
C-147 REGISTRATION PACKAGE
960 RECYCLING FACILITY
E/2 SE/4 SECTION 23, T23S, R26E
EDDY COUNTY, NEW MEXICO

PREPARED FOR



PREPARED BY



JULY 2018

James R. Stallings
7/30/18



District I
1625 N. French Dr., Hobbs, NM 88240
District II
811 S. First St., Artesia, NM 88210
District III
1000 Rio Brazos Road, Aztec, NM 87410
District IV
1220 S. St. Francis Dr., Santa Fe, NM 87505

State of New Mexico
Energy Minerals and Natural Resources
Department
Oil Conservation Division
1220 South St. Francis Dr.
Santa Fe, NM 87505

Form C-147
Revised April 3, 2017

Recycling Facility and/or Recycling Containment

Type of Facility: ☒ Recycling Facility ☒ Recycling Containment*
Type of action: ☒ Permit ☒ Registration
☐ Modification ☐ Extension
☐ Closure ☐ Other (explain) _____

* At the time C-147 is submitted to the division for a Recycling Containment, a copy shall be provided to the surface owner.

Be advised that approval of this request does not relieve the operator of liability should operations result in pollution of surface water, ground water or the environment. Nor does approval relieve the operator of its responsibility to comply with any other applicable governmental authority's rules, regulations or ordinances.

1.
Operator: 3 Bear Energy (For multiple operators attach page with information) OGRID #: 372603
Address: 1512 Larimer St #540, Denver, Colorado, 80202
Facility or well name (include API# if associated with a well): 960 Recycle Facility East Pit
OCD Permit Number: _____ (For new facilities the permit number will be assigned by the district office)
U/L or Qtr/Qtr SE/4 Section 23 Township 23 South Range 26 East County: Eddy
Surface Owner: ☐ Federal ☐ State ☒ Private ☐ Tribal Trust or Indian Allotment

2.
☒ **Recycling Facility:**
Location of recycling facility (if applicable): Latitude 32.287281 Longitude -104.257159 NAD83
Proposed Use: ☒ Drilling* ☒ Completion* ☒ Production* ☒ Plugging *
**The re-use of produced water may NOT be used until fresh water zones are cased and cemented*
☐ Other, *requires permit for other uses. Describe use, process, testing, volume of produced water and ensure there will be no adverse impact on groundwater or surface water.*
☒ Fluid Storage
☐ Above ground tanks ☒ Recycling containment ☐ Activity permitted under 19.15.17 NMAC explain type _____
☐ Activity permitted under 19.15.36 NMAC explain type: _____ ☐ Other explain _____
☐ For multiple or additional recycling containments, attach design and location information of each containment
☐ **Closure Report (required within 60 days of closure completion):** ☐ Recycling Facility Closure Completion Date: _____

3.
☒ **Recycling Containment:**
☐ Annual Extension after initial 5 years (attach summary of monthly leak detection inspections for previous year)
Center of Recycling Containment (if applicable): Latitude 32.287281 Longitude -104.257159 NAD83
☐ For multiple or additional recycling containments, attach design and location information of each containment
☒ Lined ☒ Liner type: Thickness 40 mil (secondary) 60-mil (primary) ☒ LLDPE ☒ HDPE ☐ PVC ☐ Other _____
☐ String-Reinforced
Liner Seams: ☒ Welded ☒ Factory ☒ Other Field Welds Volume: 500,000 bbl Dimensions: L 600 x W 500 x D 14.5
☐ Recycling Containment Closure Completion Date: _____

4.

Bonding:

- ☒ Covered under bonding pursuant to 19.15.8 NMAC per 19.15.34.15(A)(2) NMAC (These containments are limited to only the wells owned or operated by the owners of the containment.)
- ☐ Bonding in accordance with 19.15.34.15(A)(1). Amount of bond \$ _____ (work on these facilities cannot commence until bonding amounts are approved)
- ☐ Attach closure cost estimate and documentation on how the closure cost was calculated.

5.

Fencing:

- ☐ Four foot height, four strands of barbed wire evenly spaced between one and four feet
- ☒ Alternate. Please specify: chain Link Game Fence with barbed wire

6.

Signs:

- ☒ 12"x 24", 2" lettering, providing Operator's name, site location, and emergency telephone numbers
- ☐ Signed in compliance with 19.15.16.8 NMAC

7.

Variances:

Justifications and/or demonstrations that the proposed variance will afford reasonable protection against contamination of fresh water, human health, and the environment.

Check the below box only if a variance is requested:

- ☒ Variance(s): Requests must be submitted to the appropriate division district for consideration of approval. If a Variance is requested, include the variance information on a separate page and attach it to the C-147 as part of the application.
- If a Variance is requested, it must be approved prior to implementation.**

8.

Siting Criteria for Recycling Containment

Instructions: The applicant must provide attachments that demonstrate compliance for each siting criteria below as part of the application. Potential examples of the siting attachment source material are provided below under each criteria.

General siting

Ground water is less than 50 feet below the bottom of the Recycling Containment.

NM Office of the State Engineer - iWATERS database search; USGS; Data obtained from nearby wells

☐ Yes ☒ No
☐ NA

Within incorporated municipal boundaries or within a defined municipal fresh water well field covered under a municipal ordinance adopted pursuant to NMSA 1978, Section 3-27-3, as amended.

☐ Yes ☒ No
☐ NA

- Written confirmation or verification from the municipality; written approval obtained from the municipality

Within the area overlying a subsurface mine.

☐ Yes ☒ No

- Written confirmation or verification or map from the NM EMNRD-Mining and Minerals Division

Within an unstable area.

☐ Yes ☒ No

- Engineering measures incorporated into the design; NM Bureau of Geology & Mineral Resources; USGS; NM Geological Society; topographic map

Within a 100-year floodplain. FEMA map

☐ Yes ☒ No

Within 300 feet of a continuously flowing watercourse, or 200 feet of any other significant watercourse, or lakebed, sinkhole, or playa lake (measured from the ordinary high-water mark).

☐ Yes ☒ No

- Topographic map; visual inspection (certification) of the proposed site

Within 1000 feet from a permanent residence, school, hospital, institution, or church in existence at the time of initial application.

☐ Yes ☒ No

- Visual inspection (certification) of the proposed site; aerial photo; satellite image

Within 500 horizontal feet of a spring or a fresh water well used for domestic or stock watering purposes, in existence at the time of initial application.

☐ Yes ☒ No

- NM Office of the State Engineer - iWATERS database search; visual inspection (certification) of the proposed site

Within 500 feet of a wetland.

☐ Yes ☒ No

- US Fish and Wildlife Wetland Identification map; topographic map; visual inspection (certification) of the proposed site

9.

Recycling Facility and/or Containment Checklist:

Instructions: Each of the following items must be attached to the application. Indicate, by a check mark in the box, that the documents are attached.

- ☒ Design Plan - based upon the appropriate requirements.
- ☒ Operating and Maintenance Plan - based upon the appropriate requirements.
- ☒ Closure Plan - based upon the appropriate requirements.
- ☒ Site Specific Groundwater Data -
- ☒ Siting Criteria Compliance Demonstrations -
- ☒ Certify that notice of the C-147 (only) has been sent to the surface owner(s)

10.

Operator Application Certification:

I hereby certify that the information and attachments submitted with this application are true, accurate and complete to the best of my knowledge and belief.

Name (Print): SCOTT SPICHER Title: Vice President
 Signature: [Signature] Date: 7-26-18
 e-mail address: scott@3beersllc.com Telephone: 303-862-3960

11.

OCD Representative Signature: _____ Approval Date: _____

Title: _____ OCD Permit Number: _____

- ☐ OCD Conditions _____
- ☐ Additional OCD Conditions on Attachment _____

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Name (Print): SCOTT SPIECHER Title: Vice President
 Signature: Scott Spid Date: 7-26-18
 e-mail address: scott@3bearllc.com Telephone: 303-862-3960

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- ☐ OCD Conditions _____
- ☐ Additional OCD Conditions on Attachment _____



2500 North Eleventh Street • Enid, OK 73701 • (580) 234-8780 □ Fax (580) 237-4302 • www.envirotechconsulting.com

June 28, 2018

Mr. Bradford Billings
New Mexico EMNRD
Oil conservation Division
1220 South St. Francis Dr.
Santa Fe, New Mexico 87505

RE: Rule 34 Variance Request –Produced Water Recycling Containment

Mr. Billings:

3Bear Energy is requesting a variance to Rule 34 Part 12(A)(4) requiring secondary liners to be 30-mil string reinforced LLDPE. 3Bear is requesting approval to use 40-mil LLDPE in place of the specified material. Based on our experience, we feel that the requested material will allow us to provide greater environmental protection in our impoundments.

Due to the construction of the 30-mil reinforced LLDPE material, nondestructive QA/QC testing cannot be performed. The proposed 40-mil LLDPE will be seamed in a manner that will allow nondestructive pressure testing of the seams to ensure proper sealing.

The proposed LLDPE is appropriate material for the proposed use in the impoundment, and is compatible with the material that will be stored. This material will provide equal or better environmental protection as the specified 30-mil reinforced LLDPE. Attached with this request is a sample specification sheet for the LLDPE with the proposed material highlighted.

The proposed new liner system cross-section is as follows: prepare subgrade, 12 oz. geotextile, 40-mil LLDPE, single sided geocomposite, 60-mil HDPE (smooth on bottom, textured on slopes). This will replace the cross-section required by the current rule and submitted with the original permit application. It should also be noted that this variance has been granted on past sites.

Should you have any questions or require additional information, please contact me by phone at 580-234-8780 or by email at jstallings@envirotechconsulting.com at your convenience

Thank you for your consideration.
Best regards,

ENVIROTECH ENGINEERING & CONSULTING, INC.

A handwritten signature in black ink, appearing to read "Jimmy Stallings", is written over a horizontal line.

Jimmy Stallings, P.E.
President and Principal Engineer



2500 North Eleventh Street • Enid, OK 73701 • (580) 234-8780 ☐ Fax (580) 237-4302 • www.envirotechconsulting.com

June 28, 2018

Mr. Bradford Billings
New Mexico EMNRD
Oil conservation Division
1220 South St. Francis Dr.
Santa Fe, New Mexico 87505

RE: Rule 34 Variance Request –Produced Water Impoundment Bird Netting

Mr. Billings:

3Bear Energy is requesting a variance to Rule 34-Part 12(E) Netting to ensure the recycling facility is protected from wildlife. Based on our experience from previous projects, we believe audible bird deterrents provide equal or better protection when compared to netting. In addition, they require less inspection, maintenance and repair over the life of the facility.

3Bear is proposing to use the “Bird-X Mega Blaster Pro” system at the Hood Facility. A copy of the user’s manual is attached to this variance request letter.

This system will replace the netting required by the current rule and submitted with the original permit application.

Should you have any questions or require additional information, please contact me by phone at 580-234-8780 or by email at jstallings@envirotechconsulting.com at your convenience

Thank you for your consideration.
Best regards,

ENVIROTECH ENGINEERING & CONSULTING, INC.

A handwritten signature in black ink, appearing to read "Jimmy Stallings", with a stylized flourish at the end.

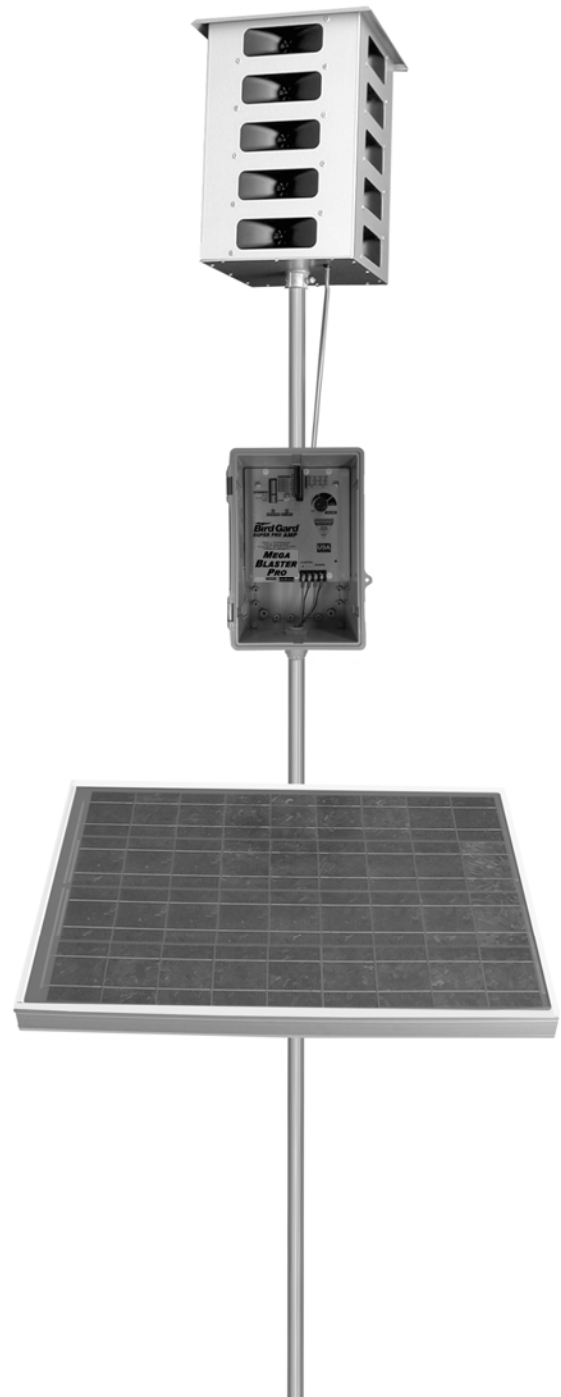
Jimmy Stallings, P.E.
President and Principal Engineer

MEGA BLASTER PRO



User's Manual

| | |
|------------------------------------|----|
| Overview | 2 |
| Bird Control Management Guidelines | 3 |
| Materials List | 4 |
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| 20-Speaker Tower | 8 |
| Solar Panel | 8 |
| Control Box | 9 |
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| Settings | 10 |
| Recordings | 10 |
| Mode Settings | 10 |
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Overview

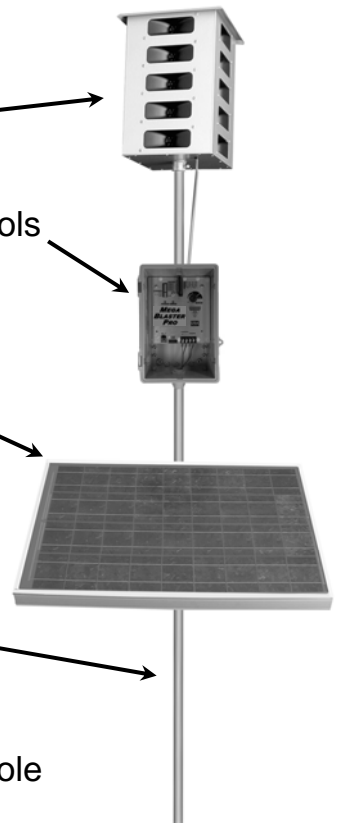
The Bird-X Mega Blaster Pro utilizes the innate power of the natural survival instincts of birds to effectively repel them. Digital recordings of distressed and alarmed birds, along with the sounds made by their natural predators are broadcast through high fidelity weather-resistant speakers over the top of areas. This action triggers a primal fear and flee response. Pest birds soon relocate to where they can feed without feeling threatened.

Your Bird-X Mega Blaster Pro system consists of:

20-Speaker Tower broadcasts the bird sounds

Control Unit produces the bird sounds and contains all operational controls

Solar Panel recharges the 12-volt deep cycle battery



Items needed but not included:

- (1) **Mounting Pole** or **Mast** tall enough to raise the 20-Speaker Tower at least 5 feet above the top of the areas, trees or other obstructions
- (1) **12-volt Deep Cycle Battery** (RV/Marine) Group 27 or larger wet cell
- (1) **T-Post** or similar (Optional) may be needed to support the mounting pole
- (1) **Bailing Wire** or **zip-tie** (Optional) to secure the Mounting Pole to the T-Post

CAUTION: THE MEGA BLASTER PRO IS CAPABLE OF PRODUCING SOUNDS UP TO 125 DECIBELS. PROPER HEARING PROTECTION MUST BE WORN ANYTIME THE UNIT IS TURNED ON.



Bird Control Management Guidelines










An active bird control management program is a key to successfully repelling pest birds. Bird feeding patterns may take several days or weeks to break. Follow all suggestions for maximum effectiveness. Read all instructions prior to installation.

For best results:

- **It is extremely important to fully protect your entire area from birds.** Any areas not fully protected will allow birds to begin feeding at the fringes of the sound coverage. They will soon become bolder and learn the sounds are nothing to fear. This will cause the effectiveness to diminish. Complete Bird-X product coverage forces birds to leave the area entirely.
- Install the Mega Blaster Pro unit at least two weeks before birds are attracted to your area. It is much easier to keep birds away before they have found a food source than it is to repel them once they have developed a feeding pattern.
- Most birds begin feeding from the perimeter of an area. Place Mega Blaster Pro units so the sound protection covers past the edges of the area.
- Birds will often use tall trees for roosting and observation. If birds are in bordering trees it is necessary to position the units so the sound protection covers the trees as well.
- Mount the 20-Speaker Tower at least five feet above trees, areas and structures for maximum coverage. The higher the better. Sound will disperse or reflect off structures or foliage. Mount control unit out of direct sun, if possible.
- When first installed, run Mega Blaster Pro units at FULL volume and on SHORT time off periods. This ensures maximum "bird stress" and creates a hostile environment.
- Watch for changes in bird activity and adjust the location of your Mega Blaster Pro unit if needed.
- **Check the battery and unit settings often to insure continuous bird control. Be certain that the system is not turned down or has a dead battery. Field hands or harvesters may turn down the volume.**
- Changing settings and switches often helps to prevent bird habituation. Periodically change the switch settings of the eight sounds (turning them ON or OFF). NEVER turn OFF the distress calls of the target birds you are trying to repel and always keep at least one predator bird sound turned ON.
- If different bird species enter the protected area and begin causing damage contact us immediately for an updated Sound Recording Card designed to repel the new invading birds.
- Remember that the Mega Blaster Pro system is a management tool, and should be used as part of your overall bird control strategy, sometimes in conjunction with other bird control techniques and devices.

Be aware that under extreme drought or other adverse conditions, birds will disregard all deterrents and risks in order to survive

Materials List

| Item | Qty | | Notes |
|------------------------------|-----|--|------------------------------|
| Mega Blaster Pro Control Box | 1 |  | |
| Sound Recording Card | 1 |  | Pre-installed in control box |
| 20-Speaker Tower | 1 |  | |
| Control Box Mounting U-Bolts | 2 |  | 1/4" x 1" x 2" |
| Control Box Brackets | 2 |  | |
| 40-Watt Solar Panel | 1 |  | |
| Solar Panel Mounting Bracket | 1 |  | |
| Solar Panel Mounting U-Bolts | 2 |  | 1/4" x 1-1/8" x 2" |
| Control Box Connector Cable | 1 |  | 2 Wire, 4 ft. Long |
| Battery Box | 1 |  | |

Assembly

Note: You will find it easier to pre-assemble the following components prior to installation in the field.

Control Unit

1. Lay the Control Unit face down
2. Attach the two Control Box Mounting Brackets to the back with the included screws (Figure 1)

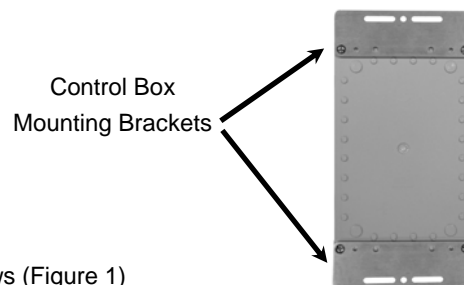


Figure 1

Solar Panel

3. Install the two Solar Panel Mounting U-Bolts in the Head of the Solar Panel Mounting Bracket (Figure 2)
4. Loosen, but do not remove the Carriage Bolts securing the movable Clamp Plates on the Solar Panel Mount Bracket
5. Lay the solar panel on a flat surface with the glass side down
6. Lay the Mounting Arm across the Solar Panel with the Clamp Plates down. Position the Mounting Arm at an angle so the Clamp Plates slide under the lip of the Solar Panel (Figure 3A)
7. Rotate the Mounting Arm and secure it to the Solar Panel by tightening the Carriage Bolts (Figure 3B)

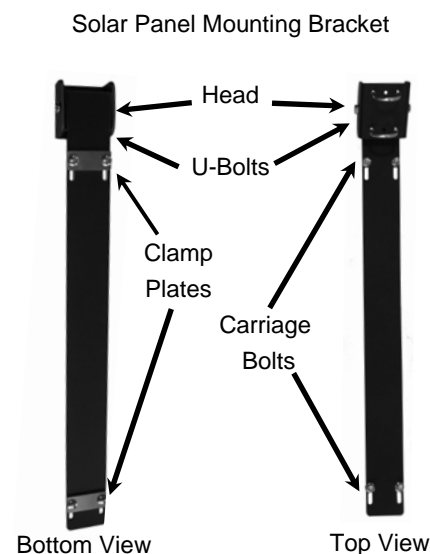


Figure 2

Clamp Plates slide under the lip of the Solar Panel

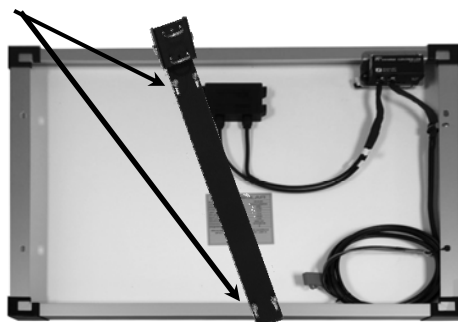


Figure 3A

Rotate Mounting Arm and tighten Carriage Bolts

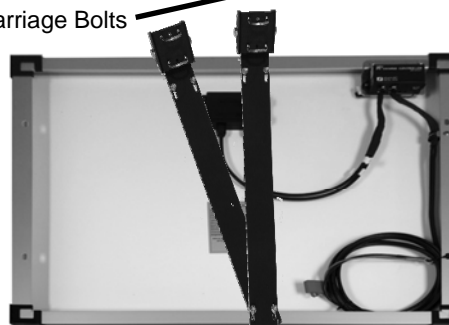


Figure 3B

Placement

Your Mega Blaster Pro will protect an area up to approximately 600 feet in all directions.

Factors to consider when selecting the best location include:

- Birds typically feed from the perimeter of the area and work their way in. Place Mega Blaster Pro units so the sound protection covers all the way to the edges of the area. For larger areas Mega Blaster Pro units should be positioned 400-500 feet inside the area and spaced every 1,200 feet.
- Mount the 20-Speaker Tower at least 5 feet above terrain, areas, trees and other obstacles.
- Placing the Mega Blaster Pro on top of a hill or small rise will give you much better coverage than at the bottom of a valley. The greater the height the further the sounds will travel.
- Wind can blow the sound waves. If the area you need to protect has consistent wind coming from the same direction, position your Mega Blaster Pro more “upwind.”
- Trees surrounding areas provide birds with a safe perch that allows them to fly in, grab food and fly out. It is much more difficult to eliminate bird damage if the birds are able to use the surrounding trees as a staging area for attacks on your areas. Your Mega Blaster Pro unit should be positioned close to any trees bordering your areas. If birds are roosting in the trees at night the TIME OF OPERATION should be set to 24 HOUR.
- Lakes, rivers and wetlands are a favorite resting and hiding place for birds. Your Mega Blaster Pro unit should be placed so the sound thoroughly covers any areas where birds frequent.
- Neighbors, businesses and others may not appreciate hearing the bird sounds. At the limits of the effective range the sounds from your Mega Blaster Pro are at a level people may find annoying. Avoid placing the unit where it becomes a nuisance.

Building a Mounting Pole or Mast

CAUTION: TALL POLES AND MASTS CAN BE HEAVY AND POTENTIALLY DANGEROUS. USE EXTREME CAUTION WHEN CONSTRUCTING OR WORKING AROUND TALL POLES AND MASTS. BIRD-X, INC., ASSUMES NO RESPONSIBILITY FOR DAMAGES OR INJURIES.

Things to consider:

- The 20-Speaker Tower is designed to mount onto a 1 in. (outside diameter) pipe at least 14 in. long. 1 in. conduit works well as it is light, rigid, inexpensive and available in 10 ft. lengths making it ideal for low areas, vineyards and bushes.
- You will want to take down your Mega Blaster Pro unit after harvest and store it in a dry location until the next season.

