr (see equivalency demonstration). Liner compatibility meets or exceeds a subsequent relevant publication to EPA SW -846 method 9090A.

The recycling containment design has a leak detection system between the upper and lower geomembrane liners of 200-mil geonet to facilitate drainage. The leak detection system consists of a properly designed drainage and collection and removal system placed above the lower geomembrane liner in depressions and sloped to facilitate the earliest possible leak detection. The containment floor design calls for a slope toward the sump in the northeast corner. This slope combined with the highly transmissive geonet drainage layer provide for rapid leak detection.

C-147 Supplemental Information: Appendix B Design and Construction Plan – Merchant Containment

The liners and drainage material will be installed consistent with the Manufacturer's specifications (See Appendix C). In addition to any specifications of the Manufacturer, protocols for liner installation include measures to:

- i. minimizing liner seams and orient them up and down, not across, a slope of the levee.
- ii. use factory-welded seams where possible.
- iii. use field seams in geosynthetic material are thermally seamed and prior to field seaming, overlap liners four to six inches.
- iv. minimize the number of field seams and comers and irregularly shaped areas.
- v. provide for no horizontal seams within five feet of the slope's toe.
- vi. use qualified personnel to perform field welding and testing.
- vii. avoid excessive stress-strain on the liner
- viii. The edges of all liners are anchored in the bottom of a compacted earth-filled trench that is at least 18 inches deep

At points of discharge into the lined earthen containment the pipe configuration effectively protects the liner from excessive hydrostatic force or mechanical damage during filling.

The design show that at any point of discharge into or suction from the recycling containment, the liner is protected from excessive hydrostatic force or mechanical damage. External discharge or suction lines do not penetrate the liner.

Pumping from the containment to hydraulic fracturing operations is the responsibility of stimulation contractors. Typically, lines are permanently placed in the containment with floats attached to prevent damage to the liner system. The containment may be equipped with permanent HDPE stinger (supported by a sacrificial liner or geotextile) for withdrawal of fluid if the owner deems necessary during operations.

External discharge or suction lines do not penetrate the liner.

Leak Detection and Fluid Removal System Installation

The leak detection system, contains the following design elements

- a. The 200-mil HyperNet Geonet drainage material between the primary and secondary liner that is sufficiently permeable to allow the transport of fluids to the observation ports (Appendix A).
- b. The containment floor is sloped towards the monitoring riser pipe to facilitate the earliest possible leak detection of the containment bottom. A pump may be placed in the observation port to provide for fluid removal.
- c. Piping will withstand chemical attack from any seepage; structural loading from stresses and disturbances from overlying water, cover materials, equipment operation or expansion or contraction (see Appendix A).

Appendix C

Operating and Maintenance Plan

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Operating and Maintenance Procedures

In this plan, underlined text represents the language of the Rule.

The operator will operate and maintain the lined earthen containment to contain liquids and solids (blow sand and minimal precipitates from the treated produced water) and maintain the integrity of the liner system in a manner that prevents contamination of fresh water and protects public health and the environment as described below. The purpose of the lined earthen containment is to facilitate recycling, reuse and reclamation of produced water derived from nearby oil and gas wells. During periods when water for E&P operations is not needed, produced water will discharge to one of the injection wells in the operator's SWD system. The containment will not be used for the disposal of produced water or other oilfield waste.

The operation of the containment is summarized below.

- A. Via pipeline, produced water generated from nearby oil and gas wells is delivered to a treatment system located as indicated in the C-147.
- B. After treatment, the produced water discharges into the containment
- C. When required, treated produced water is removed from the containment for E&P operations. At this time, treated produced water will be used for drilling beneath the fresh water zones (beneath surface casing), for well stimulation (e.g. hydraulic fracturing) and other E&P uses as approved by OCD.
- D. Whenever the maximum fluid capacity of the containment is reached, treatment and discharge to the containment ceases (see Freeboard and Overtopping Plan, below)
- E. The operator will keep accurate records and shall report monthly to the division the total volume of water received for recycling, with the amount of fresh water received listed separately, and the total volume of water leaving the facility for disposition by use on form C-148.
- F. The operator will maintain accurate records that identify the sources and disposition of all recycled water that shall be made available for review by the division upon request.
- G. The containment shall be deemed to have ceased operations if less than 20% of the total fluid capacity is used every six months following the first withdrawal of produced water for use. The operator will report cessation of operations to the appropriate division district office. The appropriate division district office may grant an extension to this determination of cessation of operations not to exceed six months.