A suggestion for masts up to 20 feet tall:

1. 3/4 inch Galvanized steel water pipe has a 1 inch outside diameter and is the correct size to fit inside the 20-Speaker Tower. It is often available in 20 ft. lengths from hardware and plumbing supply stores. If these are not available, 10 ft. lengths are common and can be fastened together with a threaded coupler. Assemble the poles on the ground.
2. Slide the 20-Speaker Tower over the pipe and tighten the set screw in the collar at the base.
3. Stand the pole assembly up just inside the drip line of a tree and securely tie the pole to a few heavy branches.
4. Drive a T-Post into the ground at the base of the pole and secure with wire.

For masts taller than 20 feet:

1. Use 20 ft. lengths of galvanized steel water pipe or similar, securely fastened together with threaded reducing couplers.
2. Starting with 3 in. pipe, step the size down with each length of pipe.
3. The last 10 ft. can be 1 in. (O.D.) conduit hose clamped to the final section of galvanized pipe.

A semi-permanent mast support can be made by digging a hole 4 ft. deep and 4 ft. round. In the middle of the hole sink a length of galvanized water pipe large enough that your mast will easily fit inside. Make sure at least 2 ft. of pipe is above ground level. Fill the area around the pipe with packed sand, leaving the last foot filled with concrete to form a cap over the hole. Your mast can be dropped into the galvanized water pipe “receiver” for support. At the end of harvest the mast can be lifted out and positioned on the ground for easy disassembly and storage.

Installation

Note: Foliage, trees, and other obstructions severely reduce the effective range of Mega Blaster Pro units. It is critical that the 20-Speaker Tower is mounted at least 5 feet above all obstructions to achieve the maximum protection.

Mounting Pole or Mast

1. The Mounting Pole or Mast will need to be supported by a T-Post, fence post, tree or other means. The Pole Support should be in place before proceeding.

20-Speaker Tower

2. Lay the 20-Speaker Tower on its side on the ground and cut the zip-tie securing the speaker cables.
3. Slide the 1 in. (outside diameter) Mounting Pole through the Collar at the bottom of the 20-Speaker Tower until it slides over the positioning bolt inside the top of the Tower (Figure 4).
4. Tighten the Set Screw in Collar securely.

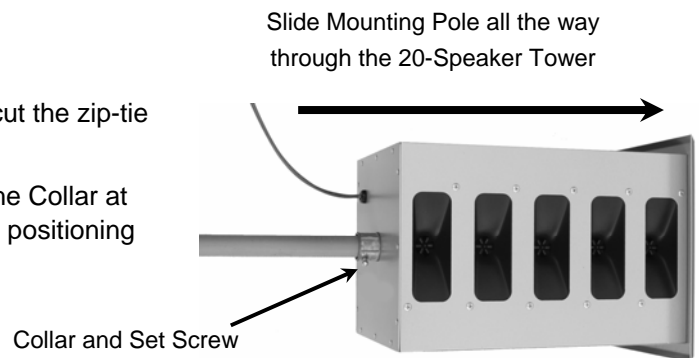


Figure 4

Solar Panel

5. Rest the lower end of the Mounting Pole on the Solar Panel Mounting Bracket approximately three feet from the bottom of the pole with the top of the solar panel facing the 20-Speaker Tower (Figure 5).
6. Lean up the Mounting Pole with the 20-Speaker Tower on top, against the Pole Support and fasten the Mounting Pole to the Pole Support securely with wire or other semi-permanent means.
7. Rotate the solar panel so it receives sunlight.

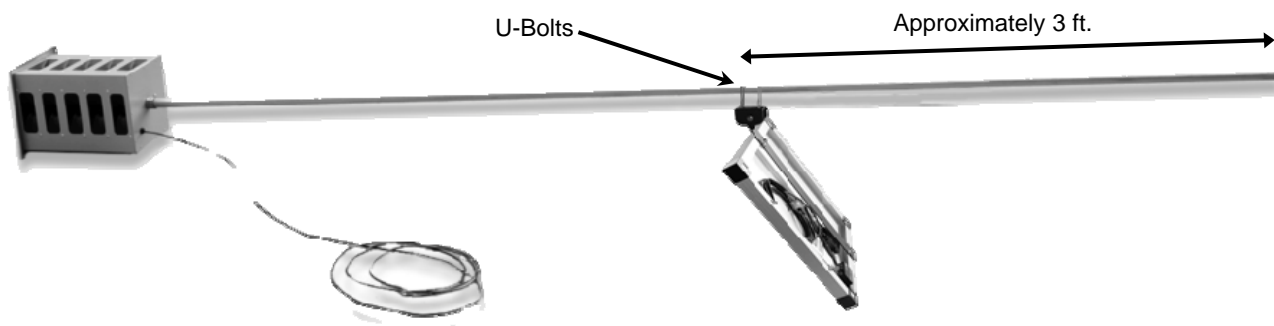


Figure 5

Control Box

8. Attach the Control Box to the Mounting Pole with the U-Bolts.
9. Feed the Speaker Cables through the Cable Strain Relief at the bottom
10. Attach the Speaker Cables from the 20-Speaker Tower to the screws marked "SPEAKER" on the faceplate of the control panel.
11. Locate the Control Box Connector Cable (the grey 2 lead cables) and feed one end through the Cable Strain Relief.
12. Connect the RED wire to the screw marked "+" and the BLACK wire to the screw marked "-" under "12V BATTERY" on the faceplate of the control panel.
13. **MAKE SURE THE POWER SWITCH IS TURNED OFF BEFORE ATTACHING BATTERY.**
14. Connect the other end of the RED wire to the "+" terminal on the 12-volt Deep Cycle battery (not included). Connect the BLACK wire to the "-" terminal on the battery.
15. Hand tighten the Tapered Cinch Nut on the bottom of the Cable Strain Relief to help keep insects and moisture out.

Control Box Connector to Voltage Regulator

Cable Strain Relief

Tapered Cinch Nut

Speaker Cables

Figure 6

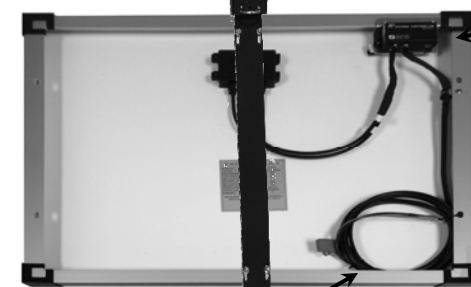
Solar Panel Connections

16. Cut the black zip-ties securing the RED and BLACK wires on the underside of the solar panel. (Figure 7)
17. Connect the RED wire to the "+" terminal on the 12-volt battery and connect the BLACK wire to the "-" terminal on the battery.

NOTE: If you are using a "Sealed Gel" 12-volt battery (instead of a Lead Acid battery) you will need to cut the indicated small BLUE wire on the attached voltage regulator. This prevents Sealed Gel batteries from being overcharged. Failure to cut this wire can result in permanent battery damage. (Figure 8)

Figure 7

Solar Controller



RED and BLACK wires connect to BATTERY



Cut Blue wire for "Sealed Gel" batteries

Figure 8

CAUTION: The Mega Blaster Pro is capable of producing sounds up to 125 decibels. Hearing protection must be worn anytime the unit is on!



Settings

Repelling birds requires regular monitoring and active management. Birds are intelligent and highly adaptable so it is important to create and maintain an environment the birds perceive as hostile and dangerous. This is achieved by playing the sounds frequently and at a high volume, otherwise the birds will not be fully repelled and will soon learn to adapt.

Below are the initial settings that should be used when your Mega Blaster Pro is first installed. Please see the “Bird Control Management Guidelines” section for more information.

Recordings

There are eight separate bird sounds contained on the Replaceable Sound Card. The label on the sound card lists each sound with a number corresponding to the eight “RECORDINGS” dip switches to the left of the Sound Card. Initially all RECORDING switches should be turned ON. If the target birds begin returning, periodically change the switch settings for the eight sounds (turning them ON or OFF). **NOTE: NEVER turn OFF the distress calls of the target birds you are trying to repel and always keep at least one predator bird sound turned ON.**

Mode Settings

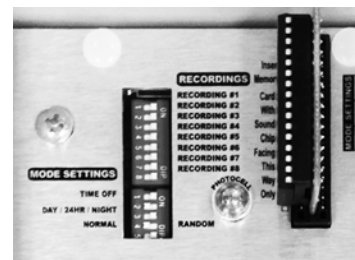
TIME OFF INTERVAL controls the time off periods between each playing of the bird recordings.

| Setting | Time Off Duration | Switch #1 | Switch #2 |
|--------------|----------------------|-----------|------------|
| SHORT | 17-50 Seconds | ON | OFF |
| MEDIUM | 1:00-4:15 Minutes | OFF | ON |
| LONG | 5:00-10:00 Minutes | ON | ON |
| XLONG | 10:00-30:00 Minutes | OFF | OFF |

When the Mega Blaster Pro unit is first installed the **TIME OFF INTERVAL** should be set to **SHORT** to create the greatest sense of danger and move the birds out of the area the fastest. Once the birds have left the area completely for a week or more you may try increasing the **TIME OFF INTERVAL** gradually, but you must monitor the birds carefully. Switch back to **SHORT** at the first sign birds are returning.

TIME OF OPERATION controls when the bird recordings play.

| Setting | Switch #3 | Switch #4 |
|-----------------|-----------|------------|
| DAY ONLY | ON | OFF |
| 24-HOUR | OFF | ON |
| NIGHT ONLY | ON | ON |



Recommended Settings

In most cases birds are only active during the day so the **DAY ONLY** is recommended. If birds are roosting in bordering trees at night you will need to set the **TIME OF OPERATION** for **24-HOUR**.

RANDOM OPERATION should always be turned **ON**. **VOLUME** should be set as high as possible.

Troubleshooting

| Problem | Possible Cause | Solution |
|-------------------------|--|---|
| No Sound | Volume turned down | Turn volume up |
| | Dead battery | Charge or replace battery |
| | Loose battery connection | Verify all battery connections are tight |
| | All RECORDINGS are turned OFF | Verify all RECORDINGS are switched to ON |
| | Sound Card not fully seated | Remove sound card and reinstall, making sure it is fully inserted into the socket |
| | Sound Card is installed backward | Unplug the sound card and reinstall with the label facing to the left |
| | TIME OF OPERATION set to DAY ONLY without enough light | Change TIME OF OPERATION to 24-HOUR |
| | Unit was not shut down before the battery was disconnected causing the unit to go into "SAFE MODE" | <ol style="list-style-type: none"> 1. Turn the POWER switch OFF 2. Disconnect the battery 3. Remove the sound card 4. Wait 30 seconds 5. Reinstall sound card 6. Reconnect the battery 7. Turn the POWER switch ON |
| Was working but stopped | The battery is dead | Connect the battery to a battery charger and see if it will hold a charge. Replace if necessary |
| | Solar Panel is not getting enough sunlight | Reposition the Solar Panel |

Limited Warranty

THIS MEGA BLASTER PRO UNIT IS WARRANTED AGAINST DEFECTS IN MATERIAL AND WORKMANSHIP FOR SIX MONTHS FROM DATE OF PURCHASE (EXTENDED WARRANTY AVAILABLE). BIRD-X WILL REPLACE OR REPAIR, PROVIDED DEFECT OCCURS UNDER NORMAL USE. *RETURNS ACCEPTED ONLY WITH AUTHORIZATION FROM OUR CHICAGO OFFICE.*



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EPA Establishment Number 075130-OR-001

Mega Blaster Pro P/N 655-0065-00 (Rev. 9/2013)





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| APPENDIX E | CLOSURE PLAN |
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1. SITE CRITERIA FOR RECYCLING CONTAINMENT

1.1 LOCATION

The 3Bear Energy 960 Recycle Facility (collectively referred to as Containment), is proposed to be located in the east half of the southeast quarter of Section 23, Township 23 South, Range 26 East of Eddy County, New Mexico.

1.2 DISTANCE TO GROUNDWATER

1.2.1 HYDROLOGY

According to information reviewed from the Bureau of Land Management (BLM) Carlsbad Field Office, the proposed Containment location is not located within a mapped major aquifer system. Major aquifers in the area include the Capitan Reef Complex, Pecos River Basin Alluvial, and the High Plains Aquifer. Available groundwater within the area of the proposed Containment is noted to be within the Carlsbad Groundwater Basin, by the New Mexico Office of the State Engineer (OSE). The Carlsbad Basin contains two major water-bearing features including shallower alluvial aquifer systems and a deeper “artesian” carbonate system. Water-bearing zones include the Triassic age Chinle Formation, of which the Santa Rosa Sandstone is the basal unit.

Groundwater wells in the area are completed at an average depth of 216-ft below ground surface. Of these wells, the closest to the site with a recorded groundwater depth reported groundwater was encountered at approximately 180-ft below ground surface. This well (C-01435) is located approximately 1.4-miles southeast of the site (refer to *Figure 1*). Groundwater in the area is recorded at an average depth of approximately 166-ft below ground surface. This data was obtained from measured water levels or logged borings for hydrogeologic information contained in the OSE database. Available groundwater data (total depth of water wells and depth to groundwater) is presented in *Figure 1*, and an Aquifer Map presenting the area of mapped aquifer systems from the BLM Carlsbad Field Office is presented as *Figure 1A*.

The New Mexico Oil and Gas Division (NMOCD) requires that groundwater (freshwater as defined by NMOCD rules) at the location be greater than 50-ft below the containment bottom. *Figure 1* is an aerial map that demonstrates the following to meet these criteria:



1. The location of the proposed containment shown on an aerial photograph with surface elevation (taken from the United States Geologic Survey (USGS) Kitchen Cove 7.5 Minute Series Topographic Map).
2. Location of area water wells (as plotted in the Office of the State Engineers (OSE) WATERS database). It should be noted, OSE wells can be mislocated as older wells are plotted in the center of the $\frac{1}{4}$, $\frac{1}{4}$, quarter section, township, and range.
3. Total depth of the wells and/or depth to water (where provided) from the most recent available data is plotted adjacent to each located water well.

From the available data, multiple groundwater wells within the immediate vicinity of the proposed pit location were logged as dry holes (with total depths averaging 239-ft). Groundwater was recorded at an approximate depth of 180-ft below ground surface in the closest groundwater well to the site. Additional wells in the area had an average depth to groundwater of 166-ft. Auger refusal was encountered at a depth of 54-ft below ground surface during onsite borings, and groundwater was not encountered in any onsite borings. Based on auger refusal and lack of encountered groundwater, the area of the proposed pit shows separation between the bottom of the containment and groundwater.

1.2.2 GEOLOGY

A geological map for the vicinity of the site was obtained from the New Mexico Bureau of Land Management, Carlsbad Field Office and was used to review the geologic setting for the proposed containment location. Based on the review of the geologic map, the containment location lies within the Halocene to Pleistocene age Piedmont alluvial deposits, consisting of interbedded wind-deposited sands and alluvium.

Area stratigraphy to a maximum depth of 54-ft below ground surface (bgs) was obtained from two (2) geotechnical borings conducted onsite by Terracon Consultants on June 19th and 20th, 2018. The boring logs recorded clayey gravels and sand or silty clay with gravel and sand to a depth of 40- to 54-ft below ground surface. All onsite borings met auger refusal encountering very dense cemented soils. Groundwater was not encountered in any borings performed onsite both before and after drilling.

Figure 2 is a reproduction of the New Mexico Bureau of Geology and Mineral Resources geologic map. Figure 2 shows the following:

1. Location of the proposed Containment
2. Geologic setting of the Containment

1.3 DISTANCE TO MUNICIPAL BOUNDARIES AND FRESH WATER FIELDS

Figure 3 demonstrates that the location is not located within incorporated municipal boundaries or within a defined municipal freshwater field covered under a municipal ordinance, adopted pursuant to NMSA 1978, Section 3-27-3. Figure 3 illustrates the following:

1. The closest municipality to the site is Carlsbad, New Mexico located approximately 6-miles north of the containment location, and Loving New Mexico, located approximately 9-miles east of the containment location.
2. The closest municipal well field is located approximately 5-miles west of the containment location (City of Carlsbad Wellhead Protection Area) serving the community of Carlsbad, New Mexico.

1.4 DISTANCE TO SUBSURFACE MINES

According to the New Mexico Mining and Minerals Division, the nearest mines to the containment locations are two surface stone aggregate mines. The site location is not within an area overlying a subsurface mine but is located within an area labeled "Industrial Mineral District." Figure 4 illustrates the following:

1. The nearest mapped mines are surface stone aggregate, located approximately 2-miles northwest of the containment area.

1.5 DISTANCE TO HIGH OR CRITICAL KARST AREAS (UNSTABLE AREAS)

Figure 5 shows the location of the proposed contaminant area with respect to BLM mapped Karst areas.

1. The proposed Containment is located within a "medium" potential karst area.
2. The nearest "critical" karst area is located approximately 4-miles west of the proposed containment area.
3. The nearest "high" karst area is located approximately 5-miles south of the proposed containment area.



1.6 DISTANCE TO 100-YEAR FLOODPLAIN

The Federal Emergency Management Agency (FEMA) Flood Insurance maps were reviewed for the location of the site. The site is located on FEMA map panel number 35015C1300D and classified as "Zone X." Zone X represents locations that are defined as outside the 0.2% annual chance floodplain. *Figure 6* demonstrates the area of the site is not located within a 100-year Floodplain.

1. The site is located within "Zone X." Zone X is described as areas outside the 0.2% annual chance floodplain. No flood hazard analysis has been conducted for this area.

1.7 DISTANCE TO SURFACE WATER

Figure 7 is a reproduction of the USGS Kitchen Cove 7.5-Minute Series topographic map that demonstrate the site location is not within 300-ft of a continuously flowing watercourse or other significant watercourse, or within 200-ft of a lakebed, sinkhole, or playa lake (as measured from the ordinary high-water mark). The nearest surface water to the site is Cass Draw, located approximately 1.8-miles south of the Containment area. *Figure 7* demonstrates the following:

1. No continuously flowing watercourses or other water bodies defined by NMOCD are located within 300-ft of the proposed containment location.
2. The closest surface water body is Cass Draw located approximately 1.8-miles south of the proposed containment location.

1.8 DISTANCE TO PERMANENT RESIDENCES OR STRUCTURES

Figure 7 is a reproduction of the USGS Kitchen Cove 7.5-Minute Series topographic map that demonstrates that the site location is not within 1,000-ft of an occupied permanent residence, school, hospital, institution, church, or other permanent structure in existence at the time of initial application. The nearest manmade structures to the site location appear to be oil field tank batteries.

1.9 DISTANCE TO NON-PUBLIC WATER SUPPLY

The site is not located within 500-horizontal feet of a private, domestic fresh water well or spring that less than five households use for domestic or stock watering purposes. In addition, the site is not located within 1,000-ft of any other freshwater well or spring, as documented at the time of this application. *Figure 1* illustrates the following:

1. *Figure 1* shows the location of area water wells, active or plugged, relative to the proposed site location.
2. There are no known domestic water wells located within 1,000-ft of the proposed site location.
3. No springs were identified within the mapping area (refer to *Figure 7*).

1.10 DISTANCE TO WETLANDS

The U.S Fish and Wildlife National Wetlands Inventory maps were reviewed for the area of the site. *Figure 8* demonstrates that the site is located within an area of a mapped wetland.

1. The nearest designated wetland to the site is freshwater emergent wetland with a wetland code PEM1Jx (Palustrine, Emergent, Persistent, Intermittently Flooded, excavated). The mapped wetland is located on the site. Envirotech reviewed the Wetlands of the US Delineation Report conducted by Cox/McLain in July 2018 (included herein as *Appendix F*). The wetlands delineation report assessed five (5) sample points within the area of the site. None of the five (5) sample points were identified as having wetland indicators (hydric soils, hydrophilic vegetation, or wetland hydrology). Based on the absence of wetland indicators on the site, the area of the proposed recycle pit is not located within an area defined as a wetland, and would therefore not be subject to jurisdiction by the USACE.

1.11 FIGURES

Site criteria compliance demonstrations to support the above information are included herein as *Figures 1 through 8*, which are described as follows:

Figure 1 – OSE Groundwater Well Location Map

Figure 1A – BLM Aquifer Map

Figure 2 – USGS Geologic Map

Figure 3 – Municipality and Freshwater Field Map

Figure 4 – New Mexico Mining and Mineral Division Active Mine

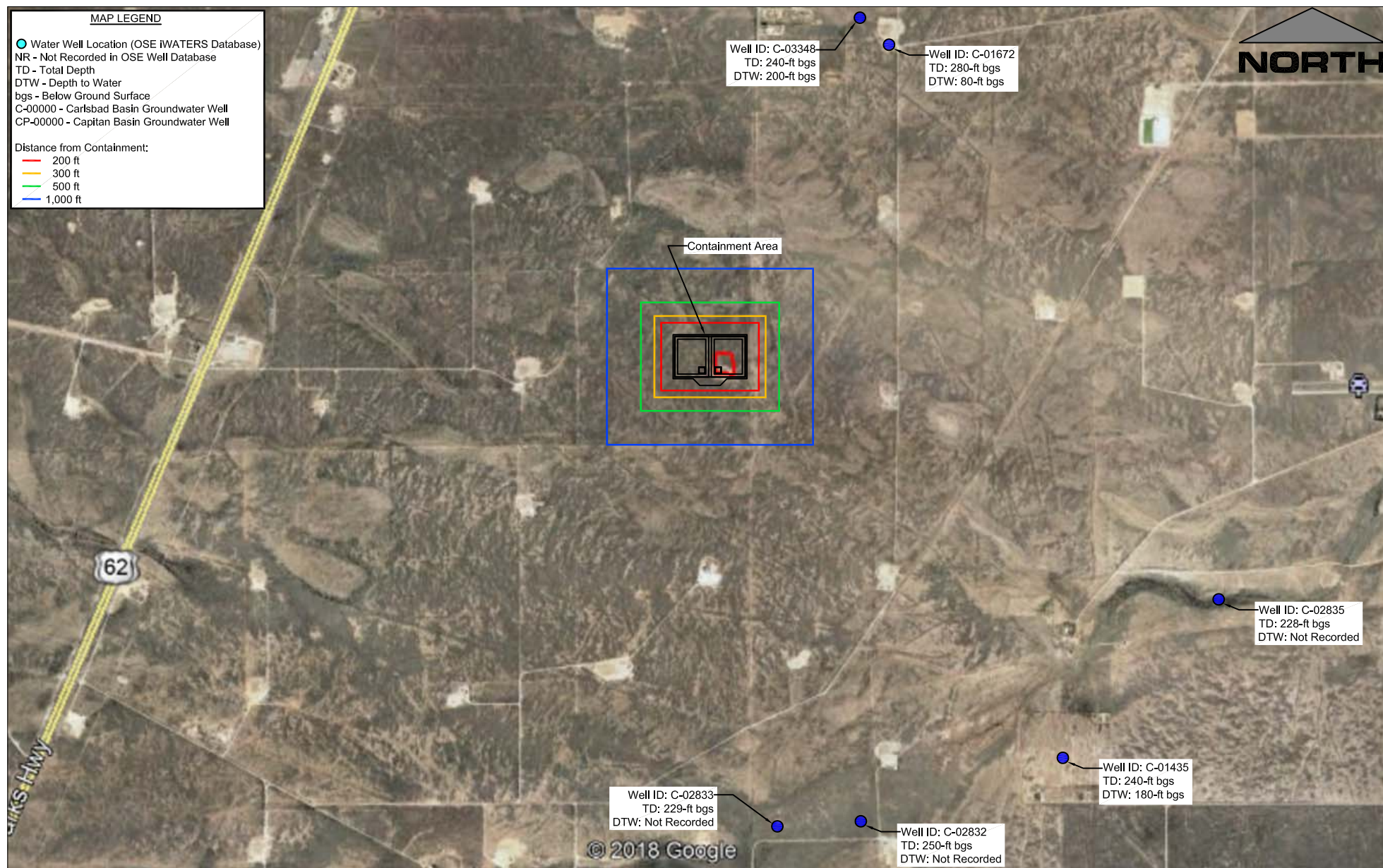
Figure 5 – BLM Karst Potential Map

Figure 6 – FEMA Floodplains Map

Figure 7 – Distance from Municipalities, Structures, and Wells

Figure 8 – Wetlands Location Map

Additionally, the location maps and logs for above-referenced geotechnical borings performed by Terracon are enclosed.