The operation of the lined earthen containment will follow the mandates listed below:

1. The operator will not discharge into or store any hazardous waste (as defined by 40 CFR 261 and NMAC 19.15.2.7.H.3) in the containments.
2. If the containment's primary liner is compromised above the fluid's surface, the operator will repair the damage or initiate replacement of the primary liner within 48 hours of discovery or seek an extension of time from the division district office.
3. If the primary liner is compromised below the fluid's surface, the operator will remove all fluid above the damage or leak within 48 hours of discovery, notify the division district office and repair the damage or replace the primary liner.
4. If any penetration of the containment liner is confirmed by sampling of fluid in the leak detection system (see Inspection and monitoring plan), The operator will

C-147 Supplemental Information: Operation and Maintenance Plan Lined Earthen Containment

- a. Begin and maintain fluid removal from the leak detection/pump-back system
 - b. Notify the district office within 48 hours (phone or email) of the discovery
 - c. Identify the location of the leak and
 - d. Repair the damage or, if necessary, replace the containment liner
5. The operator will install, or maintain on site, an oil absorbent boom or other device to contain an unanticipated release and the operator will remove any visible layer of oil from the surface of the recycling containment.
6. The operator will report releases of fluid in a manner consistent with NMAC 19.15.29
7. The containment will be operated to prevent the collection of surface water run-on.
8. The operator will maintain the containment free of miscellaneous solid waste or debris.
9. The operator will maintain at least three feet of freeboard for the containment and will use a free-standing staff gauge to allow easy determination of the required 3-foot of freeboard.
10. As described in the design/construction plan, the injection or withdrawal of fluids from the containment is accomplished through a hardware that prevents damage to the liner by erosion, fluid jets or impact from installation and removal of hoses or pipes.
11. The operator shall ensure that all gates associated with the fence are closed and locked when responsible personnel are not onsite.
12. The operator will maintain the fences in good repair

Monitoring, Inspection, and Reporting Plan

The operator will inspect the recycling containment and associated leak detection systems weekly while it contains fluids. The operator shall maintain a current log of such inspections and make the log available for review by the division upon request.

Weekly inspections consist of

- reading and recording the fluid height of staff gauges
- recording any evidence that the pond surface shows visible oil
- visually inspecting the containment's exposed liners
- checking the leak detection system for any evidence of a loss of integrity of the primary liner.

As stated above, if a liner's integrity is compromised, or if any penetration of the liner occurs above the water surface, then the operator will notify the District office within 48 hours (phone or email).

Monthly, the operator will

- A. Inspect diversion ditches and berms around the containment to check for erosion and collection of surface water run-on.
- B. Inspect the leak detection system for evidence of damage or malfunction and monitor for leakage
- C. Inspect the containment for dead migratory birds and other wildlife. Within 30 days of discovery, report the discovery of dead migratory birds or other wildlife to the appropriate wildlife agency and to the division district office in order to facilitate assessment and implementation of measures to prevent incidents from reoccurring.
- D. Report to the division the total volume of water received for recycling, with the amount of fresh water received listed separately, and the total volume of water leaving the facility for disposition by use on form C-148.

E. Record sources and disposition of all recycled water

The operator will maintain a log of all inspections and make the log available for the appropriate Division district office's review upon request. An example of the log is attached to this section of the permit application.

Freeboard and Overtopping Prevention Plan

The method of operation of the containment allows for maintaining freeboard with very few potential problems. When the capacity of the containment is reached (3-feet of freeboard), the discharge of treated produced water ceases and the produced water generated by nearby oil and gas wells is managed by one of the injection wells identified in Appendix E.

If rising water levels suggest that 3-feet of freeboard will not be maintained, the operator will implement one or more of the following options

- I. Cease discharging treated produced water to the containment
- II. Accelerate re-use of the treated produced water for purposes approved by the Division
- III. Transfer treated produced water from the containment to injection wells

The reading of the staff gauge typically occurs daily when treatment operations are ongoing and weekly when discharge to the containment is not occurring.

Protocol for Leak Detection Monitoring, Fluid Removal and Reporting

As shown in Appendix A, the leak detection system includes a monitoring system. Any fluid released from the primary liner will flow to the collection sump where fluid level monitoring is possible at the monitoring riser pipe associated with the leak detection system (see Appendix A).

Staff may employ a portable electronic water level meter to determine if fluid exists in the monitoring riser pipe. Obtaining accurate readings of water levels in a sloped pipe beneath a containment can be a challenge. An electrician's wire snake may be required to push the probe to the bottom of the port and the probe may be fixed in a 2-inch pipe "dry housing" to avoid false readings due to water condensation on the pipe. There are many techniques to determine the existence of water in the sumps – including low flow pumps and a simple small bailer affixed to an electrician's snake. The operator will use the method that works best for this containment.