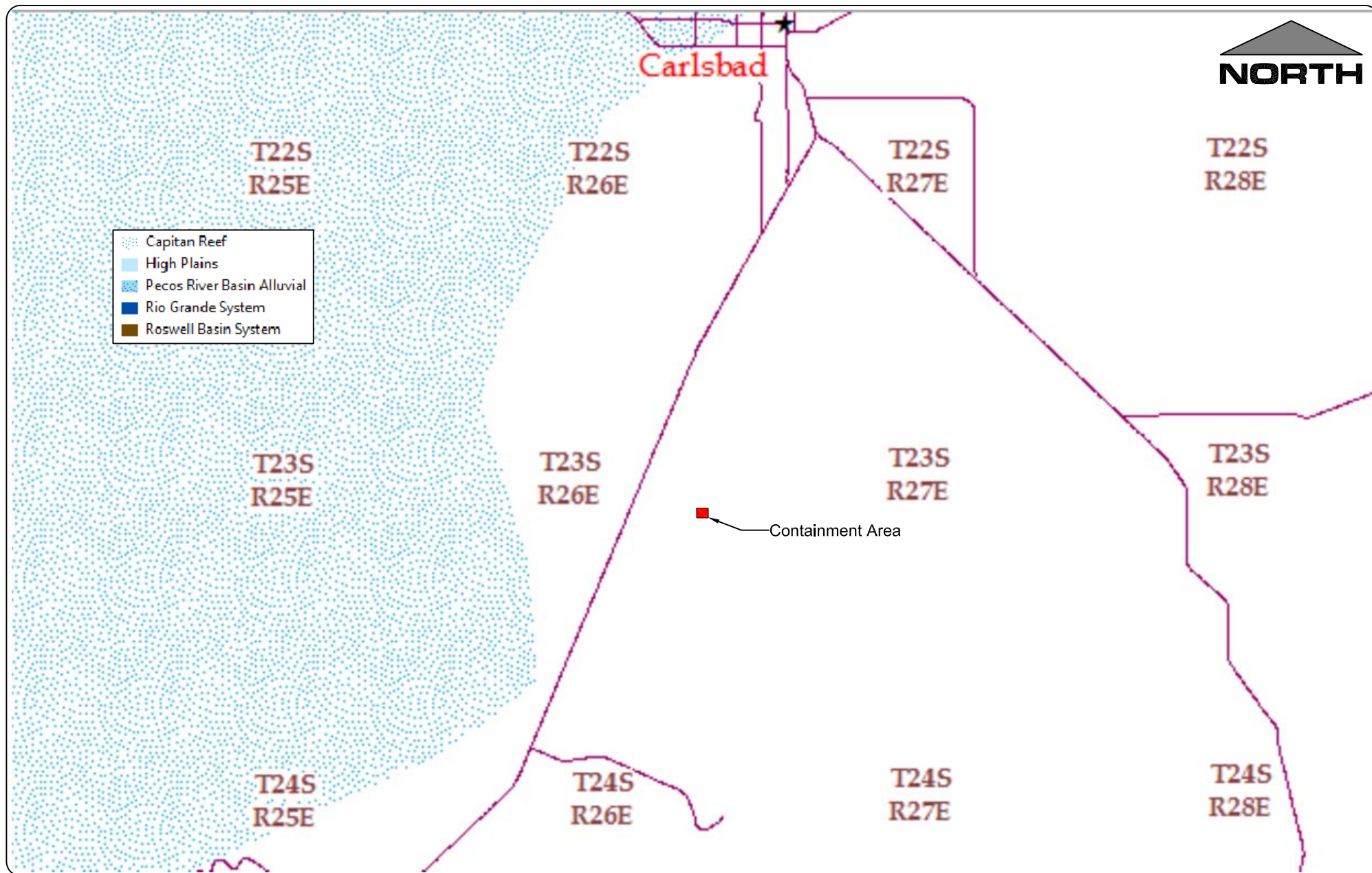
3 Bear Energy

Figure 1 - OSE Groundwater Well Location Map

Project No. 018183-00

960 Recycle Facility C147 Registration Package
E/2, SE/4, Section 23, T23S, R26E, Eddy County, New Mexico





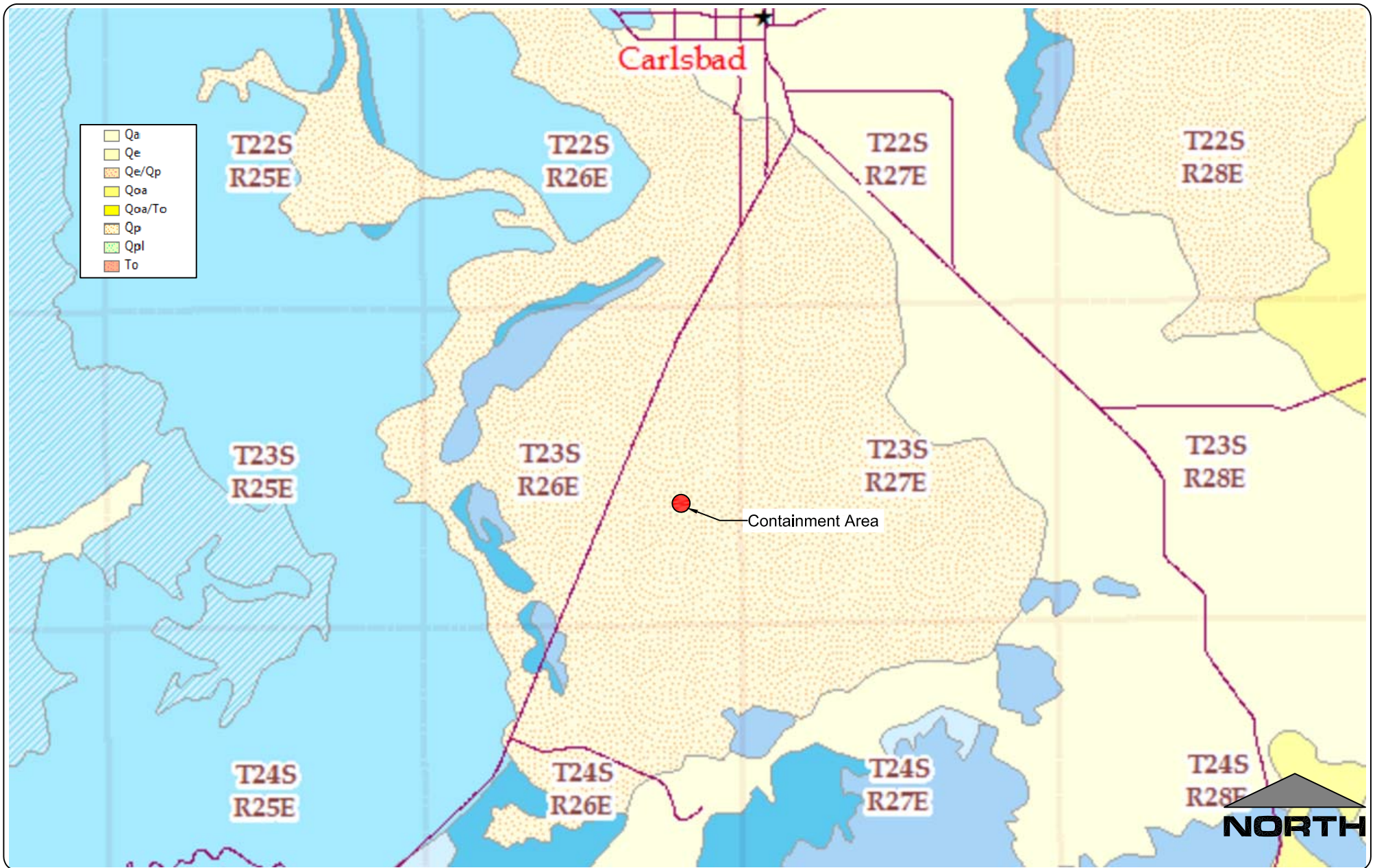
3 Bear Energy

Figure 1A - BLM Aquifer Map

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960 Recycle Facility C147 Registration Package
E/2, SE/4, Section 23, T23S, R26E, Eddy County, New Mexico



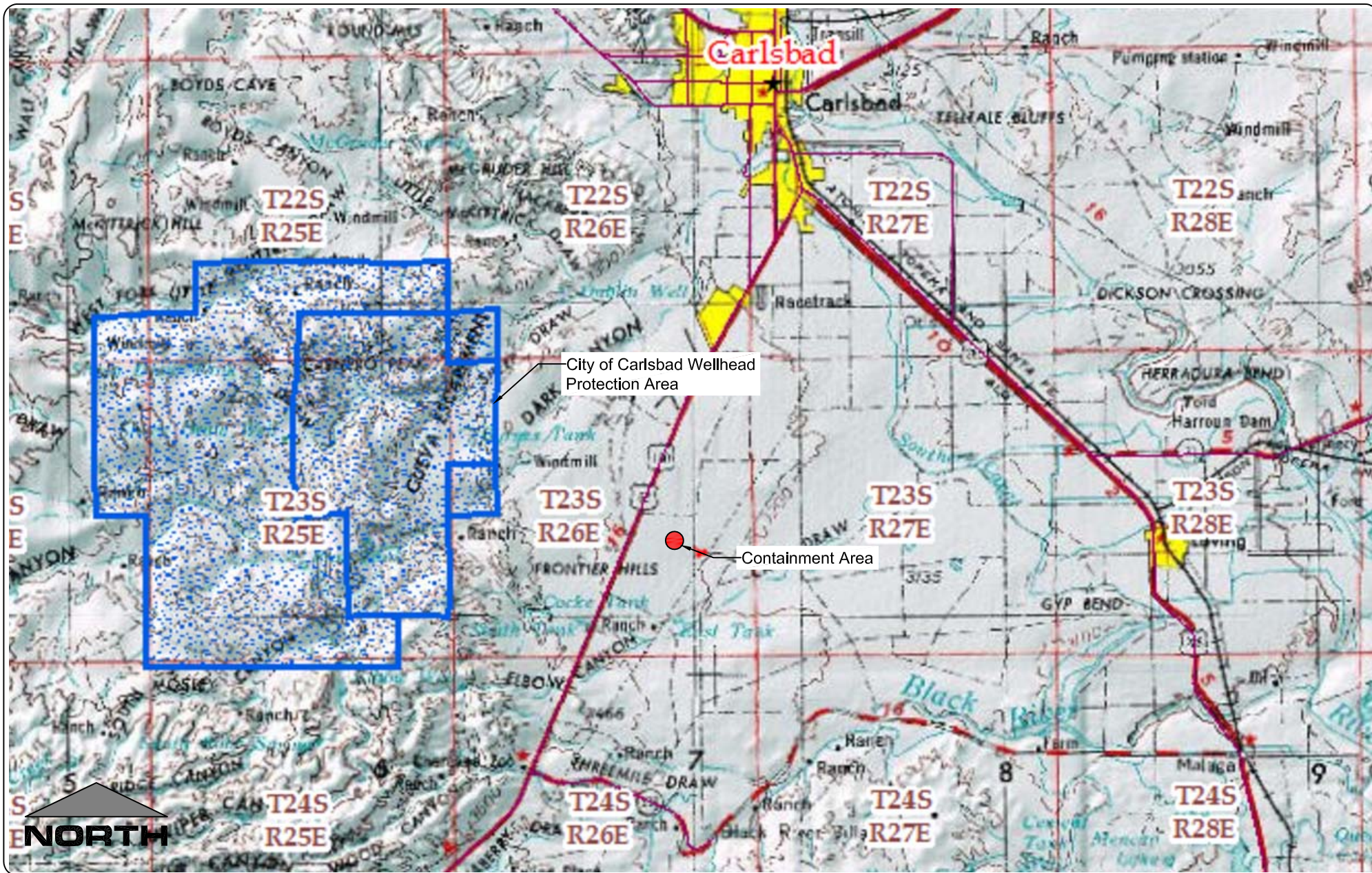


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Figure 2 - USGS Geologic Map
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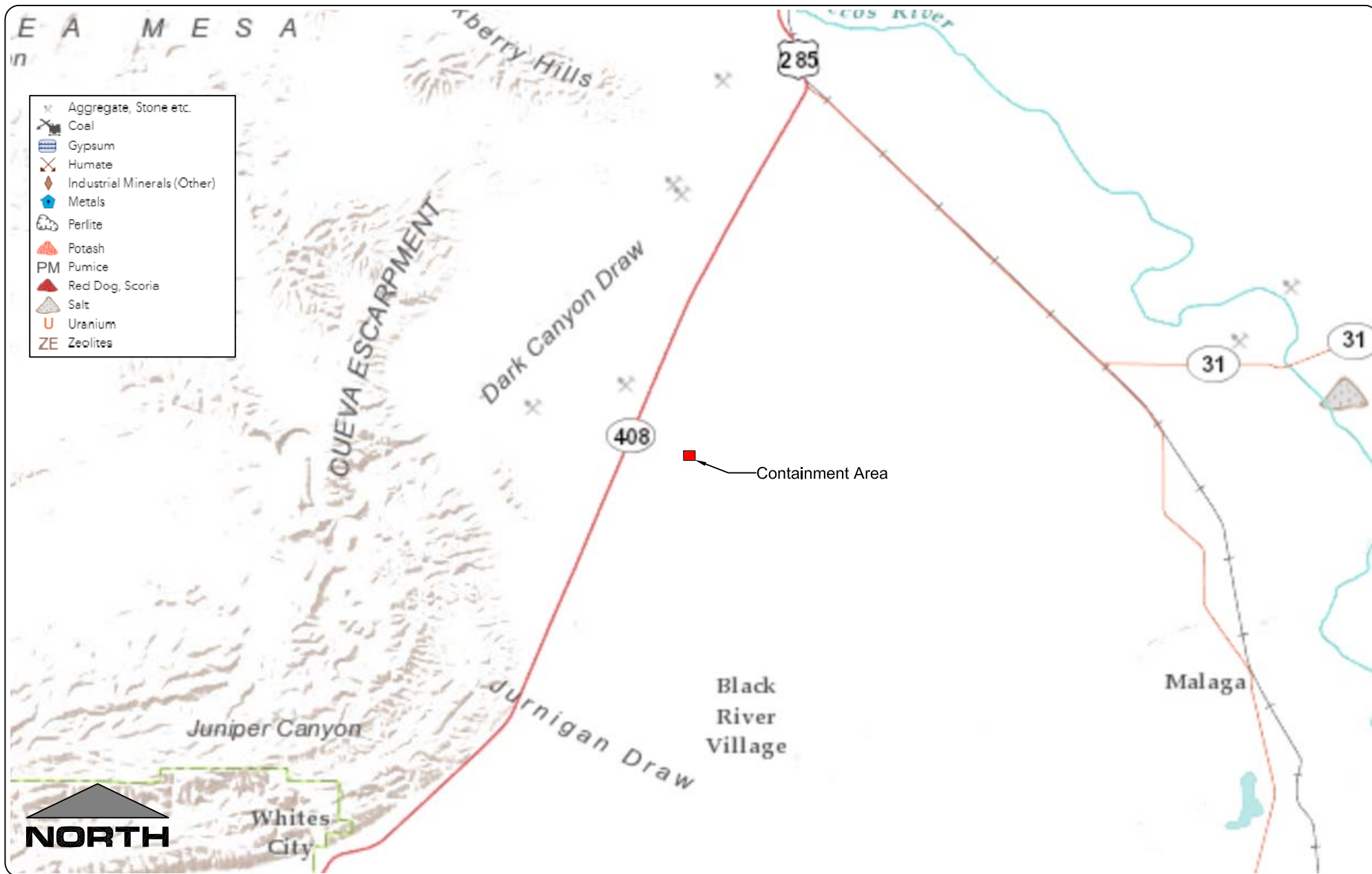
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Figure 3 - Municipality and Freshwater Fields Map

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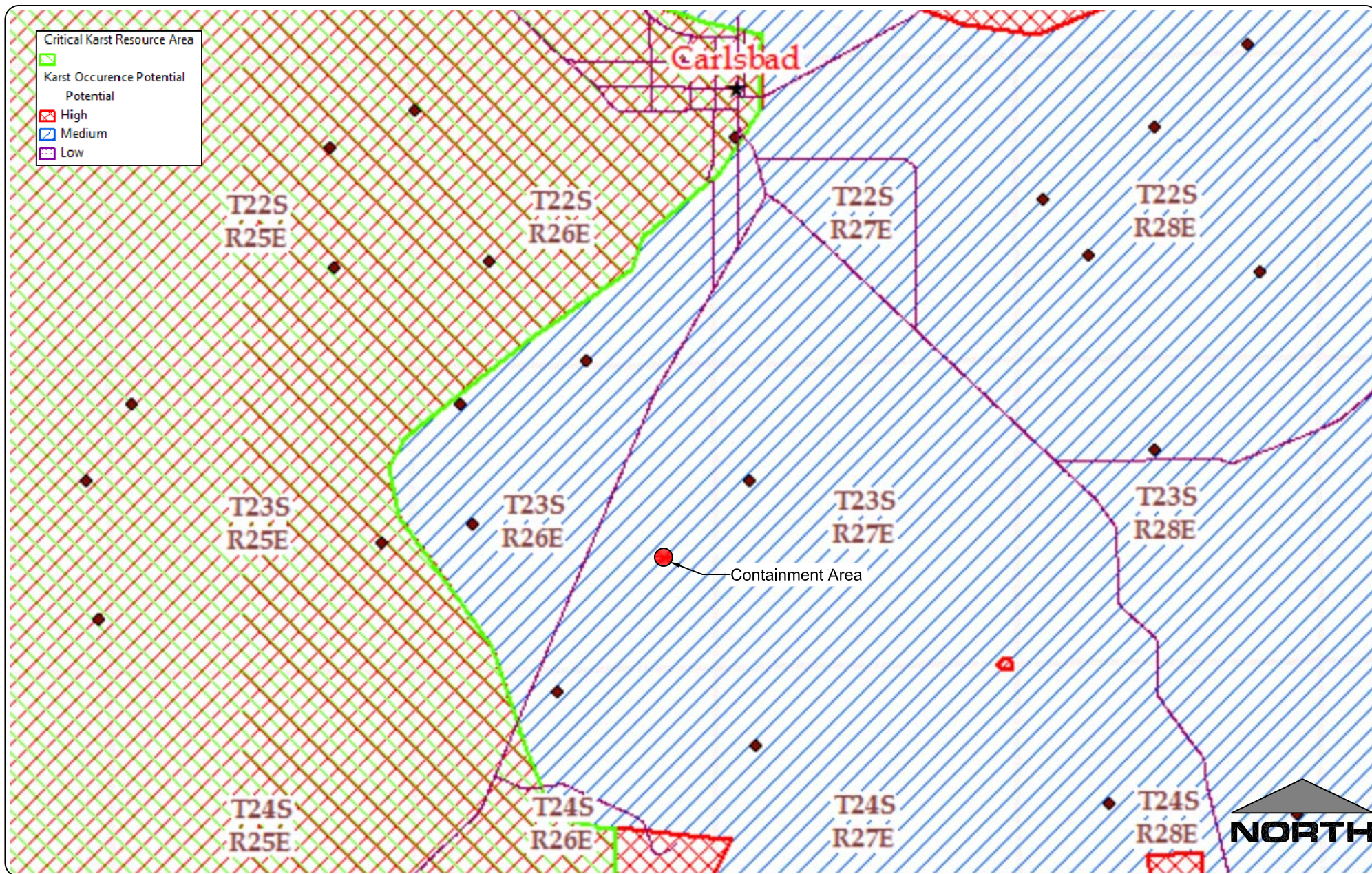
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Figure 4 - NM Mining and Minerals Division- Active Mines

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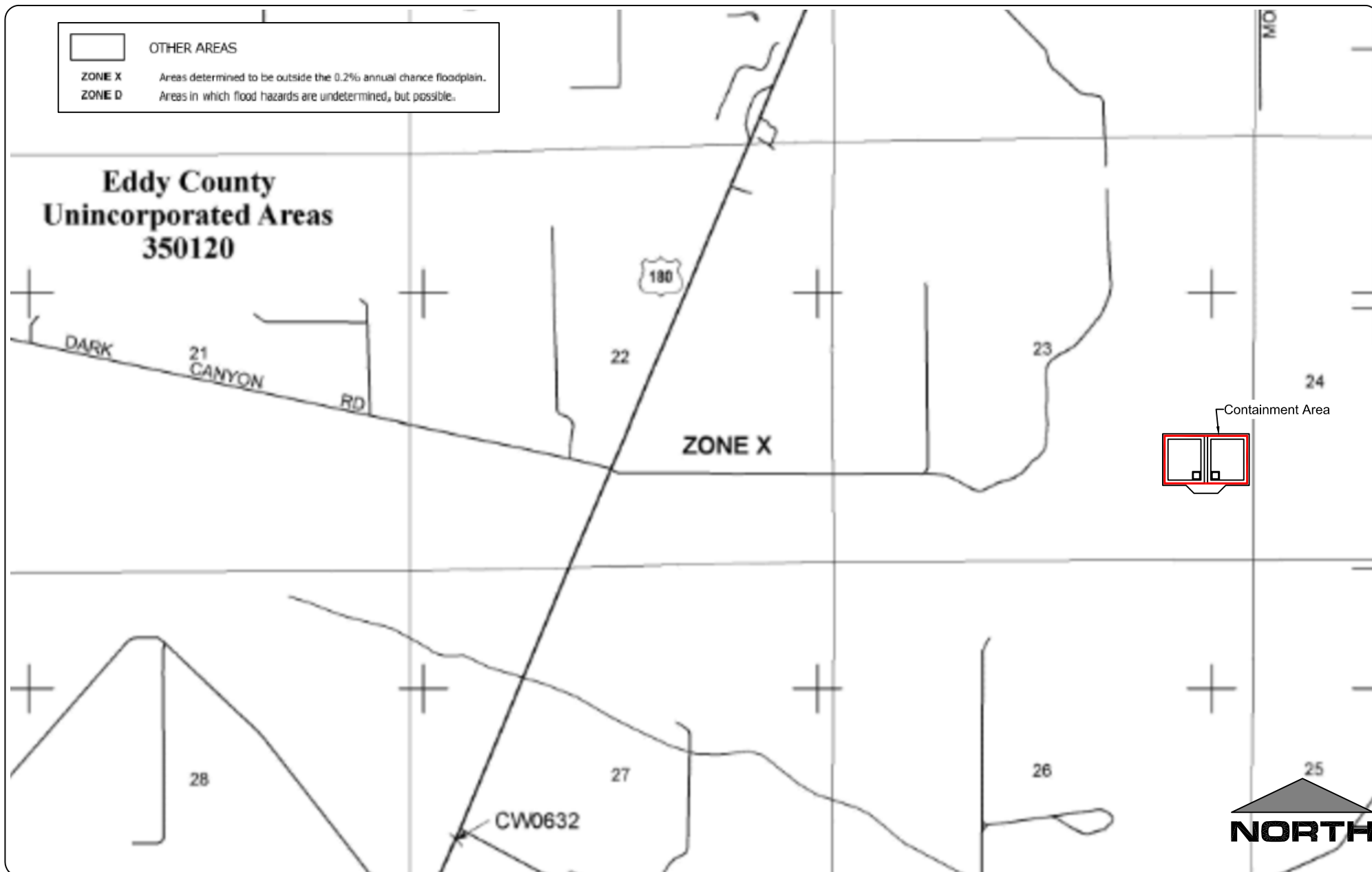
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Figure 5 - BLM Karst Potential Map

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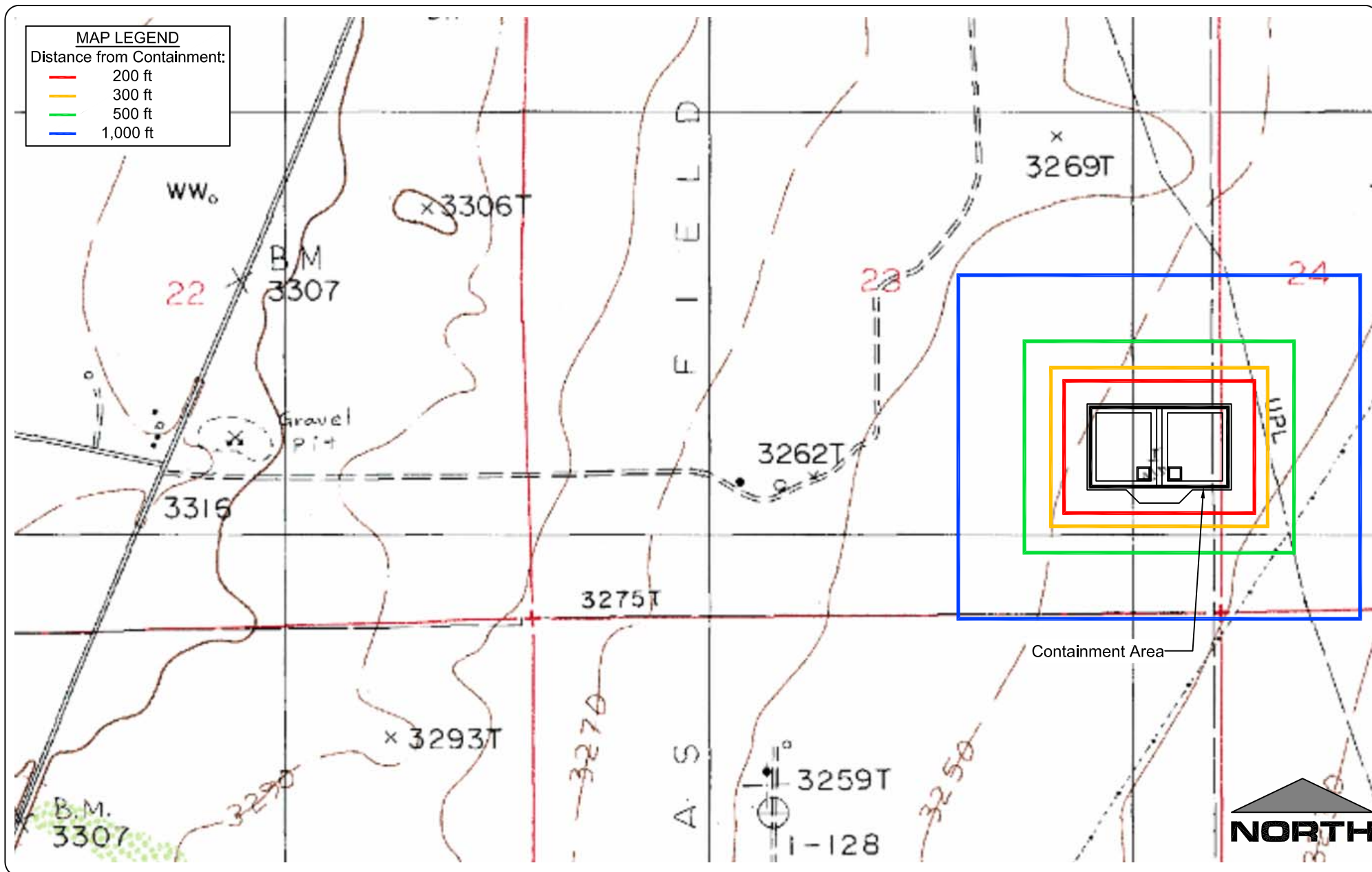
3 Bear Energy

Figure 6 - FEMA Map

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Figure 7 - Distance From Municipalities, Structures, and Wells

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- Estuarine and Marine
- Deepwater
- Estuarine and Marine Wetland
- Freshwater Emergent Wetland
- Freshwater Forested/Shrub Wetland
- Freshwater Pond
- Lake
- Other
- Riverine



3 Bear Energy

Figure 8 - Wetlands Location Map

Project No. 018183-00

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E/2, SE/4, Section 23, T23S, R26E, Eddy County, New Mexico



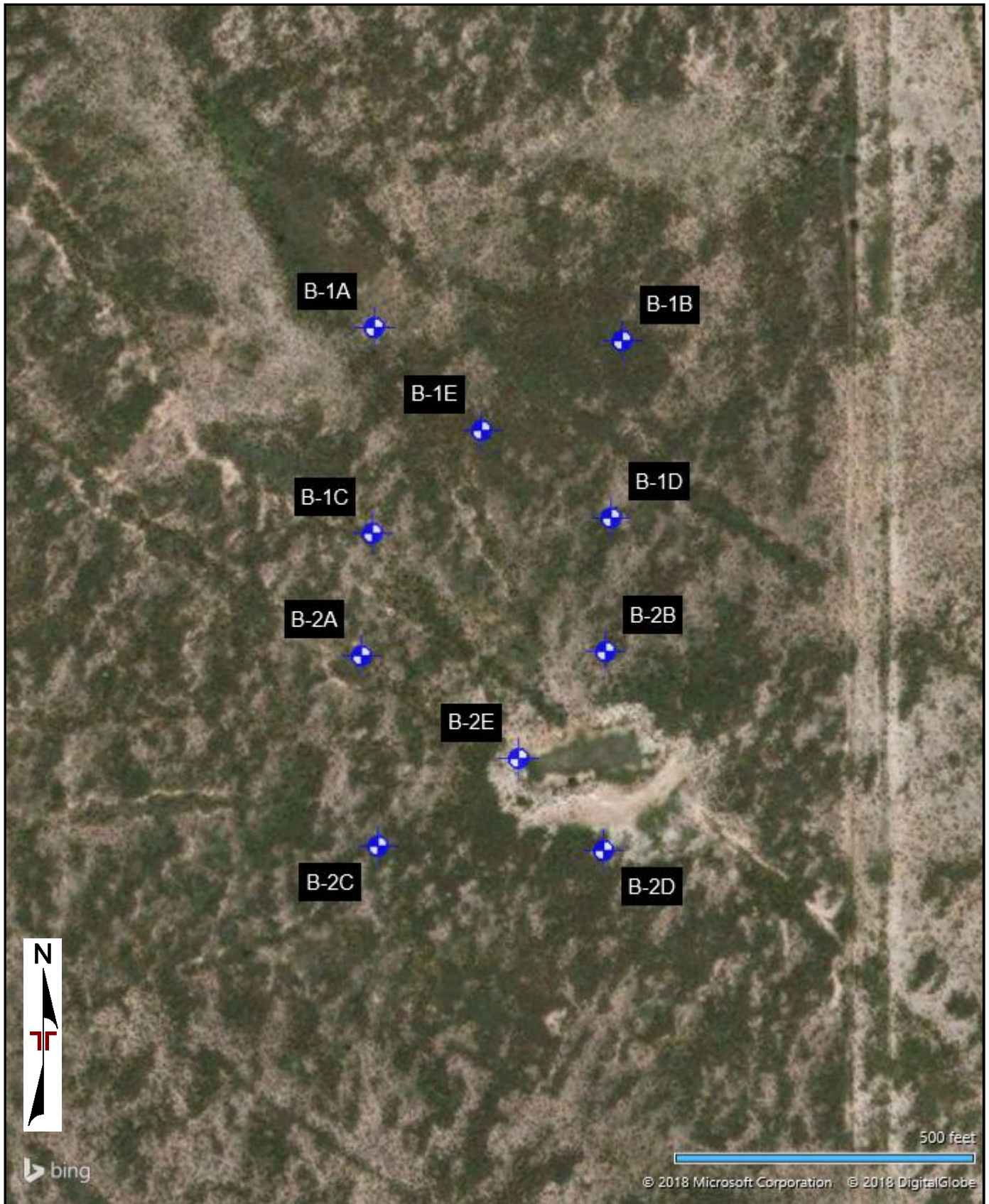


DIAGRAM IS FOR GENERAL LOCATION ONLY, AND IS NOT INTENDED FOR CONSTRUCTION PURPOSES

AERIAL PHOTOGRAPHY PROVIDED BY MICROSOFT BING MAPS

| | |
|------------------|-----------|
| Project Manager: | JDC |
| Drawn by: | DC |
| Checked by: | JDC |
| Approved by: | JDC |
| Project No. | 68185059 |
| Scale: | AS SHOWN |
| File Name: | FIGURES |
| Date: | 6/13/2018 |

Terracon
4450 Bataan Memorial E
Las Cruces, NM 88011-6000

| BORING LOCATION PLAN |
|--|
| Eddy Co. 960 Water Impoundment Facility East of Hwy 62 off of Dark Canyon Road Eddy County, NM |

| |
|---------|
| Exhibit |
| A-2 |

Geotechnical Engineering Report

Eddy Co. 960 Water Impoundment Facility ■ Eddy County, New Mexico

June 27, 2018 ■ Terracon Project No. 68185059



Field Exploration Description

A total of ten (10) test borings were drilled at the site on June 19 and 20, 2018. The borings were drilled to depths of about 14 to 54 feet below the ground surface at the approximate locations shown on the attached Boring Location Plan, Exhibit A-2. The test borings were located as follows:

| Borings | Location | Depth (feet) |
|--------------|----------------------------------|--------------|
| B-1A to B-1E | Approximate North Pond Footprint | 21-1/2 to 40 |
| B-2A to B-2E | Approximate South Pond Footprint | 14 to 54 |

The test borings were advanced with a truck-mounted CME-75 drill rig utilizing 8-inch diameter hollow-stem augers.

The borings were located in the field by using the proposed site plan and an aerial photograph of the site, measuring from existing property lines and using a hand-held GPS unit. The accuracy of boring locations should only be assumed to the level implied by the method used.

Lithologic logs of each boring were recorded by the field engineer during the drilling operations. At selected intervals, samples of the subsurface materials were taken by driving split-spoon or ring-barrel samplers. Bulk samples of subsurface materials were also obtained.

Penetration resistance measurements were obtained by driving the split-spoon and ring-barrel samplers into the subsurface materials with a 140-pound automatic hammer falling 30 inches. The penetration resistance value is a useful index in estimating the consistency or relative density of materials encountered.

A CME automatic SPT hammer was used to advance the split-barrel sampler in the borings performed on this site. The effect of the automatic hammer's efficiency has been considered in the interpretation and analysis of the subsurface information for this report.

Groundwater conditions were evaluated in the borings at the time of site exploration. For safety purposes, we backfilled the borings with auger cuttings immediately after drilling operations.

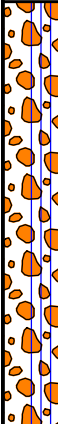
BORING LOG NO. B-1A

Page 1 of 1

PROJECT: Eddy Co. 960 Water Impoundment Facility


CLIENT: EnviroTech Engineering & Consulting Inc
Enid, OK

SITE: East of Hwy 62 off of Dark Canyon Road
Eddy County, NM

| GRAPHIC LOG | LOCATION | See Exhibit A-2 | DEPTH (Ft.) | WATER LEVEL OBSERVATIONS | SAMPLE TYPE | FIELD TEST RESULTS | WATER CONTENT (%) | DRY UNIT WEIGHT (pcf) | ATTERBERG LIMITS | PERCENT FINES |
|---|---|-----------------|-------------|--------------------------|-------------|--------------------|-------------------|-----------------------|------------------|---------------|
| | | | | | | | | | LL-PL-PI | |
| | Latitude: 32.2877° Longitude: -104.2578° | | | | | | | | | |
| | Approximate Surface Elev: 3259 (Ft.) +/- | | | | | | | | | |
| | DEPTH | ELEVATION (Ft.) | | | | | | | | |
|  | POORLY GRADED GRAVEL WITH SILT (GP-GM) , light brown, very dense | | 5 | | | 20-50/1" | | | | |
| | tan | | 10 | | | 50/4" | | | | |
| | | | 15 | | | 25-40-43 N=83 | | | | |
| | | | 20 | | | 20-16-10 N=26 | 1 | | NP | 6 |
| | 21.5 medium dense | | 3237.5+/- | | | | | | | |
| | Boring Terminated at 21.5 Feet | | | | | | | | | |

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

| | | | |
|---|---|----------------------------|------------------------------|
| Advancement Method: Hollow Stem Auger | See Exhibit A-3 for description of field procedures. See Appendix B for description of laboratory procedures and additional data (if any). See Appendix C for explanation of symbols and abbreviations. | Notes: | |
| Abandonment Method: Boring backfilled with auger cuttings upon completion. | | | |
| WATER LEVEL OBSERVATIONS |  4450 Bataan Memorial E Las Cruces, NM | Boring Started: 06-19-2018 | Boring Completed: 06-19-2018 |
| | | Drill Rig: CME 75 | Driller: Terra Test |
| | | Project No.: 68185059 | Exhibit: A-4 |
| | | | |
| | | | |

THIS BORING LOG IS NOT VALID IF SEPARATED FROM ORIGINAL REPORT. GEO SMART LOG-NO WELL 68185059 EDDY CO. 960 WATE.GPJ TERRACON DATATEMPLATE.GDT 6/27/18

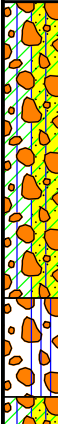
BORING LOG NO. B-1B

Page 1 of 1

PROJECT: Eddy Co. 960 Water Impoundment Facility

CLIENT: EnviroTech Engineering & Consulting Inc
Enid, OK

SITE: East of Hwy 62 off of Dark Canyon Road
Eddy County, NM

| GRAPHIC LOG | LOCATION See Exhibit A-2 | | DEPTH (Ft.) | WATER LEVEL OBSERVATIONS | SAMPLE TYPE | FIELD TEST RESULTS | WATER CONTENT (%) | DRY UNIT WEIGHT (pcf) | ATTERBERG LIMITS | PERCENT FINES |
|---|--|-----------|-----------------|--------------------------|-------------|--------------------|-------------------|-----------------------|------------------|---------------|
| | Latitude: 32.2877° Longitude: -104.2564° | | | | | | | | LL-PL-PI | |
| Approximate Surface Elev: 3259 (Ft.) +/- | | DEPTH | ELEVATION (Ft.) | | | | | | | |
|  | SILTY CLAYEY GRAVEL WITH SAND (GC-GM) , tan, very dense | | | | | | | | | |
| | | | 5 | | X | 8-21-43 N=64 | 4 | | 22-18-4 | 47 |
| | carbonate indurated | | 10 | | X | 16-41-50/4" | | | | |
| | 15.0 | 3244+/- | 15 | | X | 28-50/2" | | | | |
| | POORLY GRADED GRAVEL WITH SILT (GP-GM) , white, very dense, carbonate indurated | | | | | | | | | |
| | 20.0 | 3239+/- | | | | | | | | |
| | 21.5 | 3237.5+/- | 20 | | X | 13-27-43 N=70 | | | | |
| Boring Terminated at 21.5 Feet | | | | | | | | | | |

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Hollow Stem Auger

See Exhibit A-3 for description of field procedures.
See Appendix B for description of laboratory procedures and additional data (if any).
See Appendix C for explanation of symbols and abbreviations.

Notes:

Abandonment Method:
Boring backfilled with auger cuttings upon completion.

WATER LEVEL OBSERVATIONS

Terracon
4450 Bataan Memorial E
Las Cruces, NM

Boring Started: 06-19-2018

Boring Completed: 06-19-2018

Drill Rig: CME 75

Driller: Terra Test

Project No.: 68185059

Exhibit: A-5

BORING LOG NO. B-1C

Page 1 of 1

PROJECT: Eddy Co. 960 Water Impoundment Facility

CLIENT: EnviroTech Engineering & Consulting Inc
Enid, OK

SITE: East of Hwy 62 off of Dark Canyon Road
Eddy County, NM

| GRAPHIC LOG | LOCATION See Exhibit A-2 | | DEPTH (Ft.) | WATER LEVEL OBSERVATIONS | SAMPLE TYPE | FIELD TEST RESULTS | WATER CONTENT (%) | DRY UNIT WEIGHT (pcf) | ATTERBERG LIMITS | PERCENT FINES |
|--|---|--------------------|-----------------|--------------------------|-------------|--------------------|-------------------|-----------------------|------------------|---------------|
| | Latitude: 32.2868° Longitude: -104.2578° | | | | | | | | LL-PL-PI | |
| Approximate Surface Elev: 3259 (Ft.) +/- | | | ELEVATION (Ft.) | | | | | | | |
| DEPTH | | | | | | | | | | |
| | LEAN CLAY WITH SAND (CL) , brown, hard | | 5 | | | 4-15-17 N=32 | 6 | | 27-17-10 | 84 |
| | 10.0 | 3249+/- | 10 | | | 35-50/5" | | | | |
| | | | 15 | | | 16-50/3" | | | | |
| | 21.5 | white 3237.5+/- | 20 | | | 20-33-45 N=78 | | | | |
| Boring Terminated at 21.5 Feet | | | | | | | | | | |

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Hollow Stem Auger

See Exhibit A-3 for description of field procedures.
See Appendix B for description of laboratory procedures and additional data (if any).
See Appendix C for explanation of symbols and abbreviations.

Notes:

Abandonment Method:
Boring backfilled with auger cuttings upon completion.

WATER LEVEL OBSERVATIONS

Terracon
4450 Bataan Memorial E
Las Cruces, NM

Boring Started: 06-19-2018

Boring Completed: 06-19-2018

Drill Rig: CME 75

Driller: Terra Test

Project No.: 68185059

Exhibit: A-6


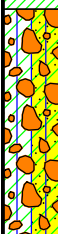
BORING LOG NO. B-1D

Page 1 of 1

PROJECT: Eddy Co. 960 Water Impoundment Facility

CLIENT: EnviroTech Engineering & Consulting Inc
Enid, OK

SITE: East of Hwy 62 off of Dark Canyon Road
Eddy County, NM

| GRAPHIC LOG | LOCATION See Exhibit A-2 | | DEPTH (Ft.) | WATER LEVEL OBSERVATIONS | SAMPLE TYPE | FIELD TEST RESULTS | WATER CONTENT (%) | DRY UNIT WEIGHT (pcf) | ATTERBERG LIMITS | PERCENT FINES |
|---|---|-----------|-------------|--------------------------|-------------|--------------------|-------------------|-----------------------|------------------|---------------|
| | Approximate Surface Elev: 3259 (Ft.) +/- ELEVATION (Ft.) | | | | | | | | LL-PL-PI | |
| | DEPTH | | | | | | | | | |
|  | LEAN CLAY (CL) , brown, hard | | 5 | | | | | | | |
| | | | | X | | 6-17-19 N=36 | 7 | | 33-18-15 | 88 |
|  | 10.0 | 3249+/- | 10 | | | 50/3" | | | | |
| | | | 15 | | | 50/6" | | | | |
| | | | 20 | | | 28-45-50/5" | | | | |
| | 21.5 | 3237.5+/- | | | | | | | | |
| Boring Terminated at 21.5 Feet | | | | | | | | | | |

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Hollow Stem Auger

See Exhibit A-3 for description of field procedures.
See Appendix B for description of laboratory procedures and additional data (if any).
See Appendix C for explanation of symbols and abbreviations.

Notes:

Abandonment Method:
Boring backfilled with auger cuttings upon completion.

WATER LEVEL OBSERVATIONS

Terracon
4450 Bataan Memorial E
Las Cruces, NM

Boring Started: 06-19-2018

Boring Completed: 06-19-2018

Drill Rig: CME 75

Driller: Terra Test

Project No.: 68185059

Exhibit: A-7

THIS BORING LOG IS NOT VALID IF SEPARATED FROM ORIGINAL REPORT. GEO SMART LOG-NO WELL 68185059 EDDY CO. 960 WATE.GPJ TERRACON DATATEMPLATE.GDT 6/27/18


BORING LOG NO. B-1E

Page 1 of 1

PROJECT: Eddy Co. 960 Water Impoundment Facility

CLIENT: EnviroTech Engineering & Consulting Inc
Enid, OK

SITE: East of Hwy 62 off of Dark Canyon Road
Eddy County, NM

| GRAPHIC LOG | LOCATION See Exhibit A-2 Latitude: 32.2873° Longitude: -104.2572° | | DEPTH (Ft.) | WATER LEVEL OBSERVATIONS | SAMPLE TYPE | FIELD TEST RESULTS | WATER CONTENT (%) | DRY UNIT WEIGHT (pcf) | ATTERBERG LIMITS | PERCENT FINES |
|--|--|-----------------|-------------|-----------------------------|-------------|-----------------------|----------------------|--------------------------|---------------------|---------------|
| | DEPTH | ELEVATION (Ft.) | | | | | | | LL-PL-PI | |
|  | CLAYEY GRAVEL WITH SAND (GC) , tan, very dense, carbonate indurated | | | | | 12-40-50/5" | | | | |
| | | | 5 | | | 11-39-46 N=85 | 2 | | 27-16-11 | 22 |
| | light brown | | | | | 16-33-50 N=83 | | | | |
| | tan | | 10 | | | 20-50 | | | | |
| | | | | | | 25-50/5" | | | | |
| | light brown | | 15 | | | 11-35-40 N=75 | | | | |
| | | | | | | 20-50/4" | | | | |
| | dense | | 20 | | | 18-22-24 N=46 | | | | |
| | | | 25 | | | | | | | |
| | | | 30 | | | | | | | |
| | 40.0 | 3219+/- | 40 | | | | | | | |
| Auger refusal due to very dense cemented soils at 40 Feet | | | | | | | | | | |

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Hollow Stem Auger

See Exhibit A-3 for description of field procedures.
See Appendix B for description of laboratory procedures and additional data (if any).
See Appendix C for explanation of symbols and abbreviations.

Notes:

Abandonment Method:
Boring backfilled with auger cuttings upon completion.

WATER LEVEL OBSERVATIONS

Terracon
4450 Bataan Memorial E
Las Cruces, NM

Boring Started: 06-19-2018

Boring Completed: 06-19-2018

Drill Rig: CME 75

Driller: Terra Test

Project No.: 68185059

Exhibit: A-8

BORING LOG NO. B-2A

Page 1 of 1

PROJECT: Eddy Co. 960 Water Impoundment Facility

CLIENT: EnviroTech Engineering & Consulting Inc
Enid, OK

SITE: East of Hwy 62 off of Dark Canyon Road
Eddy County, NM

| GRAPHIC LOG | LOCATION See Exhibit A-2 Latitude: 32.2862° Longitude: -104.2578° | | DEPTH (Ft.) | WATER LEVEL OBSERVATIONS | SAMPLE TYPE | FIELD TEST RESULTS | WATER CONTENT (%) | DRY UNIT WEIGHT (pcf) | ATTEBERG LIMITS | PERCENT FINES |
|--|---|---|-------------|-----------------------------|-------------|-----------------------|----------------------|--------------------------|--------------------|---------------|
| | DEPTH | Approximate Surface Elev: 3259 (Ft.) +/- ELEVATION (Ft.) | | | | | | | LL-PL-PI | |
| | LEAN CLAY WITH SAND (CL) , brown, hard | | 5 | | | 14-16-17 N=33 | 7 | | 28-18-10 | 82 |
| | 10.0 | 3249+/- | 10 | | | 34-50/3" | | | | |
| | CLAYEY GRAVEL WITH SAND (GC) , light brown, very dense, carbonate indurated | | 15 | | | 26-50/4" | | | | |
| | 19.0 | 3240+/- | | | | | | | | |
| Auger refusal due to very dense cemented soils at 19 Feet | | | | | | | | | | |

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

| | | | |
|---|---|------------------------------|--|
| Advancement Method: Hollow Stem Auger | See Exhibit A-3 for description of field procedures. See Appendix B for description of laboratory procedures and additional data (if any). See Appendix C for explanation of symbols and abbreviations. | Notes: | |
| Abandonment Method: Boring backfilled with auger cuttings upon completion. | | | |
| WATER LEVEL OBSERVATIONS | 4450 Bataan Memorial E Las Cruces, NM | Boring Started: 06-19-2018 | |
| | | Boring Completed: 06-19-2018 | |
| | | Drill Rig: CME 75 | |
| | | Driller: Terra Test | |
| | | Project No.: 68185059 | |
| | | Exhibit: A-9 | |

THIS BORING LOG IS NOT VALID IF SEPARATED FROM ORIGINAL REPORT. GEO SMART LOG-NO WELL 68185059 EDDY CO. 960 WATE.GPJ TERRACON DATATEMPLATE.GDT 6/27/18


BORING LOG NO. B-2B

Page 1 of 1

PROJECT: Eddy Co. 960 Water Impoundment Facility


CLIENT: EnviroTech Engineering & Consulting Inc
Enid, OK

SITE: East of Hwy 62 off of Dark Canyon Road
Eddy County, NM

| GRAPHIC LOG | LOCATION | See Exhibit A-2 | DEPTH (Ft.) | WATER LEVEL OBSERVATIONS | SAMPLE TYPE | FIELD TEST RESULTS | WATER CONTENT (%) | DRY UNIT WEIGHT (pcf) | ATTEBERG LIMITS | PERCENT FINES |
|---|--|--|-------------|--|-------------|--------------------|-------------------|-----------------------|-----------------|---------------|
| | Latitude: 32.2863° Longitude: -104.2565° | | | | | | | | LL-PL-PI | |
| DEPTH | | ELEVATION (Ft.) | | Approximate Surface Elev: 3259 (Ft.) +/- | | | | | | |
|  | CLAYEY GRAVEL WITH SAND (GC) , light brown, very dense, carbonate indurated | | 5 | | | 23-38-38 N=76 | 2 | | 25-15-10 | 25 |
| | | | 10 | | | 50/5" | | | | |
| | 14.0 | | 3245+/- | | | | | | | |
| | | Auger refusal due to very dense cemented soils at 14 Feet | | | | | | | | |

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

| | | | |
|---|---|----------------------------|------------------------------|
| Advancement Method: Hollow Stem Auger | See Exhibit A-3 for description of field procedures. See Appendix B for description of laboratory procedures and additional data (if any). See Appendix C for explanation of symbols and abbreviations. | Notes: | |
| Abandonment Method: Boring backfilled with auger cuttings upon completion. | | | |
| WATER LEVEL OBSERVATIONS |  <p>4450 Bataan Memorial E Las Cruces, NM</p> | Boring Started: 06-19-2018 | Boring Completed: 06-19-2018 |
| | | Drill Rig: CME 75 | Driller: Terra Test |
| | | Project No.: 68185059 | Exhibit: A-10 |
| | | | |

THIS BORING LOG IS NOT VALID IF SEPARATED FROM ORIGINAL REPORT. GEO SMART LOG-NO WELL 68185059 EDDY CO. 960 WATE.GPJ TERRACON_DATATEMPLATE.GDT 6/27/18

BORING LOG NO. B-2C

Page 1 of 1

PROJECT: Eddy Co. 960 Water Impoundment Facility

CLIENT: EnviroTech Engineering & Consulting Inc
Enid, OK

SITE: East of Hwy 62 off of Dark Canyon Road
Eddy County, NM

| GRAPHIC LOG | LOCATION See Exhibit A-2 Latitude: 32.2854° Longitude: -104.2577° | | DEPTH (Ft.) | WATER LEVEL OBSERVATIONS | SAMPLE TYPE | FIELD TEST RESULTS | WATER CONTENT (%) | DRY UNIT WEIGHT (pcf) | ATTERBERG LIMITS | PERCENT FINES |
|---------------------------------------|--|--|-------------|--------------------------|-------------|--------------------|-------------------|-----------------------|------------------|---------------|
| | DEPTH | ELEVATION (Ft.) | | | | | | | LL-PL-PI | |
| | | Approximate Surface Elev: 3259 (Ft.) +/- | | | | | | | | |
| | | | 5 | | | 13-24-19 N=43 | | | | |
| | 10.0 | 3249+/- | 10 | | | 7-10-50/4" | | | | |
| | 15.0 | 3244+/- | 15 | | | 26-25-14 N=39 | 1 | | 22-13-9 | 9 |
| | 21.5 | 3237.5+/- | 20 | | | 13-25-23 N=48 | | | | |
| Boring Terminated at 21.5 Feet | | | | | | | | | | |

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Hollow Stem Auger

See Exhibit A-3 for description of field procedures.
See Appendix B for description of laboratory procedures and additional data (if any).
See Appendix C for explanation of symbols and abbreviations.