If seepage from the containment into the leak detection system is suspected by a positive fluid level measurement, the operator will

1. Re-measure fluid levels in the monitoring riser pipe on a daily basis for one week to determine the rate of seepage.
2. Collect a water sample from the monitoring riser pipe to confirm the seepage is treated produced water from the containment via field conductivity and chloride measurements.
3. Notify NMOCD of a confirmed positive detection in the system within 48-hours of sampling (initial notification).
4. Install a pump into the monitoring riser pipe sump to continually (manually on a daily basis or via automatic timers) remove fluids from the leak detection

C-147 Supplemental Information: Operation and Maintenance Plan Lined Earthen Containment

- system into the containment until the liner is repaired or replaced.
5. Dispatch a liner professional to inspect the portion of the containment suspected of leakage during a “low water” monitoring event.
 6. Provide NMOCD a second report describing the inspection and/or repair within 20 days of the initial notification

If the point of release is obvious from a low water inspection, the liner professional will repair the loss of integrity. If the point of release cannot be determined by the inspection, the liner professional will develop a more robust plan to identify the point(s) of release. The inspection plan and schedule will be submitted to OCD with the second report. The operator will implement the plan upon OCD approval.

Containment Inspection Form

Month Oct-14

Day	Weekly	Low Water	Activity	Monthly	Staff Gauge	Comments
1 - Wed						
2	x				8.75	Gate unlocked upon arrival - notified Jerry Smith, no birds in pit
3					10	
4					12	
5			x			Water transfer to frac - pipes are good
6			x			Water transfer to frac - pipes are good
7		x			2.5	No visible liner problems
8					3	
9	x				4	All OK - no oil on surface, no birds in pit
10					5	
11					5	
12					6	
13					7	
14					7.5	
15				x	8	No fluid in leak detection, outer berm and stormwater diversion OK, H2S - no alarm,
16					9	
17					9	
18					9.5	
19	x				10	All OK
20					11	
21					12	
22			x			Water transfer to frac - no problems
23			x			Water transfer to frac - no problems
24		x			1.75	No visible liner problems
25					2.25	
26	x				3.75	High wind -liner is good, no birds
27					4.75	
28					5.5	
29					6.75	
30					7.75	
31					8.5	

Appendix D

Closure Plan

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C-144 Supplemental Information: Closure Plan Earthen Lined Containment

In this plan, underlined text represents the language of the Rule.

After operations cease, the operator will remove all fluids within 60 days and close the containment within six months from the date the operator ceases operations from the containment for use.

The operator shall substantially restore the impacted surface area to

- the condition that existed prior to the construction of the recycling containment or
- to a condition imposed by federal, state trust land or tribal agencies on lands managed by those agencies as these provisions govern the obligations of any operator subject to those provisions.

As this containment will excavate caliche for future use and pay the surface owner (BLM) for the harvest and use of this material. We anticipate the surface owner will impose a closure design that conforms to one of a caliche mine rather than the condition that existed prior to construction. Until a change to closure as a caliche mine is required by BLM, the prescriptive mandates set forth in this plan will be in effect. The operator understands that a variance will be submitted to OCD to allow for any alternative closure protocol.

Excavation and Removal Closure Plan – Protocols and Procedures

The containment is expected to hold a small volume of solids, the majority of which will be windblown sand and dust with some mineral precipitates from the water

1. The operator will remove all liquids from the pits and either:
 - a. Dispose of the liquids in a division-approved facility, or
 - b. Recycle, reuse or reclaim the water for reuse in drilling and stimulation.
2. The operator will close the recycling containment by first removing all fluids, contents and synthetic liners and transferring these materials to a division approved facility.
3. After the removal of the pit contents and liners, soils beneath the containment will be tested by collection of a five-point (minimum) composite sample which includes stained or wet soils, if any, and that sample shall be analyzed for the constituents listed in Table I of 19.15.34.14.
4. After review of the laboratory results
 - a. If any contaminant concentration is higher than the parameters listed in Table I, additional delineation may be required and the operator must receive approval before proceeding with closure.
 - b. If all contaminant concentrations are less than or equal to the parameters listed in Table I, then the operator will proceed to
 - i. backfill with non-waste containing, uncontaminated, earthen material. Or
 - ii. undertake an alternative closure process pursuant to a variance request after approval by OCD

Reclamation and Re-vegetation

- a. The operator will reclaim the containment's location to a safe and stable condition that blends with the surrounding undisturbed area.
- b. Topsoils and subsoils shall be replaced to their original relative positions and contoured so as to achieve erosion control, long-term stability and preservation of surface water flow patterns.

C-144 Supplemental Information: Closure Plan Earthen Lined Containment

- c. The disturbed area shall then be reseeded in the first favorable growing season following closure of a recycling containment.

Closure Documentation

Within 60 days of closure completion, the operator shall submit a closure report on form C-147, including required attachments, to document all closure activities including sampling results and the details on any backfilling, capping or covering, where applicable. The closure report shall certify that all information in the report and attachments is correct and that the operator has complied with all applicable closure requirements and conditions specified in division rules or directives.

The operator shall notify the division when reclamation and re-vegetation are complete. Specifically the notice will document that all ground surface disturbing activities at the site have been completed, and a uniform vegetative cover has been established that reflects a life-form ratio of plus or minus fifty percent (50%) of pre-disturbance levels and a total percent plant cover of at least seventy percent (70%) of pre-disturbance levels, excluding noxious weeds.