Notes:

Abandonment Method:
Boring backfilled with auger cuttings upon completion.

WATER LEVEL OBSERVATIONS

Terracon
4450 Bataan Memorial E
Las Cruces, NM

Boring Started: 06-20-2018

Boring Completed: 06-20-2018

Drill Rig: CME 75

Driller: Terra Test

Project No.: 68185059

Exhibit: A-11

THIS BORING LOG IS NOT VALID IF SEPARATED FROM ORIGINAL REPORT. GEO SMART LOG-NO WELL 68185059 EDDY CO. 960 WATE.GPJ TERRACON.DATATEMPLATE.GDT 6/27/18

BORING LOG NO. B-2D

Page 1 of 1

PROJECT: Eddy Co. 960 Water Impoundment Facility

CLIENT: EnviroTech Engineering & Consulting Inc
Enid, OK

SITE: East of Hwy 62 off of Dark Canyon Road
Eddy County, NM

| GRAPHIC LOG | LOCATION See Exhibit A-2 | | DEPTH (Ft.) | WATER LEVEL OBSERVATIONS | SAMPLE TYPE | FIELD TEST RESULTS | WATER CONTENT (%) | DRY UNIT WEIGHT (pcf) | ATTERBERG LIMITS | PERCENT FINES |
|-------------|--|-----------------|-------------|--------------------------|-------------|--------------------|-------------------|-----------------------|------------------|---------------|
| | Latitude: 32.2853° Longitude: -104.2565° | | | | | | | | LL-PL-PI | |
| | Approximate Surface Elev: 3259 (Ft.) +/- | | | | | | | | | |
| | DEPTH | ELEVATION (Ft.) | | | | | | | | |
| | LEAN CLAY WITH SAND (CL) , brown, very stiff | | 5 | | | | | | | |
| | | | 10.0 | | | 4-13-17 N=30 | | | | |
| | WELL GRADED GRAVEL WITH CLAY AND SAND (GW-GC) , light brown, very dense | | 10 | | | 30-50/3" | | | | |
| | medium dense | | 15 | | | 10-10-16 N=26 | 1 | | 31-13-18 | 5 |
| | | | 20 | | | 29-50/3" | | | | |
| | POORLY GRADED GRAVEL WITH SILT (GP-GM) , white, very dense, carbonate indurated | | 20 | | | | | | | |
| | Boring Terminated at 21.5 Feet | | | | | | | | | |

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

| | | | |
|---|---|------------------------------|--|
| Advancement Method: Hollow Stem Auger | See Exhibit A-3 for description of field procedures. See Appendix B for description of laboratory procedures and additional data (if any). See Appendix C for explanation of symbols and abbreviations. | Notes: | |
| Abandonment Method: Boring backfilled with auger cuttings upon completion. | | | |
| WATER LEVEL OBSERVATIONS | 4450 Bataan Memorial E Las Cruces, NM | Boring Started: 06-20-2018 | |
| | | Boring Completed: 06-20-2018 | |
| | | Drill Rig: CME 75 | |
| | | Driller: Terra Test | |
| | | Project No.: 68185059 | |
| | | Exhibit: A-12 | |

THIS BORING LOG IS NOT VALID IF SEPARATED FROM ORIGINAL REPORT. GEO SMART LOG-NO WELL 68185059 EDDY CO. 960 WATE.GPJ TERRACON.DATATEMPLATE.GDT 6/27/18

BORING LOG NO. B-2E

Page 1 of 1

PROJECT: Eddy Co. 960 Water Impoundment Facility

CLIENT: EnviroTech Engineering & Consulting Inc
Enid, OK

SITE: East of Hwy 62 off of Dark Canyon Road
Eddy County, NM

THIS BORING LOG IS NOT VALID IF SEPARATED FROM ORIGINAL REPORT. GEO SMART LOG-NO WELL 68185059 EDDY CO. 960 WATE GPJ TERRACON DATATEMPLATE.GDT 6/27/18

| GRAPHIC LOG | LOCATION See Exhibit A-2 | | DEPTH (Ft.) | WATER LEVEL OBSERVATIONS | SAMPLE TYPE | FIELD TEST RESULTS | WATER CONTENT (%) | DRY UNIT WEIGHT (pcf) | ATTERBERG LIMITS | | PERCENT FINES |
|--|---|-----------------|-------------|--------------------------|-------------|--------------------|-------------------|-----------------------|------------------|----|---------------|
| | Latitude: 32.2858° Longitude: -104.257° | | | | | | | | LL-PL-PI | | |
| | DEPTH | ELEVATION (Ft.) | | | | | | | | | |
| | LEAN CLAY (CL) , brown | | | | | | | | | | |
| | | | | | | | | | | | |
| | very stiff | | 5 | | | | | | | | |
| | 7.5 | 3251.5+/- | | | | 9-14-13 N=27 | 7 | | 30-20-10 | 88 | |
| | SILTY CLAYEY GRAVEL WITH SAND (GC-GM) , tan, medium dense, carbonate indurated | | 10 | | | 10-13-14 N=27 | | | | | |
| | very dense | | | | | 40-50/4" | | | | | |
| | | | | | | | | | | | |
| | tan with yellow | | | | | 24-34-50 N=84 | | | | | |
| | tan, dense | | 15 | | | 18-32-34 N=66 | | | | | |
| | very dense | | | | | 34-30-50/3" | | | | | |
| | | | 20 | | | 50 | | | | | |
| | | | | | | | | | | | |
| | | 25 | | | | | | | | | |
| | | 30 | | | | | | | | | |
| | | 35 | | | | | | | | | |
| | | 40 | | | | | | | | | |
| | | 45 | | | | | | | | | |
| | | 50 | | | | | | | | | |
| | | | | | | | | | | | |
| | 54.0 | 3205+/- | | | | | | | | | |
| Auger refusal due to very dense cemented soils at 54 Feet | | | | | | | | | | | |

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Hollow Stem Auger

See Exhibit A-3 for description of field procedures.
See Appendix B for description of laboratory procedures and additional data (if any).
See Appendix C for explanation of symbols and abbreviations.

Notes:

Abandonment Method:
Boring backfilled with auger cuttings upon completion.

WATER LEVEL OBSERVATIONS

Terracon
4450 Bataan Memorial E
Las Cruces, NM

Boring Started: 06-20-2018

Boring Completed: 06-20-2018

Drill Rig: CME 75

Driller: Terra Test

Project No.: 68185059

Exhibit: A-13



Appendix A

Engineer Drawings

960 RECYCLE FACILITY

*E/2, SE/4, Section 23 - Township 23 South, Range 26 East,
N.M.P.M. Eddy County, New Mexico*

for



Index to Drawings

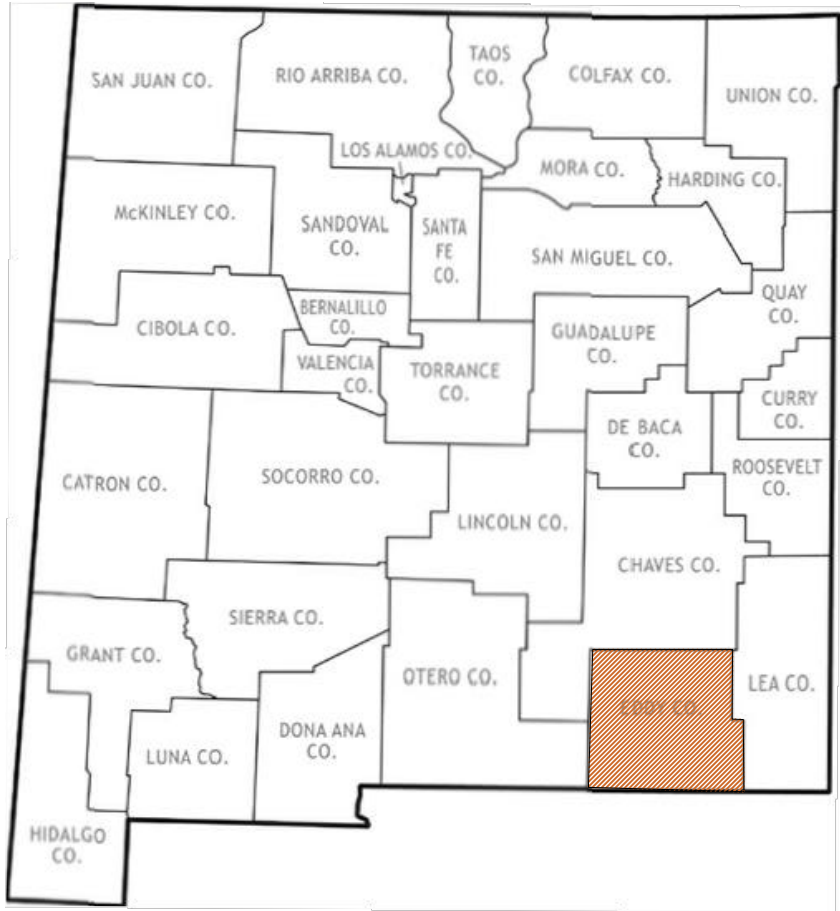
| Sheet No. | Description |
|-----------|--------------------------|
| 1. | Cover Sheet |
| 2. | Project Location Plan |
| 3. | Site Plan |
| 4. | Site Prep / Staking Plan |
| 5. | Cross Sections |
| 6. | Sump Plan & Details |
| 7. | Miscellaneous Details |
| 8. | Miscellaneous Details |

Contacts

3Bear Energy - Kevin Burns

Envirotech Engineering - Jimmy Stallings 580-234-8780
(Design Engineer)





| | | | |
|--------------------------|--|--------|------------|
| Owner | 3Bear Energy | | |
| Site Name | Carlsbad 960 Recycle Facility Capacity | | |
| | Top FB | Bottom | Max |
| Lagoon Features | | | Liq. Level |
| Side slope Ratio | 3 | | 3 |
| Maximum Depth (ft) | 14.5 | | 11.5 |
| Lagoon Top Width (ft) | 500 | 413 | 476 |
| Lagoon Top Length (ft) | 600 | 513 | 576 |
| Maximum Total Vol (ft³) | 3,692,759 | | 2,822,135 |
| Maximum Total Vol (bbls) | 657,663 | | 502,609 |



| Lagoon Liq Depth ft | Storage ft | Surface Area ac | Remaining Stor Vol ft³ | Gallons Storage gal | BBLs Storage bbls | Percent of Total Volume % | Vol in lagoon ft³ | Gallons Storage gal | Vol in Lagoon bbls | Vol in Lagoon ac-ft | Percent Total Vol % |
|------------------------|---------------|--------------------|---------------------------|------------------------|----------------------|------------------------------|----------------------|------------------------|-----------------------|------------------------|------------------------|
| 14.5 | 0.0 | 6.89 | - | - | - | 0.0% | 3,692,759 | 27,621,834 | 657,663 | 84.77 | 100% |
| 14.0 | 0.5 | 6.81 | 106,631 | 797,596 | 18,990 | 2.9% | 3,543,582 | 26,505,993 | 631,095 | 81.35 | 96% |
| 13.5 | 1.0 | 6.74 | 214,659 | 1,605,649 | 38,230 | 5.8% | 3,396,047 | 25,402,428 | 604,820 | 77.96 | 92% |
| 13.0 | 1.5 | 6.66 | 324,095 | 2,424,227 | 57,720 | 8.8% | 3,250,143 | 24,311,070 | 578,835 | 74.61 | 88% |
| 12.5 | 2.0 | 6.59 | 434,946 | 3,253,396 | 77,462 | 11.8% | 3,105,863 | 23,231,852 | 553,139 | 71.30 | 84% |
| 12.0 | 2.5 | 6.51 | 547,223 | 4,093,224 | 97,458 | 14.8% | 2,963,196 | 22,164,706 | 527,731 | 68.03 | 80% |
| 11.5 | 3.0 | 6.44 | 660,933 | 4,943,779 | 117,709 | 17.9% | 2,822,135 | 21,109,566 | 502,609 | 64.79 | 76% |
| 11.0 | 3.5 | 6.37 | 776,087 | 5,805,127 | 138,217 | 21.0% | 2,682,669 | 20,066,364 | 477,771 | 61.59 | 73% |
| 10.5 | 4.0 | 6.29 | 892,692 | 6,677,336 | 158,984 | 24.2% | 2,544,791 | 19,035,033 | 453,215 | 58.42 | 69% |
| 10.0 | 4.5 | 6.22 | 1,010,759 | 7,560,474 | 180,011 | 27.4% | 2,408,490 | 18,015,505 | 428,941 | 55.29 | 65% |
| 9.5 | 5.0 | 6.15 | 1,130,295 | 8,454,607 | 201,300 | 30.6% | 2,273,759 | 17,007,714 | 404,946 | 52.20 | 62% |
| 9.0 | 5.5 | 6.08 | 1,251,311 | 9,359,803 | 222,852 | 33.9% | 2,140,587 | 16,011,591 | 381,228 | 49.14 | 58% |
| 8.5 | 6.0 | 6.01 | 1,373,814 | 10,276,129 | 244,670 | 37.2% | 2,008,967 | 15,027,069 | 357,787 | 46.12 | 54% |
| 8.0 | 6.5 | 5.94 | 1,497,815 | 11,203,652 | 266,754 | 40.6% | 1,878,888 | 14,054,082 | 334,621 | 43.13 | 51% |
| 7.5 | 7.0 | 5.87 | 1,623,321 | 12,142,441 | 289,106 | 44.0% | 1,750,343 | 13,092,562 | 311,728 | 40.18 | 47% |
| 7.0 | 7.5 | 5.80 | 1,750,343 | 13,092,562 | 311,728 | 47.4% | 1,623,321 | 12,142,441 | 289,106 | 37.27 | 44% |
| 6.5 | 8.0 | 5.73 | 1,878,888 | 14,054,082 | 334,621 | 50.9% | 1,497,815 | 11,203,652 | 266,754 | 34.39 | 41% |
| 6.0 | 8.5 | 5.66 | 2,008,967 | 15,027,069 | 357,787 | 54.4% | 1,373,814 | 10,276,129 | 244,670 | 31.54 | 37% |
| 5.5 | 9.0 | 5.59 | 2,140,587 | 16,011,591 | 381,228 | 58.0% | 1,251,311 | 9,359,803 | 222,852 | 28.73 | 34% |
| 5.0 | 9.5 | 5.52 | 2,273,759 | 17,007,714 | 404,946 | 61.6% | 1,130,295 | 8,454,607 | 201,300 | 25.95 | 31% |
| 4.5 | 10.0 | 5.45 | 2,408,490 | 18,015,505 | 428,941 | 65.2% | 1,010,759 | 7,560,474 | 180,011 | 23.20 | 27% |
| 4.0 | 10.5 | 5.39 | 2,544,791 | 19,035,033 | 453,215 | 68.9% | 892,692 | 6,677,336 | 158,984 | 20.49 | 24% |
| 3.5 | 11.0 | 5.32 | 2,682,669 | 20,066,364 | 477,771 | 72.6% | 776,087 | 5,805,127 | 138,217 | 17.82 | 21% |
| 3.0 | 11.5 | 5.25 | 2,822,135 | 21,109,566 | 502,609 | 76.4% | 660,933 | 4,943,779 | 117,709 | 15.17 | 18% |
| 2.5 | 12.0 | 5.19 | 2,963,196 | 22,164,706 | 527,731 | 80.2% | 547,223 | 4,093,224 | 97,458 | 12.56 | 15% |
| 2.0 | 12.5 | 5.12 | 3,105,863 | 23,231,852 | 553,139 | 84.1% | 434,946 | 3,253,396 | 77,462 | 9.98 | 12% |
| 1.5 | 13.0 | 5.06 | 3,250,143 | 24,311,070 | 578,835 | 88.0% | 324,095 | 2,424,227 | 57,720 | 7.44 | 9% |
| 1.0 | 13.5 | 4.99 | 3,396,047 | 25,402,428 | 604,820 | 92.0% | 214,659 | 1,605,649 | 38,230 | 4.93 | 6% |
| 0.5 | 14.0 | 4.93 | 3,543,582 | 26,505,993 | 631,095 | 96.0% | 106,631 | 797,596 | 18,990 | 2.45 | 3% |
| 0.0 | 14.5 | 4.86 | 3,692,759 | 27,621,834 | 657,663 | 100.0% | - | - | - | - | 0% |

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PROJECT LOCATION PLAN

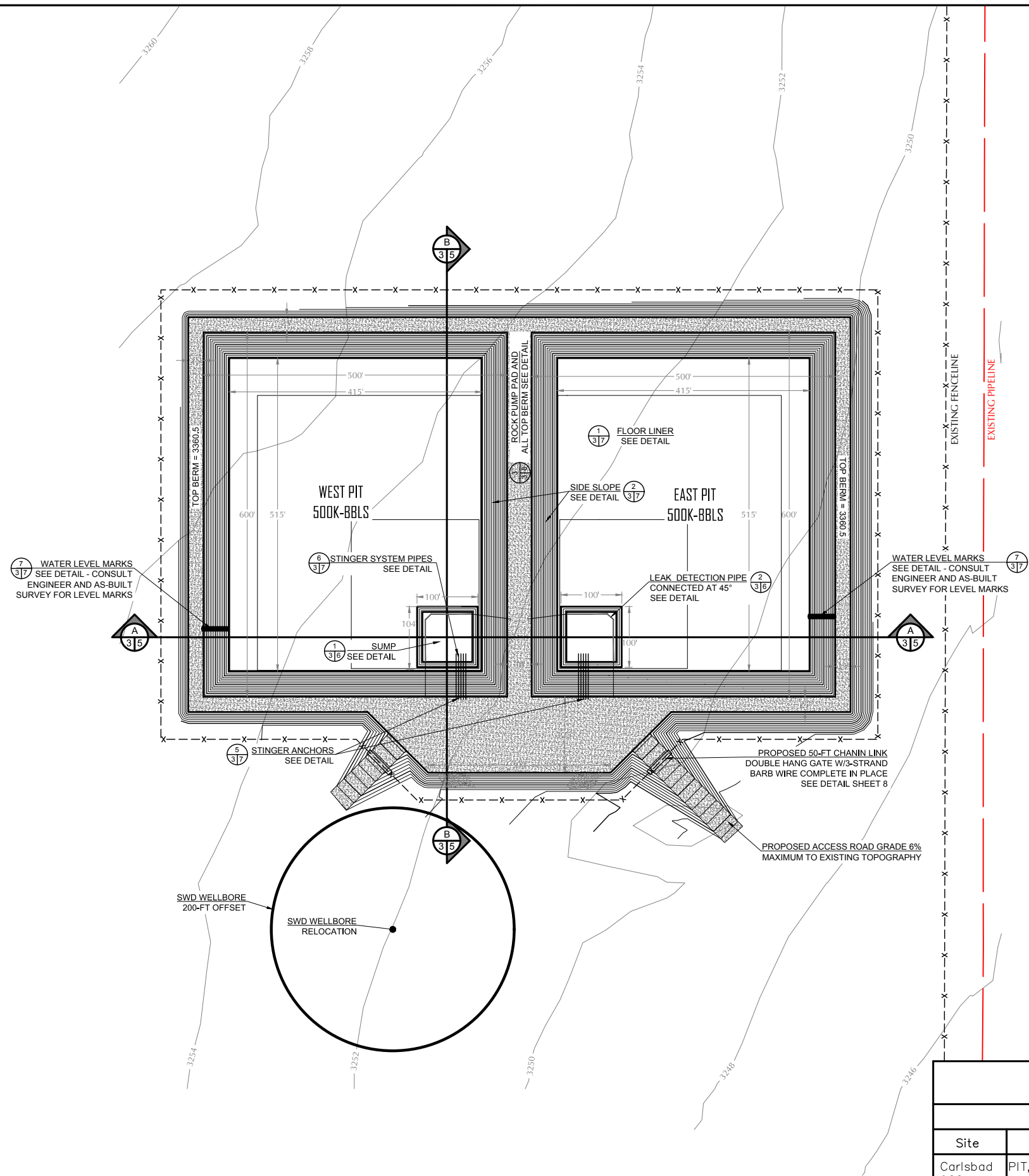
960 RECYCLE FACILITY
E/2, SE/4 Sec 23-T23S-R26E
N.M.P.M. - Eddy County, New Mexico

| | |
|--------------|--------------|
| Date: | July 2018 |
| Scale: | N.T.S. |
| Designed by: | Mr. Johnson |
| Drawn by: | Mr. Johnson |
| Checked by: | J. Stallings |
| Project No. | 018183-00 |

Sheet No.

2 of 8





- GENERAL NOTES:
1. PREPARED SUBGRADE MEANS COMPACTED SMOOTH SUBGRADE FREE OF ROCK, ROOTS, WOOD DEBRIS, CONCRETE RUBBLE AND ANY SHARP OBJECTS THAT MIGHT PUNCTURE THE HDPE LINER.
 2. ALL INTERIOR SLOPES AND TOP OF BERMS TO BE SMOOTH DRUM ROLLED.
 3. ALL EMBANKMENT SLOPES SHALL HAVE A RATIO OF 3:1, COMPACTED EARTH EMBANKMENTS TO BE CONSTRUCTED WITH 12 INCH (MAXIMUM LOOSE LIFTS, COMPACTED TO 95% STANDARD PROCTOR DENSITY.
 4. PERFORM GEOTECHNICAL ANALYSIS ON EXISTING SOIL TO CONFIRM SOIL IS SUITABLE FOR USE IN THE LEVEE.
 5. ALL BOTTOM OF PITS SHALL SLOPE TO THE SUMP @ 0.5%.

Preliminary Site Volume Table: Adjusted

| | | | | | Cut | Fill | NET | |
|-----------------------|----------------|-----------|-----------|-------------|------------|------------|------------|--------|
| Site | Stratum | Surface 1 | Surface 2 | Fill Factor | cubic yard | cubic yard | cubic yard | Method |
| Carlsbad 960 Facility | PIT, PAD, RAMP | Existing | Proposed | 20% | 109,185 | 108,315 | 870 (c) | Grid |
| | | | | | | | | |

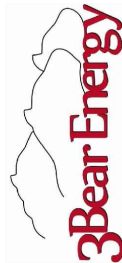
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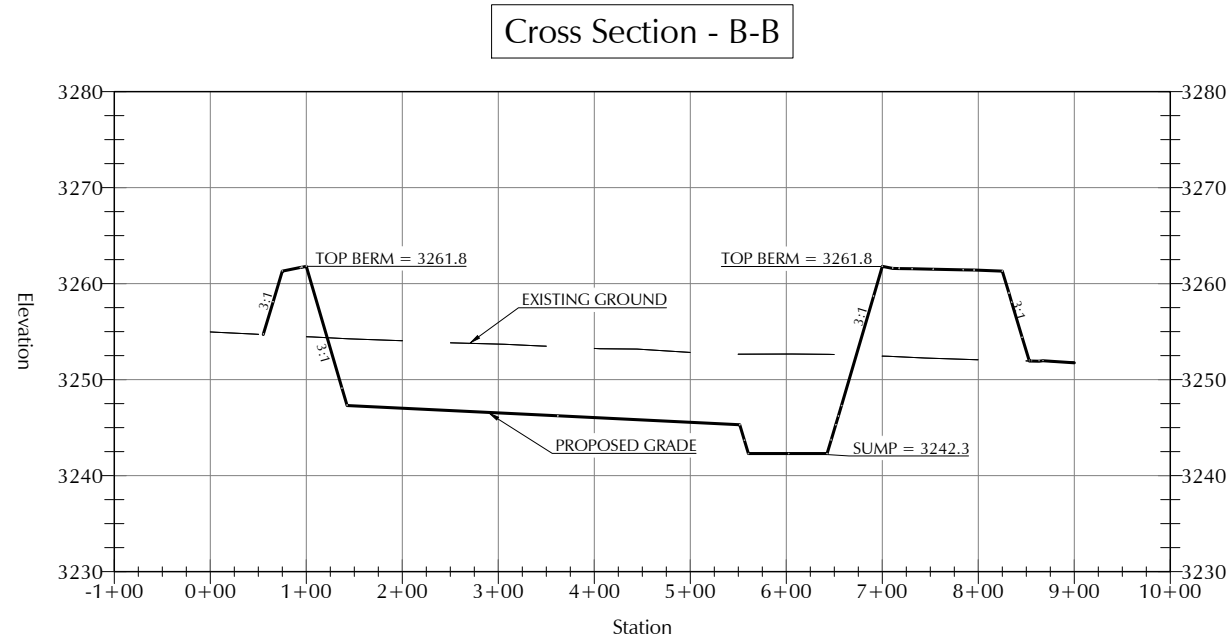
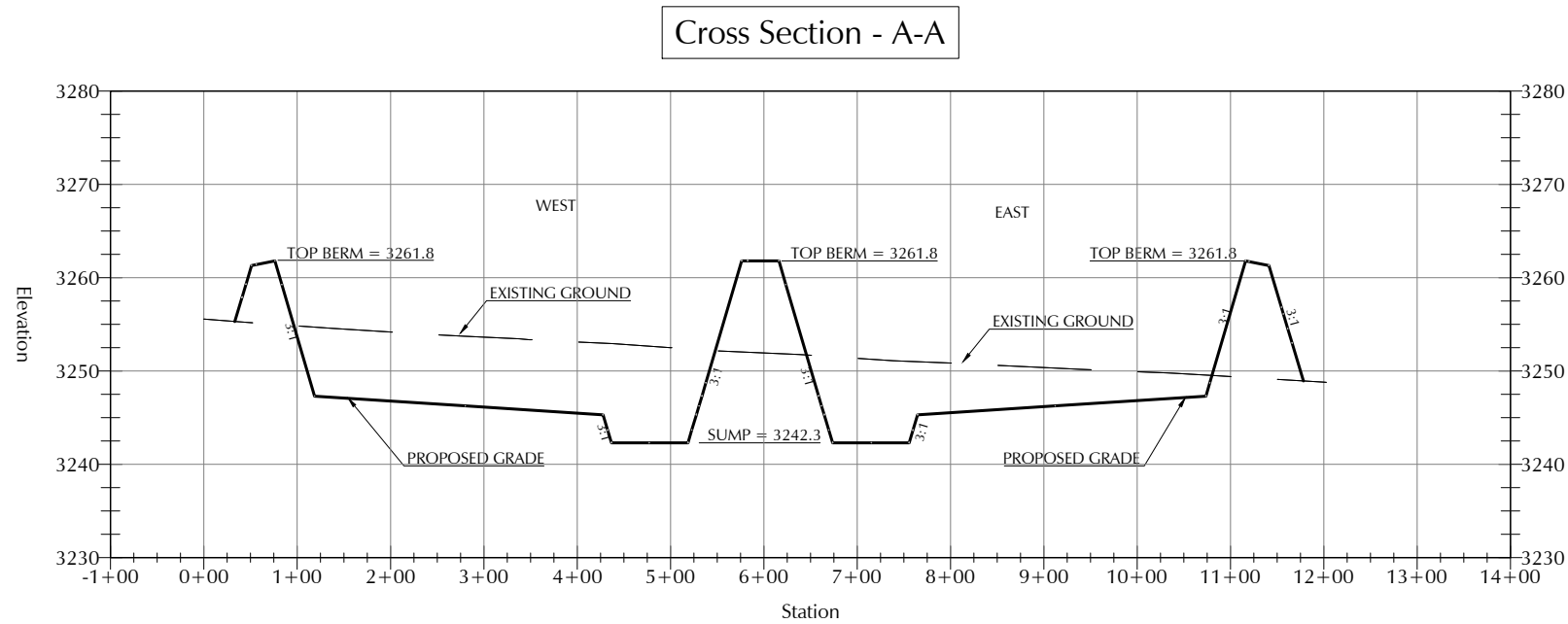
SITE PLAN

960 RECYCLE FACILITY
E/2, SE/4 Sec 23-T23S-R26E
N.M.P.M. - Eddy County, New Mexico

| |
|--------------------------|
| Date: July 2018 |
| Scale: 1" = 200' |
| Designed by: M. Johnson |
| Drawn by: M. Johnson |
| Checked by: J. Stallings |
| Project No. 018183-00 |

Sheet No.

3 of 8



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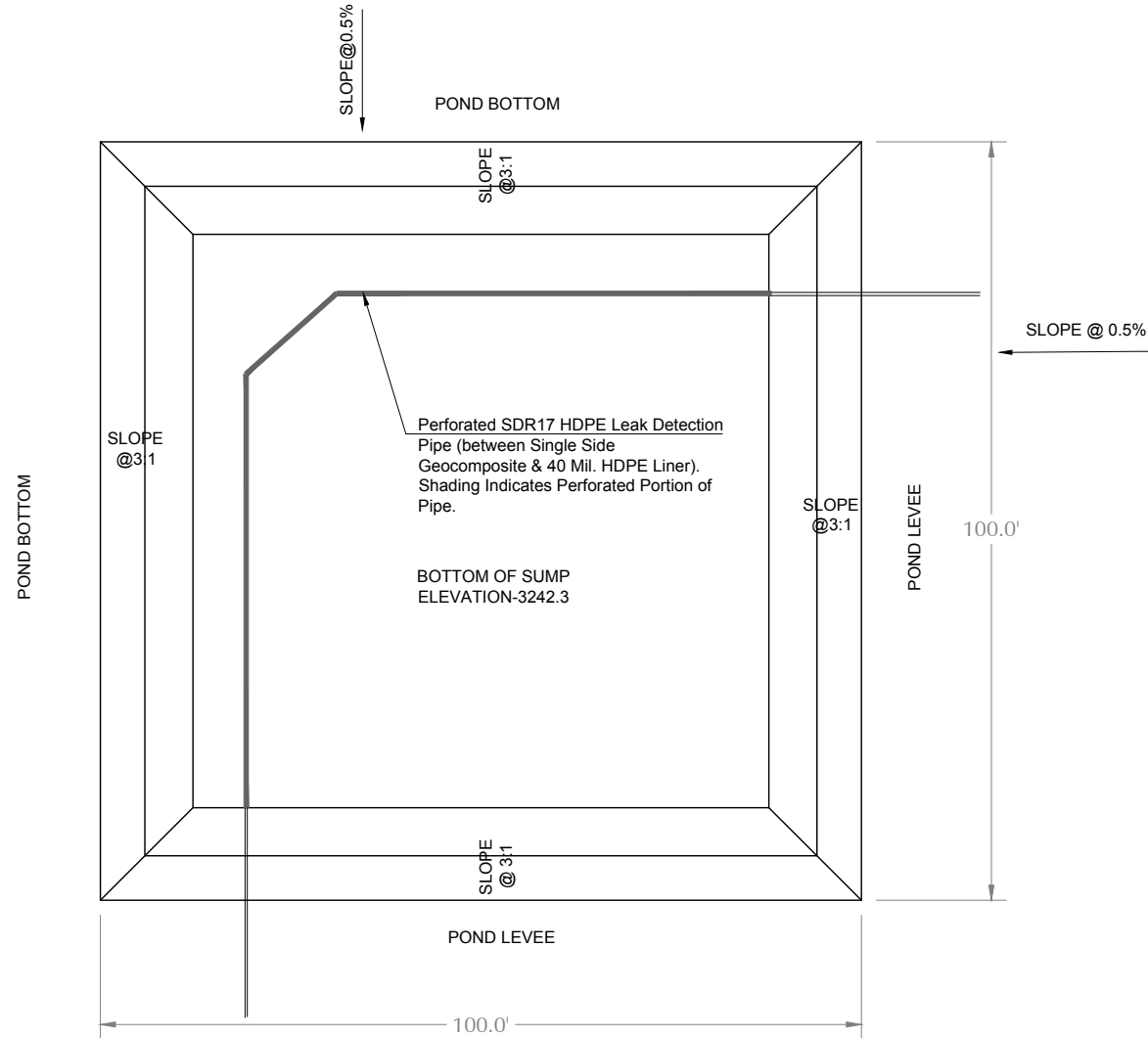


CROSS SECTIONS
960 RECYCLE FACILITY
E/2, SE/4 Sec 23-T23S-R26E
N.M.P.M. - Eddy County, New Mexico

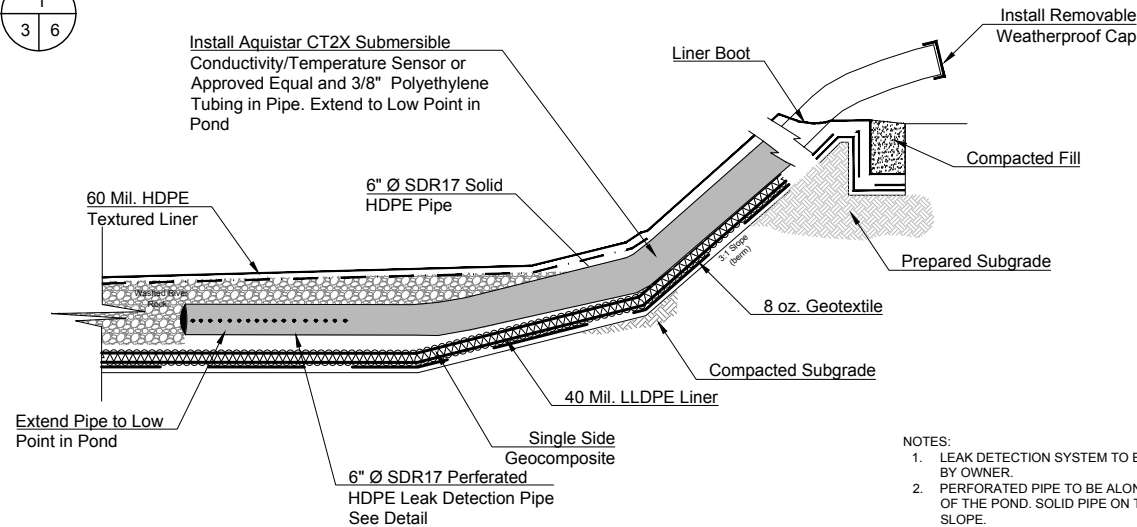
| |
|-----------------------------|
| Date: July 2018 |
| Scale: H:1"=200', V: 1"=20' |
| Designed by: M. Johnson |
| Drawn by: M. Johnson |
| Checked by: J. Stallings |
| Project No. 018183-00 |

Sheet No.

5 of 8



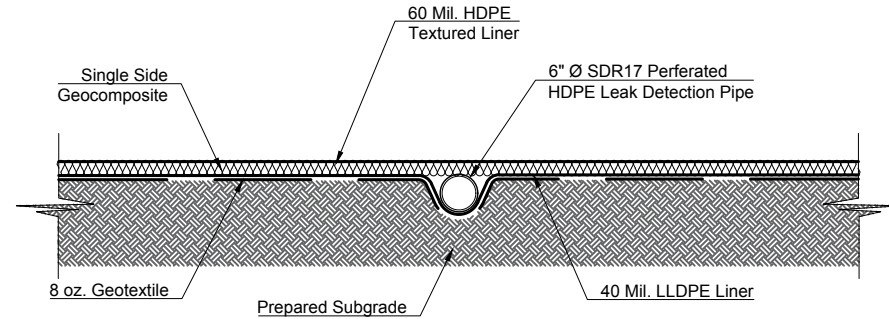
POND SUMP PLAN VIEW
Not to Scale



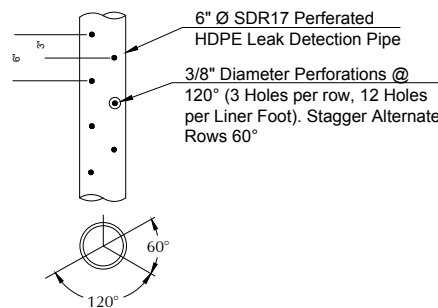
LEAK DETECTION/SAMPLING SYSTEM DETAIL
Not to Scale



- NOTES:
1. LEAK DETECTION SYSTEM TO BE INSTALLED BY OWNER.
 2. PERFORATED PIPE TO BE ALONG THE BOTTOM OF THE POND. SOLID PIPE ON THE SIDE SLOPE.
 3. CONSTRUCT COMPACTED SUBGRADE TO 95% STANDARD PROCTOR AS PER ASTM D-698.
 4. EXTEND 60 MIL. RUB SHEET 1.0-FT PAST TOP OF SHOULDER OF SUMP.
 5. WASH RIVER ROCK SHALL BE 3/8" MIN. & 3/4" MAX.



LEAK DETECTION PIPE DETAIL
Not to Scale



PERFORATED PIPE DETAIL
Not to Scale



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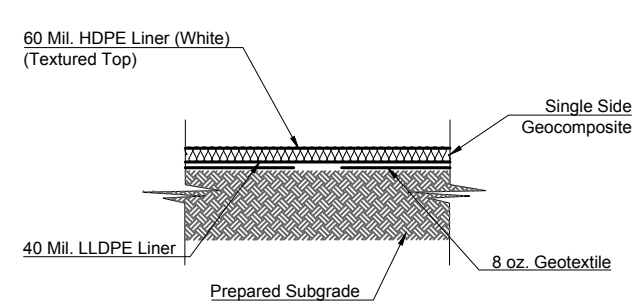
SUMP PLAN & DETAILS

960 RECYCLE FACILITY
E/2, SE/4 Sec 23-T23S-R26E
N.M.P.M. - Eddy County, New Mexico

| | |
|--------------------------|-----------------------|
| Date: July 2018 | Scale: N.T.S. |
| Designed by: M. Johnson | Drawn by: M. Johnson |
| Checked by: J. Stallings | Project No: 018183-00 |

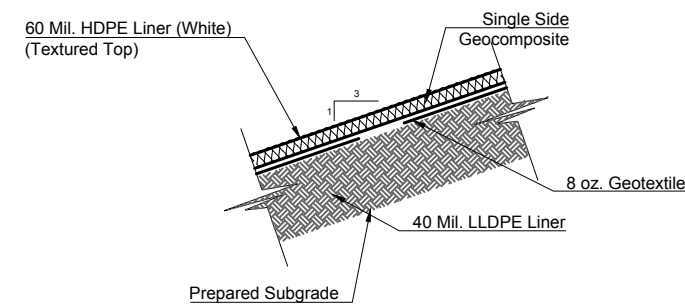
Sheet No.

6 of 8



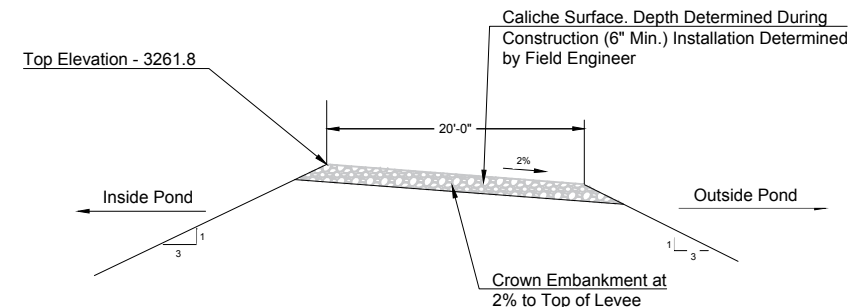
LINER SYSTEM FLOOR DETAIL
Not to Scale

1
3 7



LINER SYSTEM SIDE SLOPE DETAIL
Not to Scale

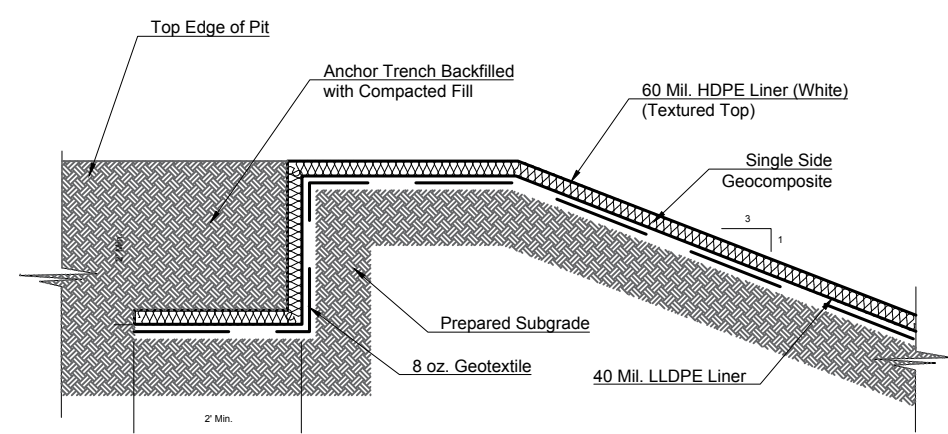
2
3 7



TYPICAL CREST DETAIL
Not to Scale

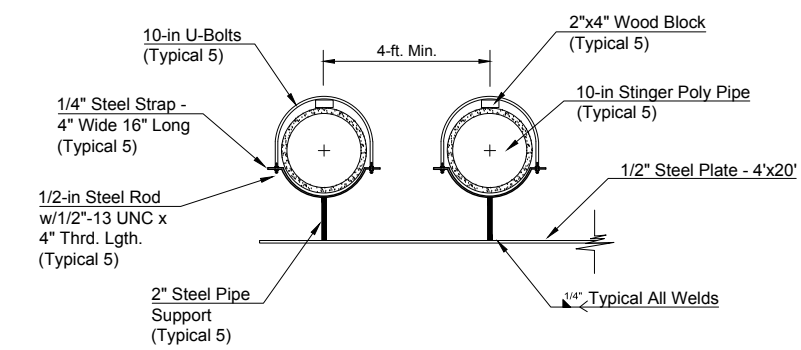
3
3 7

- GENERAL NOTES:
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 5. ALL BOTTOM OF PITS SHALL SLOPE TO THE SUMP @ 0.5%.



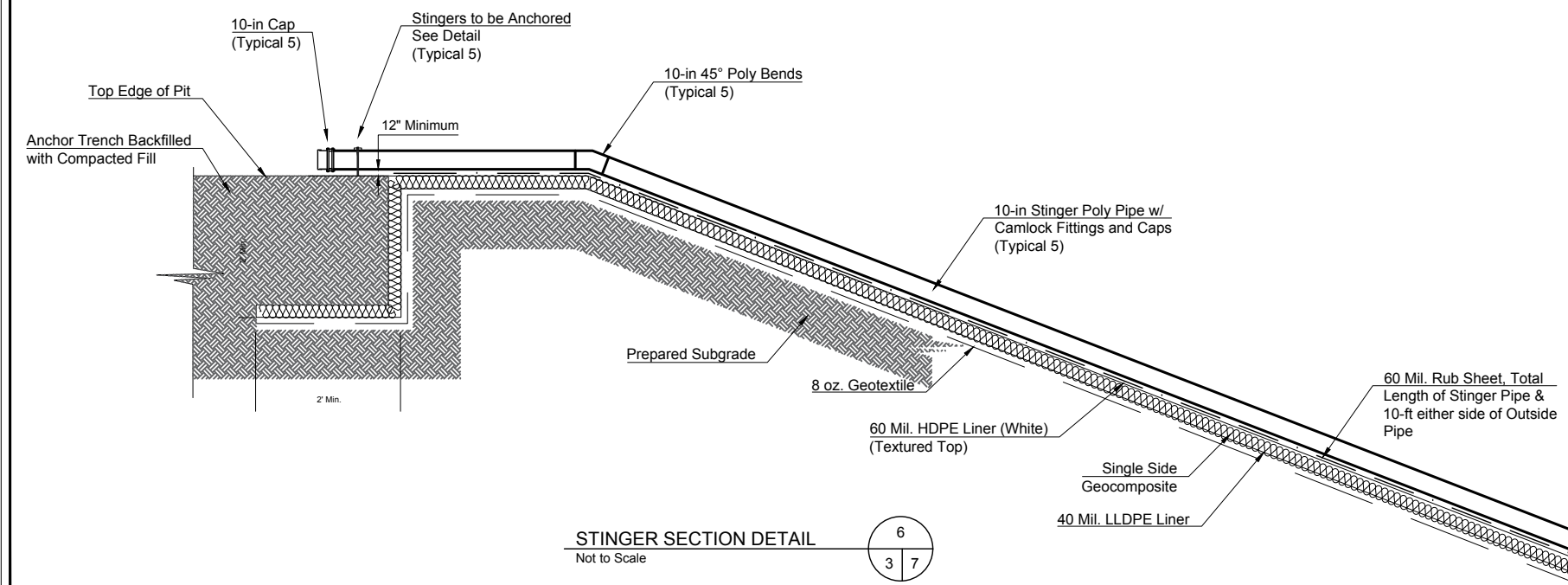
ANCHOR TRENCH DETAIL
Not to Scale

4
3 7



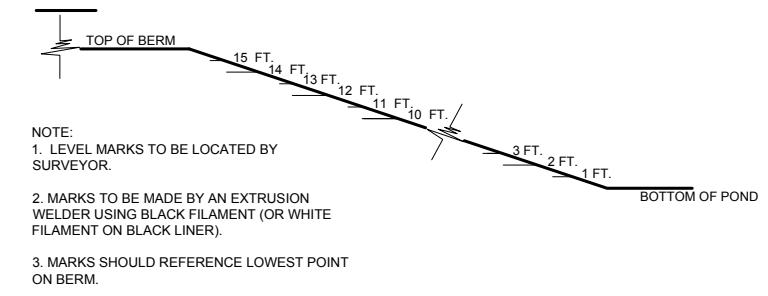
Stinger System Anchor Detail
Not to Scale

5
3 7



STINGER SECTION DETAIL
Not to Scale

6
3 7

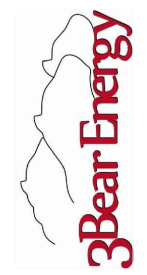


WATER LEVEL MARKS
Not to Scale

7
3 7

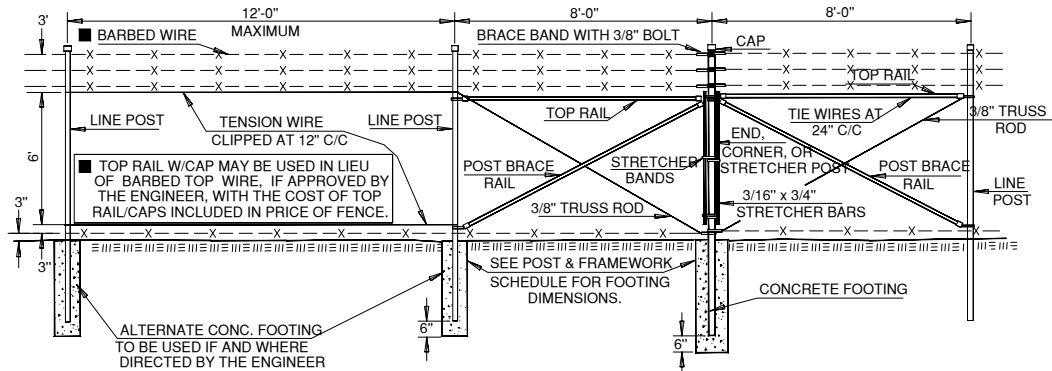
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MISCELLANEOUS DETAILS
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| | |
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| Date: July 2018 | Scale: N.T.S. |
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| Checked by: J. Stallings | Project No. 018183-00 |
| Sheet No. 7 | of 8 |

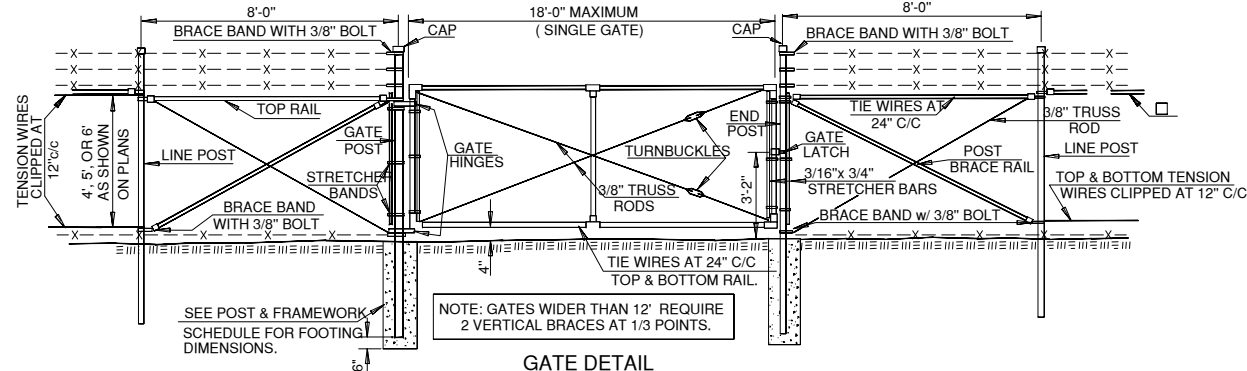


TYPICAL LINE POST DETAIL

NOTE: LINE POSTS MAY BE DRIVEN OR EARTH EMBEDDED. SEE SPECIFICATIONS.

END, CORNER, & STRETCHER DETAILS

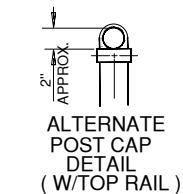
USE STRETCHER DETAILS AT ALL CORNERS, BENDS IN R/W, ON HILLTOPS, IN VALLEYS OR DEEP DEPRESSIONS, AND AT 500' MAXIMUM SPACING. (REQUIRES CONCRETE FOOTING)



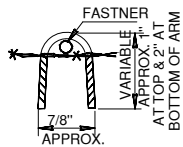
GATE DETAIL

SEE PLANS FOR SIZE AND LOCATION OF GATES. WHERE WIDTH GREATER THAN 18' IS REQUIRED, USE DOUBLE SWING GATES WITH MIDDLE LATCH.

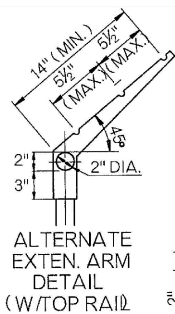
NOTE: Signage Provided by 3Bear Energy- Installed by Contractor



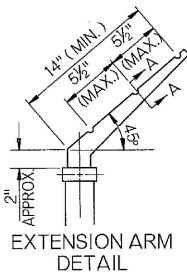
ALTERNATE POST CAP DETAIL (W/TOP RAIL)



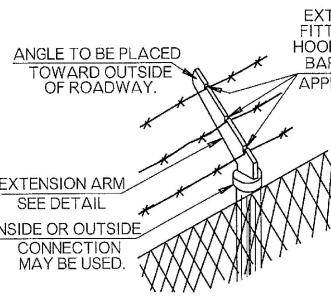
SECTION A-A



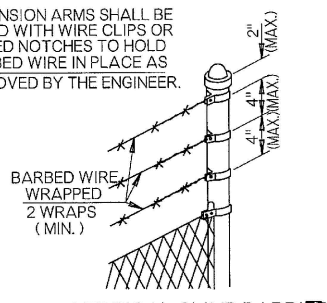
ALTERNATE EXTEN. ARM DETAIL (W/TOP RAIL)



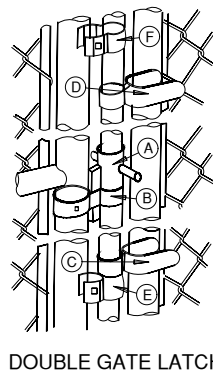
EXTENSION ARM DETAIL



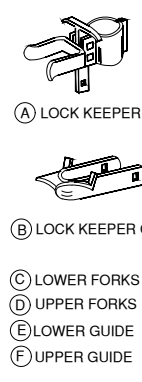
ANGULAR CLIMB BARRIER FOR LINE POSTS



VERTICAL CLIMB BARRIER FOR END & GATE POSTS






















DOUBLE GATE LATCH



SINGLE GATE LATCH

TYPICAL GATE LATCH DETAIL

ALTERNATE TYPE LATCH MAY BE USED IF APPROVED BY THE ENGINEER.

| POST & FRAMEWORK SCHEDULE | | | | | | | | | | | | | | |
|----------------------------------|---|---|---|---|---|---|--|---|---|---|--|---|---|---|
| SHAPE | LINE POST | | | | END, CORNER, OR STRETCHER POSTS | | GATE POSTS  | | | TOP RAIL OR POST BRACE RAIL | | GATE FRAMES | | |
| |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| NOMENCLATURE | 1.5" PIPE | ROLL FORMED HEAVY "C" | ROLL FORMED STAND. "C" | "H" RAIL | 2" PIPE | ROLL FORMED | 2.5" PIPE | 3.5" PIPE | 5.0" PIPE | 1 1/4" PIPE | ROLL FORMED | 1 1/4" PIPE | 1 1/2" PIPE | 1 1/2" PIPE |
| DIMENSIONS | 1.9" O.D. 1.6" I.D. 0.145" THK | 2.25" x 1.7" 0.121" THK | 1.875" x 1.625" 0.105" THK | 2.25" x 1.7" 0.125" THK | 2.38" O.D. 2.07" I.D. 0.154" THK | 3.5" x 3.5" 1.7" 0.128" THK | 2.88" O.D. 2.47" I.D. 0.203" THK | 4.0" O.D. 3.55" I.D. 0.226" THK | 5.563" O.D. 5.047" I.D. 0.258" THK | 1.66" O.D. 1.44" I.D. 0.11" THK | 1.625" x 1.25" 0.075" THK | 1.66" O.D. 1.44" I.D. 0.11" THK | 1.9" O.D. 1.67" I.D. 0.114" THK | 1.9" O.D. 1.61" I.D. 0.145" THK |
| CRITICAL AXIS SEC. MODULUS | .326 IN. ³ | .506 IN. ³ | .368 IN. ³ | .661 IN. ³ | .561 IN. ³ | 1.00 IN. ³ | 1.06 IN. ³ | 2.39 IN. ³ | 5.45 IN. ³ | 0.195 IN. ³ | 0.165 IN. ³ | 0.195 IN. ³ | 0.270 IN. ³ | 0.326 IN. ³ |
| WEIGHT | 2.72 LBS./LIN. FT. | 2.64 LBS./LIN. FT. | 1.85 LBS./LIN. FT. | 3.26 LBS./LIN. FT. | 3.65 LBS./LIN. FT. | 4.85 LBS./LIN. FT. | 5.79 LBS./LIN. FT. | 9.11 LBS./LIN. FT. | 14.62 LBS./LIN. FT. | 1.81 LBS./LIN. FT. | 1.35 LBS./LIN. FT. | 1.81 LBS./LIN. FT. | 2.17 LBS./LIN. FT. | 2.72 LBS./LIN. FT. |
| LENGTH FOR GIVEN FENCE FAB. H | 4 | 6'-10" W/CONC. FOOTING: 7'-4" WHEN DRIVEN. | 7'-4" W/CONC. FOOTING | | | | 7'-10" | | | |  MAXIMUM WIDTH OF SINGLE SWING GATE TO DIAMETERS AS SHOWN ARE MINIMUM VALUES ARE MINIMUM FOR 6 FT. HIGH FENCE, AND  WIRE FABRIC TO BE WOVEN INTO LOCK LOCK  SECTION MODULUS AS SHOWN IS BASED UPON FORMULA ON CLASS 2 COLD FORMED STEEL  SECTION MODULUS AS SHOWN IS BASED UPON FORMULA ON CLASS 2 COLD FORMED STEEL | | | |
| | 8 | 8'-1" W/CONC. FOOTING: 8'-7" WHEN DRIVEN. | 8'-7" W/CONC. FOOTING | | | | 9'-1" | | | | | | | |
| | 9 | 9'-4" W/CONC. FOOTING: 9'-10" WHEN DRIVEN. | 9'-10" W/CONC. FOOTING | | | | 10'-4" | | | | | | | |
| EMBEDMENT FOR GIVEN FENCE FAB. H | 4 | 24" IN CONC. FOOTING: 30" WHEN DRIVEN. | 30" IN CONC. FOOTING | | | | 36" | | | | | | | |
| | 5 | 27" IN CONC. FOOTING: 33" WHEN DRIVEN. | 33" IN CONC. FOOTING | | | | 39" | | | | | | | |
| | 6 | 30" IN CONC. FOOTING: 36" WHEN DRIVEN. | 36" IN CONC. FOOTING | | | | 42" | | | | | | | |
| FOOTING DIM. IN EARTH | | 9" DIA. 36" DEEP | 9" DIA. 14" DIA. 12" DIA. 16" DIA. | | | | 18" DIA. | | | | | | | |
| FOOTING DIM. IN ROCK | | 4" DIA. 9" DEEP | 4" DIA. 6" DIA. 12" DIA. 16" DIA. 24" DIA. | | | | | | | | | | | |

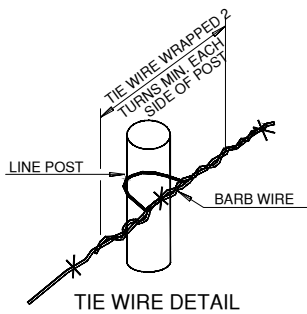
○ MAXIMUM WIDTH OF SINGLE SWING GATE TO BE 18 FT.; OPENING MAY BE UP TO 36 FT. WIDE

DIAMETERS AS SHOWN ARE MINIMUM VALUES. DEPTHS FOR ROCK ARE MINIMUMS. DEPTHS SHOWN FOR CONCRETE FOOTINGS IN EARTH ARE MINIMUM FOR 6 FT. HIGH FENCE, AND MAY BE REDUCED 3 IN. FOR EACH FOOT OF FENCE HEIGHT LESS THAN 6 FT. HIGH.

▲ WIRE FABRIC TO BE WOVEN INTO LOCK LOOPS FOR THE ENTIRE WIDTH OF THE FABRIC.

▼ SECTION MODULUS AS SHOWN IS BASED UPON ASTM A53, AND AASHTO M181. SEE SPECIFICATIONS FOR SUBSTITUTION FORMULA ON CLASS 2 COLD FORMED STEEL PIPE.

● SECTION MODULUS AS SHOWN IS BASED UPON ASTM A 501 AND AASHTO M 181. SEE SPECIFICATIONS FOR SUBSTITUTION FORMULA ON CLASS 2 COLD FORMED STEEL PIPE.



TIE WIRE DETAIL

- GENERAL NOTES**
- ALL CONSTRUCTION AND MATERIAL REQUIREMENTS SHALL BE IN ACCORDANCE WITH THE 1988 STANDARD SPECIFICATIONS AND APPLICABLE SPECIAL PROVISIONS.
 - COST OF BARB WIRE AND EXTRA LENGTH POSTS FOR FAN TO BE INCLUDED IN PRICE BID FOR CHAIN LINK FENCE.
 - ALL MISCELLANEOUS HARDWARE SHALL BE FURNISHED GALVANIZED OR ALUMINUM ALLOY.
 - CLIMB BARRIER SHOWN INTENDED ONLY TO SHOW AN ACCEPTABLE TYPE. ALTERNATE CLIMB BARRIERS APPROVED BY THE ENGINEER PRIOR TO INSTALLATION MAY BE USED. FENCE POST EXTENSION ARM SHALL BE MADE OF PRESSED STEEL OR MALLEABLE IRON AND SHALL BE GALVANIZED AFTER FABRICATION.
 - CHAIN LINK FABRIC MAY BE ACCEPTED KNUCKLED BOTH SELVAGES IN ALL WIDTHS. NO FABRIC WITH TWISTS AND BARBS ON BOTH SELVAGES WILL BE ACCEPTED.
 - NOTE: CLASS A IN THE PAY ITEM DENOTES NO CLIMB BARRIER; CLASS B DENOTES CLIMB BARRIER. (CLASS A = NOBAR; CLASS B = BARR)
 - STRETCHER POSTS TO BE USED IN GENERAL AT HILL TOPS AND AT BOTTOM OF VALLEYS AND AT A MAXIMUM OF 500 FEET APART.
 - ALL POSTS WITH THE EXCEPTION OF LINE POSTS. FAN POSTS AND HEADWALL CONNECTION STRETCHER POSTS SHALL BE EMBEDDED IN CONCRETE WHEN FENCE IS BEING ERECTED ON EARTHEN FOUNDATIONS. OTHER POSTS MAY BE EMBEDDED IN CONCRETE IF AND AS DIRECTED BY THE ENGINEER TO SATISFY SPECIFIC FOOTING REQUIREMENTS.

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N.M.P.M. - Eddy County, New Mexico

| | |
|--------------|--------------|
| Date: | July 2018 |
| Scale: | N.T.S. |
| Designed by: | Mr. Johnson |
| Drawn by: | Mr. Johnson |
| Checked by: | J. Stallings |
| Project No. | 018183-00 |

Sheet No.

8 of 8



Appendix B

Design and Construction Plans



OPERATION AND MAINTENANCE PROCEDURES

Applicable mandates in Rule 34 are underlined. This plan addresses construction of lined earthen containments. *Appendix A* presents Engineering Design Plans. *Appendix C* provides liner and geotextile specifications.

Field conditions may create the need for minor modification of the containment design (e.g. changing the length, width, or depth.)

Dike Protection and Structural Integrity

Design elements are addressed in the section of this submission containing the foundation recommendations. The recommendations are based on site-specific data. The operator, engineer, and selected contractor will review the recommendations prior to beginning the earthwork and adhere to the specific recommendations.

The design and operation provide for the confinement of produced water to prevent releases and to prevent overtopping due to wave action or rainfall. Additionally, the design prevents run-on of surface water as the containment is surrounded by an above-grade levee (berm) and diversion ditch to prevent run-on of surface water.

Stockpile Topsoil

Where topsoil is present, prior to constructing containment, the operator will strip and stockpile the topsoil for use as the final cover or fill at the time of closure. The topsoil will be stockpiled adjacent to a perimeter fence surrounding the containment or incorporated into the levee.

Signage

The design calls for an upright sign no less than 12-in by 24-in with lettering not less than two inches in height in a conspicuous place on the fence surrounding the containment. The sign is posted in a manner and location such that a person can easily read the legend. The sign will provide the following information:

1. The operator's name,
2. The location of the site by quarter-quarter or unit letter, section, township and range, and
3. Emergency telephone numbers.



Fencing

The design provides for a fence to enclose the Recycling Containment in a manner that deters unauthorized wildlife and human access. The design calls for a 7-ft tall chain link and barbed wire fence around the containment to exclude wildlife (see detail on last page of engineering design). This fence provides greater wildlife (and human) deterrence than the minimum required barbed wire fence with four strands evenly spaced in the interval between one foot and four feet above ground level. The fence will be gated to provide access for maintenance and placement of pumps and other necessary equipment. As stated in the O&M plan, the operator will ensure that all gates associated with the fence are closed and locked when responsible personnel are not onsite.

Netting and Protection of Wildlife

The game fence on the containment levee will be effective in excluding antelope, coyotes, and most other terrestrial wildlife.

The Recycling Containment is otherwise protective of wildlife, including migratory birds. The containment will contain treated produced water that has not shown to be a material threat to birds due to hydrogen sulfide gas or floating, free-phase hydrocarbons. The O&M plan calls for the operator to inspect for and, within 30 days of discovery, report the discovery of dead migratory birds or other wildlife to the appropriate wildlife agency and to the division district office in order to facilitate assessment and implementation of measures to prevent incidents from reoccurring.

The containment will have a properly constructed foundation and interior slopes consisting of a firm, unyielding base, smooth and free of rocks, debris, sharp edges or irregularities to prevent the liner's rupture or tear. Geotextile may be placed under the liner when needed to reduce localized stress-strain or protuberances that otherwise may compromise the liner's integrity.

Appendix A shows:

1. The levee has an inside grade no steeper than three horizontal feet to one vertical foot (3H:1V).
2. The levee outside grade is no steeper than three horizontal feet to one vertical foot (3H:1V).



3. The top of the levee is wide enough to install an anchor trench and provide adequate room for inspection and maintenance.
4. The caliche gravel placed on the outside levee provides additional erosion control.

Field conditions may create the need for changes to the design. Any changes to the construction or grade requirements due to unforeseen conditions will be reviewed and approved prior to initiating installation of the liner system. Any design change that does not conform to the NMOCD Rule will be the subject of a variance request and will be submitted to the OCD for review and approval.

LINER AND DRAINAGE GEOTEXTILE INSTALLATION

The containment has a primary (upper) liner and a secondary (lower) liner with a leak detection system appropriate to the site's conditions.

The primary (upper) liner is a geomembrane liner composed of an impervious, synthetic material that is resistant to ultraviolet light, petroleum hydrocarbons, salts and acidic and alkaline solutions. It is 60-mil HDPE. The secondary liner is 40-mil LLDPE. Liner compatibility meets or exceeds a subsequent relevant publication to EPA SW-846 method 9090A.

The Recycling Containment design has a leak detection system between the upper and lower geomembrane liners of 200-mil geonet to facilitate drainage. The leak detection system consists of a properly designed drainage and collection and removal system placed above the lower geomembrane liner in depressions and sloped to facilitate the earliest possible leak detection. The containment floor design calls for a slope of approximately 0.5% toward the sump. This slope, combined with the highly transmissive geonet drainage layer, provides for the earliest possible leak detection.

The liners and drainage material will be installed consistent with the manufacture's specifications (See *Appendix C*). In addition to any specifications of the manufacturer, protocols for liner installation include measures to:

1. Minimize liner seams and orient them up and down, not across, a slope of the levee.
2. Use factory welded seams where possible.
3. Field seams in geosynthetic material are thermally seamed; prior to field seaming, overlap liner four to six inches.



4. Minimize the number of field seams, corners, and irregularly shaped areas.
5. Provide for no horizontal seams within five feet of the slope's toe.
6. Use qualified personnel to perform field welding and testing.
7. Avoid excessive stress-strain on the liner.
8. The edges of all liners are anchored in the bottom of a compacted earth-filled trench that is at least 18-in deep.

At points of discharge into the lined earthen containment, the pipe configuration (see *Appendix A*) effectively protects the liner from excessive hydrostatic force or mechanical damage during filling. The design shows that at any point of discharge into or suction from the recycling containment, the liner is protected from excessive hydrostatic force or mechanical damage. External discharge or suction lines do not penetrate the liner.

Pumping from the containment to hydraulic fracturing operations is the responsibility of stimulation contractors. Typically, numerous lines are permanently placed in the containment with floats attached to prevent damage to the liner system. The containment may be equipped with permanent HDPE stinger (supported by a sacrificial liner or geotextile) for withdrawal of fluid during operations, if the owner deems necessary. External discharge or suction lines do not penetrate the liner.

LEAK DETECTION AND FLUID REMOVAL SYSTEM INSTALLATION

The leak detection system, contains the following design elements:

1. The 200-mil Hypernet drainage material between the primary and secondary liner is sufficiently permeable to allow the transport of fluids to the observation ports (see *Appendices A and G*).
2. The containment floor, sloped towards the monitoring riser pipe, facilitates the earliest possible leak detection of the containment bottom. A pump may be placed in an observation port to provide for fluid removal.
3. Piping will withstand chemical attack from any seepage, structural loading from stresses, and disturbances from overlying water, cover materials, equipment operation, and expansion or contraction (see *Appendix A*).
4. The slope of the interior subgrade is approximately 1%.



Appendix C

Material Specifications



GEOMEMBRANE SPECIFICATION

This specification covers the technical requirements for the Manufacturing and Installation of the geomembrane. All materials meet or exceed the requirements of this specification, and all work will be performed in accordance with the procedures provided in these project specifications

1.1 REFERENCES

- A. American Society for Testing and Materials (ASTM)
1. D 1004 Test Method for Initial Tear Resistance of Plastic Film and Sheeting
 2. D 1238 Standard Test Method for Flow Rates of Thermoplastics by Extrusion Plastometer
 3. D 1505 Test Method for Density of Plastics by the Density-Gradient Technique
 4. D 1603 Test Method for Carbon Black in Olefin Plastics
 5. D 3895 Standard Test Method for Oxidative-Induction Time of Polyolefins by Differential Scanning Calorimetry
 6. D 4218 Standard Test Method for Determination of Carbon Black in Polyethylene Compounds
 7. D 4833 Standard Test Method for Index Puncture Resistance of Geotextiles, Geomembranes, and Related Products
 8. D 5199 Standard Test Method for Measuring Nominal Thickness of Geotextiles and Geomembranes
 9. D 5397 Standard Test Method for Evaluation of Stress Crack Resistance of Polyolefin Geomembranes Using Notched Constant Tensile Load Test
 10. D 5596 Standard Test Method for Microscopic Evaluation of the Dispersion of Carbon Black in Polyolefin Geosynthetics
 11. D 5994 Standard Test Method for Measuring Core Thickness of Textured Geomembranes
 12. D 6392 Standard Test Method for Determining the Integrity of Nonreinforced Geomembrane Seams Produced Using Thermo-Fusion Methods



13. D 6693 Standard Test Method for Determining Tensile Properties of Nonreinforced Polyethylene and Nonreinforced Flexible Polypropylene Geomembranes
 14. D 7240 Standard Practice for Leak Location using Geomembranes with an Insulating Layer in Intimate Contact with a Conductive Layer via Electrical Capacitance Technique (Conductive Geomembrane Spark Test)
- B. Geosynthetic Research Institute
1. GRI GM 13 Test Properties, Testing Frequency and Recommended Warranty for High Density Polyethylene (HDPE) Smooth and Textured Geomembranes
 2. GRI GM 17 Test Properties, Testing Frequency and Recommended Warranty for Linear Low Density Polyethylene (LLDPE) Smooth and Textured Geomembranes

1.2 DEFINITIONS

- A. Lot - A quantity of resin (usually the capacity of one rail car) used in the manufacture of geomembranes. Finished roll will be identified by a roll number traceable to the resin lot used.
- B. Construction Quality Assurance Consultant (CONSULTANT) – The Party, independent from MANUFACTURER and INSTALLER, that is responsible for observing and documenting activities related to quality assurance during the lining system construction.
- C. ENGINEER- The individual or firm responsible for the design and preparation of the project's Contract Drawings and Specifications.
- D. Geomembrane Manufacturer (MANUFACTURER) - The party responsible for manufacturing the geomembrane rolls.
- E. Geosynthetic Quality Assurance Laboratory (TESTING LABORATORY) – The Party, independent from the OWNER, MANUFACTURER, and INSTALLER, responsible for conducting laboratory tests on samples of geosynthetics obtained at the site or during manufacturing, usually under the direction of the OWNER.
- F. INSTALLER- The Party responsible for field handling, transporting, storing, deploying, seaming, and testing of the geomembrane seams.



- G. Panel- Unit area of geomembrane that will be seamed in the field that is larger than 100-ft².
- H. Patch - Unit area of geomembrane that will be seamed in the field that is less than 100-ft².
- I. Subgrade Surface - Soil layer surface which immediately underlies the geosynthetic material(s).

1.3 SUBMITTALS POST-AWARD

- A. Furnish the following product data, in writing, to ENGINEER prior to installation of the geomembrane material:
 - 1. Resin Data shall include the following:
 - a. Certification stating that the resin meets the specification requirements (see *Table 1.9B*).
 - 2. Geomembrane Roll
 - a. Statement certifying no recycled polymer and no more than 10% rework of the same type of material is added to the resin (product run may be recycled).
- B. The INSTALLER shall furnish the following information to the ENGINEER and OWNER prior to installation:
 - 1. Installation layout drawings
 - a. Must show proposed panel layout including field seams and details
 - b. Must be approved prior to installing the geomembrane
 - 2. Approved drawings will be for concept only; actual panel placement will be determined by site conditions.
 - 3. Installer's Geosynthetic Field Installation Quality Assurance Plan
- C. The INSTALLER will submit the following to the ENGINEER upon completion of installation:
 - 1. Certificate stating the geomembrane has been installed in accordance with the Contract Documents
 - 2. Material and installation warranties
 - 3. As-built drawings showing actual geomembrane placement and seams including typical anchor trench detail



1.4 QUALITY ASSURANCE

- A. The OWNER will engage and pay for the services of a Geosynthetic Quality Assurance Consultant and Laboratory to monitor geomembrane installation.

1.5 QUALIFICATIONS

A. MANUFACTURER

- 1. Geomembrane shall be manufactured by the following:
 - a. GSE Lining Technology, LLC
 - b. approved equal
- 2. MANUFACTURER shall have manufactured a minimum of 10,000,000-ft² of polyethylene geomembrane during the last year.

B. INSTALLER

- 1. Installation shall be performed by one of the following installation companies (or approved equal)
 - a. GSE Lining Technology, LLC
 - b. GSE Approved Installers
- 2. INSTALLER shall have installed a minimum of 5,000,000-ft² of HDPE geomembrane during the last two years.
- 3. INSTALLER shall have worked in a similar capacity on at least 5 projects similar in complexity to the project described in the contract documents, and with at least 500,000-ft² of HDPE geomembrane installation on each project.
- 4. The Installation Supervisor shall have worked in a similar capacity on projects similar in size and complexity to the project described in the Contract Documents.
- 5. The INSTALLER shall provide a minimum of one Master Seamer for work on the project.
 - a. Must have completed a minimum of 1,000,000-ft² of geomembrane seaming work using the type of seaming apparatus proposed for the use on this Project.



1.6 MATERIAL LABELING, DELIVERY, STORAGE AND HANDLING

- A. Labeling - Each roll of geomembrane delivered to the site shall be labeled by the MANUFACTURER. The label will identify:
 - a. manufacturer's name
 - b. product identification
 - c. thickness
 - d. length
 - e. width
 - f. roll number
- B. Delivery- Rolls of liner will be prepared to ship by appropriate means to prevent damage to the material and to facilitate off-loading.
- C. Storage- The onsite storage location for geomembrane material, provided by the CONTRACTOR to protect the geomembrane from punctures, abrasions and excessive dirt and moisture, should have the following characteristics:
 - a. level (no wooden pallets)
 - b. smooth
 - c. dry
 - d. protected from theft and vandalism
 - e. adjacent to the area being lined
- D. Handling- Materials are to be handled so as to prevent damage.

1.7 WARRANTY

- A. Material shall be warrantied, on a pro-rata basis, against Manufacturer's defects for a period of 5 years from the date of geomembrane installation.
- B. Installation shall be warrantied against defects in workmanship for a period of 1 year from the date of geomembrane completion.

1.8 GEOMEMBRANE PROPERTIES

- A. Material shall be smooth/textured polyethylene geomembrane as shown on the drawings.

B. Resin

1. Resin shall be new, first quality, compounded and manufactured specifically for producing geomembrane.
2. Natural resin (without carbon black) shall meet the following requirements:

| Table 1.9B RAW MATERIAL PROPERTIES | | | |
|------------------------------------|---------------------------|--------|--------|
| Property | Test Method | HDPE | LLDPE |
| Density (g/cm ³) | ASTM D 1505 | ≥0.932 | ≥0.915 |
| Melt Flow Index (g/10 min) | ASTM D 1238 (190/2.16) | ≤1.0 | ≤1.0 |
| OIT (minutes) | ASTM D 3895 (1 atm/200°C) | ≥100 | ≥100 |

C. Geomembrane Rolls

1. Do not exceed a combined maximum total of 1 percent by weight of additives other than carbon black.
2. Geomembrane shall be free of holes, pinholes as verified by on-line electrical detection, bubbles, blisters, excessive contamination by foreign matter, and nicks and cuts on roll edges.
3. Geomembrane material is to be supplied in roll form. Each roll is to be identified with labels indicating roll number, thickness, length, width, and MANUFACTURER.
4. All liner sheets produced at the factory shall be inspected prior to shipment for compliance with the physical property requirements listed in *Section 1.09 D* and be tested by an acceptable method of inspecting for pinholes. If pinholes are located, identified and indicated during manufacturing, these pinholes may be corrected during installation.

D. Smooth surfaced geomembrane shall meet the requirements shown in the following data sheets below:

1. *Table 1.1* for Black HDPE
2. *Table 1.2* for Green HDPE
3. *Table 1.3* for White HDPE
 - a. The geomembrane shall be a white-surfaced, coextruded geomembrane.
 - b. The white surface shall be installed upwards.



4. *Table 1.4* for Smooth Leak Location Liner HDPE
 - a. The geomembrane shall have a coextruded, electrically conductive layer.
 - b. The conductive layer is installed downward.
 - c. Electrical testing shall be performed after liner installation by the INSTALLER.
5. *Table 1.5* for Smooth White Leak Location Liner HDPE
 - a. The geomembrane shall have a coextruded, electrically conductive layer.
 - b. The conductive layer is installed downward.
 - c. The geomembrane shall be a white-surfaced, coextruded geomembrane.
 - d. The white surface shall be installed upwards.
 - e. Electrical testing shall be performed after liner installation by the INSTALLER.
6. *Table 1.6* for Black LLDPE
7. *Table 1.7* for White-surfaced LLDPE
 - a. The geomembrane shall be a white-surfaced, coextruded geomembrane.
 - b. The white surface shall be installed upwards.
8. *Table 1.8* for Leak Location Liner LLDPE
 - a. The geomembrane shall have a coextruded, electrically conductive layer.
 - b. The conductive layer is installed downward.
 - c. Electrical testing shall be performed after liner installation by the INSTALLER.
9. *Table 1.9* for White Leak Location Liner LLDPE
 - a. The geomembrane shall be a white-surfaced, coextruded geomembrane.
 - b. The white surface shall be installed upwards.
 - c. The geomembrane shall have a coextruded, electrically conductive layer.
 - d. The conductive layer is installed downward.
 - e. Electrical testing shall be performed after liner installation by the INSTALLER.



TABLE 1.1: GSE HD SMOOTH GEOMEMBRANE

| Tested Property | Test Method | Frequency | Minimum Average Values | | | | |
|---|---|-------------|------------------------|---------------------|---------------------|---------------------|---------------------|
| | | | 30 mil | 40 mil | 60 mil | 80 mil | 100 mil |
| Thickness, mil Lowest individual reading | ASTM D 5199 | every roll | 30 27 | 40 36 | 60 54 | 80 72 | 100 90 |
| Density, g/cm ³ , (min.) | ASTM D 1505 | 200,000 lbs | 0.940 | 0.940 | 0.940 | 0.940 | 0.940 |
| Tensile Properties (each direction) | ASTM D 6693, Type IV Dumbbell, 2 ipm G.L. 2.0 in G.L. 1.3 in | 20,000 lbs | 114 | 152 | 228 | 304 | 380 |
| Strength at Break, lb/in-width | | | 63 | 84 | 126 | 168 | 210 |
| Strength at Yield, lb/in-width | | | 700 | 700 | 700 | 700 | 700 |
| Elongation at Break, % | | | 12 | 12 | 12 | 12 | 12 |
| Elongation at Yield, % | | | | | | | |
| Tear Resistance, lb | ASTM D 1004 | 45,000 lbs | 21 | 28 | 42 | 56 | 70 |
| Puncture Resistance, lb | ASTM D 4833 | 45,000 lbs | 54 | 72 | 108 | 144 | 180 |
| Carbon Black Content, % (Range) | ASTM D 1603*/4218 | 20,000 lbs | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 |
| Carbon Black Dispersion | ASTM D 5596 | 45,000 lbs | Note ⁽¹⁾ | Note ⁽¹⁾ | Note ⁽¹⁾ | Note ⁽¹⁾ | Note ⁽¹⁾ |
| Notch Constant Tensile Load, hr | ASTM D 5397, Appendix | 200,000 lbs | 300 | 300 | 300 | 300 | 300 |
| Oxidative Induction Time, min | ASTM D 3895, 200°C; O ₂ , 1 atm | 200,000 lbs | >100 | >100 | >100 | >100 | >100 |
| Typical Roll Dimensions | | | | | | | |
| Roll Length ⁽²⁾ , ft | | | 1,120 | 870 | 560 | 430 | 340 |
| Roll Width ⁽²⁾ , ft | | | 22.5 | 22.5 | 22.5 | 22.5 | 22.5 |
| Roll Area, ft ² | | | 25,200 | 19,575 | 12,600 | 9,675 | 7,650 |

NOTES:

- ⁽¹⁾Dispersion only applies to near spherical agglomerates. 9 of 10 views shall be Category 1 or 2. No more than 1 view from Category 3.
- ⁽²⁾Roll lengths and widths have a tolerance of ± 1%.
- GSE HD Smooth is available in rolls weighing approximately 4,000 lb.
- All GSE geomembranes have dimensional stability of ±2% when tested according to ASTM D 1204 and LTB of <-77° C when tested according to ASTM D 746.
- *Modified.



TABLE 1.2: GSE GREEN SMOOTH GEOMEMBRANE

| Tested Property | Test Method | Frequency | Minimum Average Values | | | | |
|---|---|-------------|------------------------|------------------------|-------------------------|-------------------------|-------------------------|
| | | | 30 mil | 40 mil | 60 mil | 80 mil | 100 mil |
| Thickness, mil Lowest individual reading | ASTM D 5199 | every roll | 30 27 | 40 36 | 60 54 | 80 72 | 100 90 |
| Density, g/cm ³ , (min.) | ASTM D 1505 | 200,000 lbs | 0.940 | 0.940 | 0.940 | 0.940 | 0.940 |
| Tensile Properties (each direction) Strength at Break, lb/in-width Strength at Yield, lb/in-width Elongation at Break, % Elongation at Yield, % | ASTM D 6693, Type IV Dumbbell, 2 ipm G.L. 2.0 in G.L. 1.3 in | 20,000 lbs | 114 63 700 12 | 152 84 700 12 | 228 126 700 12 | 304 168 700 12 | 380 210 700 12 |
| Tear Resistance, lb | ASTM D 1004 | 45,000 lbs | 21 | 28 | 42 | 56 | 70 |
| Puncture Resistance, lb | ASTM D 4833 | 45,000 lbs | 54 | 72 | 108 | 144 | 180 |
| Carbon Black Content ⁽¹⁾ , % (Range) | ASTM D 1603*/4218 | 20,000 lbs | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 |
| Carbon Black Dispersion | ASTM D 5596 | 45,000 lbs | Note ⁽²⁾ | Note ⁽²⁾ | Note ⁽²⁾ | Note ⁽²⁾ | Note ⁽²⁾ |
| Notch Constant Tensile Load, hr | ASTM D 5397, Appendix | 200,000 lbs | 300 | 300 | 300 | 300 | 300 |
| Oxidative Induction Time, min | ASTM D 3895, 200°C; O ₂ , 1 atm | 200,000 lbs | >100 | >100 | >100 | >100 | >100 |
| Typical Roll Dimensions | | | | | | | |
| Roll Length ⁽³⁾ , ft | | | 1,120 | 870 | 560 | 430 | 340 |
| Roll Width ⁽³⁾ , ft | | | 22.5 | 22.5 | 22.5 | 22.5 | 22.5 |
| Roll Area, ft ² | | | 25,200 | 19,575 | 12,600 | 9,675 | 7,650 |

NOTES:

- ⁽¹⁾GSE Green Smooth may have an overall ash content of 3.0% due to the green layer. These values apply to the black layer only.
- ⁽²⁾Dispersion applies to near spherical agglomerates. 9 of 10 views shall be Category 1 or 2. No more than 1 view from Category 3.
- ⁽³⁾Roll lengths and widths have a tolerance of ± 1%.
- GSE Green Smooth is available in rolls weighing approximately 4,000 lb.
- All GSE geomembranes have dimensional stability of ±2% when tested according to ASTM D 1204 and LTB of <-77° C when tested according to ASTM D 746.
- *Modified.



TABLE 1.3: GSE WHITE SMOOTH GEOMEMBRANE

| Tested Property | Test Method | Frequency | Minimum Average Values | | | | |
|---|---|-------------|------------------------|------------------------|-------------------------|-------------------------|-------------------------|
| | | | 30 mil | 40 mil | 60 mil | 80 mil | 100 mil |
| Thickness, mil Lowest individual reading | ASTM D 5199 | every roll | 30 27 | 40 36 | 60 54 | 80 72 | 100 90 |
| Density, g/cm ³ , (min.) | ASTM D 1505 | 200,000 lbs | 0.940 | 0.940 | 0.940 | 0.940 | 0.940 |
| Tensile Properties (each direction) Strength at Break, lb/in-width Strength at Yield, lb/in-width Elongation at Break, % Elongation at Yield, % | ASTM D 6693, Type IV Dumbbell, 2 ipm G.L. 2.0 in G.L. 1.3 in | 20,000 lbs | 114 63 700 12 | 152 84 700 12 | 228 126 700 12 | 304 168 700 12 | 380 210 700 12 |
| Tear Resistance, lb | ASTM D 1004 | 45,000 lbs | 21 | 28 | 42 | 56 | 70 |
| Puncture Resistance, lb | ASTM D 4833 | 45,000 lbs | 54 | 72 | 108 | 144 | 180 |
| Carbon Black Content ⁽¹⁾ , % (Range) | ASTM D 1603*/4218 | 20,000 lbs | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 |
| Carbon Black Dispersion | ASTM D 5596 | 45,000 lbs | Note ⁽²⁾ | Note ⁽²⁾ | Note ⁽²⁾ | Note ⁽²⁾ | Note ⁽²⁾ |
| Notch Constant Tensile Load, hr | ASTM D 5397, Appendix | 200,000 lbs | 300 | 300 | 300 | 300 | 300 |
| Oxidative Induction Time, min | ASTM D 3895, 200°C; O ₂ , 1 atm | 200,000 lbs | >100 | >100 | >100 | >100 | >100 |
| Typical Roll Dimensions | | | | | | | |
| Roll Length ⁽³⁾ , ft | | | 1,120 | 870 | 560 | 430 | 340 |
| Roll Width ⁽³⁾ , ft | | | 22.5 | 22.5 | 22.5 | 22.5 | 22.5 |
| Roll Area, ft ² | | | 25,200 | 19,575 | 12,600 | 9,675 | 7,650 |

NOTES:

- ⁽¹⁾GSE White Smooth may have an overall ash content of 3.0% due to the white layer. These values apply to the black layer only.
- ⁽²⁾Dispersion applies to near spherical agglomerates. 9 of 10 views shall be Category 1 or 2. No more than 1 view from Category 3.
- ⁽³⁾Roll lengths and widths have a tolerance of ± 1%.
- GSE White Smooth is available in rolls weighing approximately 4,000 lb.
- All GSE geomembranes have dimensional stability of ±2% when tested according to ASTM D 1204 and LTB of <-77° C when tested according to ASTM D 746.
- *Modified.



| TABLE 4.1: GSE LEAK LOCATION SMOOTH GEOMEMBRANE | | | | | | |
|---|---|-------------|------------------------|---------------------|---------------------|---------------------|
| Tested Property | Test Method | Frequency | Minimum Average Values | | | |
| | | | 40 mil | 60 mil | 80 mil | 100 mil |
| Thickness, mil Lowest individual reading | ASTM D 5199 | every roll | 40 36 | 60 54 | 80 72 | 100 90 |
| Density, g/cm ³ , (min.) | ASTM D 1505 | 200,000 lbs | 0.940 | 0.940 | 0.940 | 0.940 |
| Tensile Properties (each direction) | ASTM D 6693, Type IV Dumbbell, 2 ipm | 20,000 lbs | 152 | 228 | 304 | 380 |
| Strength at Break, lb/in-width | | | 84 | 126 | 168 | 210 |
| Strength at Yield, lb/in-width | G.L. 2.0 in | | 700 | 700 | 700 | 700 |
| Elongation at Break, % | G.L. 1.3 in | | 12 | 12 | 12 | 12 |
| Elongation at Yield, % | | | | | | |
| Tear Resistance, lb | ASTM D 1004 | 45,000 lbs | 28 | 42 | 56 | 70 |
| Puncture Resistance, lb | ASTM D 4833 | 45,000 lbs | 72 | 108 | 144 | 180 |
| Carbon Black Content ⁽¹⁾ , % (Range) | ASTM D 1603*/4218 | 20,000 lbs | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 |
| Carbon Black Dispersion | ASTM D 5596 | 45,000 lbs | Note ⁽²⁾ | Note ⁽²⁾ | Note ⁽²⁾ | Note ⁽²⁾ |
| Notch Constant Tensile Load, hr | ASTM D 5397, Appendix | 200,000 lbs | 300 | 300 | 300 | 300 |
| Oxidative Induction Time, min | ASTM D 3895, 200°C; O ₂ , 1 atm | 200,000 lbs | >100 | >100 | >100 | >100 |
| Typical Roll Dimensions | | | | | | |
| Roll Length ⁽³⁾ , ft | | | 870 | 560 | 430 | 340 |
| Roll Width ⁽³⁾ , ft | | | 22.5 | 22.5 | 22.5 | 22.5 |
| Roll Area, ft ² | | | 19,575 | 12,600 | 9,675 | 7,650 |

NOTES:

- ⁽¹⁾GSE Leak Location Smooth may have an overall ash content of 3.0% due to the conductive layer. These values apply to the non-conductive black layer only.
- ⁽²⁾Dispersion applies to near spherical agglomerates. 9 of 10 views shall be Category 1 or 2. No more than 1 view from Category 3.
- ⁽³⁾Roll lengths and widths have a tolerance of $\pm 1\%$.
- GSE Leak Location Smooth is available in rolls weighing approximately 4,000 lb.
- All GSE geomembranes have dimensional stability of $\pm 2\%$ when tested according to ASTM D 1204 and LTB of $< -77^{\circ}\text{C}$ when tested according to ASTM D 746.
- *Modified.



TABLE 1.5: GSE LEAK LOCATION WHITE SMOOTH GEOMEMBRANE

| Tested Property | Test Method | Frequency | Minimum Average Values | | | |
|---|---|-------------|------------------------|---------------------|---------------------|---------------------|
| | | | 40 mil | 60 mil | 80 mil | 100 mil |
| Thickness, mil Lowest individual reading | ASTM D 5199 | every roll | 40 36 | 60 54 | 80 72 | 100 90 |
| Density, g/cm ³ , (min.) | ASTM D 1505 | 200,000 lbs | 0.940 | 0.940 | 0.940 | 0.940 |
| Tensile Properties (each direction) | ASTM D 6693, Type IV Dumbbell, 2 ipm G.L. 2.0 in G.L. 1.3 in | 20,000 lbs | 152 | 228 | 304 | 380 |
| Strength at Break, lb/in-width | | | 84 | 126 | 168 | 210 |
| Strength at Yield, lb/in-width | | | 700 | 700 | 700 | 700 |
| Elongation at Break, % | | | 12 | 12 | 12 | 12 |
| Elongation at Yield, % | | | | | | |
| Tear Resistance, lb | ASTM D 1004 | 45,000 lbs | 28 | 42 | 56 | 70 |
| Puncture Resistance, lb | ASTM D 4833 | 45,000 lbs | 72 | 108 | 144 | 180 |
| Carbon Black Content ⁽¹⁾ , % (Range) | ASTM D 1603*/4218 | 20,000 lbs | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 |
| Carbon Black Dispersion | ASTM D 5596 | 45,000 lbs | Note ⁽²⁾ | Note ⁽²⁾ | Note ⁽²⁾ | Note ⁽²⁾ |
| Notch Constant Tensile Load, hr | ASTM D 5397, Appendix | 200,000 lbs | 300 | 300 | 300 | 300 |
| Oxidative Induction Time, min | ASTM D 3895, 200°C; O ₂ , 1 atm | 200,000 lbs | >100 | >100 | >100 | >100 |
| Typical Roll Dimensions | | | | | | |
| Roll Length ⁽³⁾ , ft | | | 870 | 560 | 430 | 340 |
| Roll Width ⁽³⁾ , ft | | | 22.5 | 22.5 | 22.5 | 22.5 |
| Roll Area, ft ² | | | 19,575 | 12,600 | 9,675 | 7,650 |

NOTES:

- ⁽¹⁾GSE Leak Location White Smooth may have an overall ash content of 3.0% due to the white and conductive layers. These values apply to the black layer only.
- ⁽²⁾Dispersion applies to near spherical agglomerates. 9 of 10 views shall be Category 1 or 2. No more than 1 view from Category 3.
- ⁽³⁾Roll lengths and widths have a tolerance of $\pm 1\%$.
- GSE Leak Location White Smooth is available in rolls weighing approximately 4,000 lb.
- All GSE geomembranes have dimensional stability of $\pm 2\%$ when tested according to ASTM D 1204 and LTB of $< -77^{\circ}\text{C}$ when tested according to ASTM D 746.
- *Modified.



TABLE 1.6: GSE ULTRAFLEX SMOOTH GEOMEMBRANE

| Tested Property | Test Method | Frequency | Minimum Average Value | | | |
|---|--|-------------|-----------------------|---------------------|---------------------|---------------------|
| | | | 40 mil | 60 mil | 80 mil | 100 mil |
| Thickness, mil Lowest individual reading | ASTM D 5199 | every roll | 40 36 | 60 54 | 80 72 | 100 90 |
| Density, g/cm ³ (max.) | ASTM D 1505 | 200,000 lbs | 0.939 | 0.939 | 0.939 | 0.939 |
| Tensile Properties (each direction) Strength at Break, lb/in-width Elongation at Break, % | ASTM D 6693, Type IV Dumbbell, 2 ipm G.L. 2.0 in | 20,000 lbs | 152 800 | 228 800 | 304 800 | 380 800 |
| Tear Resistance, lb | ASTM D 1004 | 45,000 lbs | 22 | 33 | 44 | 55 |
| Puncture Resistance, lb | ASTM D 4833 | 45,000 lbs | 56 | 84 | 112 | 140 |
| Carbon Black Content, % (Range) | ASTM D 1603*/4218 | 20,000 lbs | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 |
| Carbon Black Dispersion | ASTM D 5596 | 45,000 lbs | Note ⁽¹⁾ | Note ⁽¹⁾ | Note ⁽¹⁾ | Note ⁽¹⁾ |
| Oxidative Induction Time, min | ASTM D 3895, 200°C; O ₂ , 1 atm | 200,000 lbs | >100 | >100 | >100 | >100 |
| Typical Roll Dimensions | | | | | | |
| Roll Length ⁽²⁾ , ft | | | 870 | 560 | 430 | 340 |
| Roll Width ⁽²⁾ , ft | | | 22.5 | 22.5 | 22.5 | 22.5 |
| Roll Area, ft ² | | | 19,575 | 12,600 | 9,675 | 7,650 |

NOTES:

- ⁽¹⁾Dispersion only applies to near spherical agglomerates. 9 of 10 views shall be Category 1 or 2. No more than 1 view from Category 3.
- ⁽²⁾Roll lengths and widths have a tolerance of $\pm 1\%$.
- GSE UltraFlex is available in rolls weighing approximately 4,000 lb.
- All GSE geomembranes have dimensional stability of $\pm 2\%$ when tested according to ASTM D 1204 and LTB of $< -77^{\circ}\text{C}$ when tested according to ASTM D 746.
- *Modified.



| TALBE 1.7: GSE ULTRAFLEX WHITE SMOOTH GEOMEMBRANE | | | | | | |
|---|--|-------------|-----------------------|---------------------|---------------------|---------------------|
| Tested Property | Test Method | Frequency | Minimum Average Value | | | |
| | | | 40 mil | 60 mil | 80 mil | 100 mil |
| Thickness, mil Lowest individual reading | ASTM D 5199 | every roll | 40 36 | 60 54 | 80 72 | 100 90 |
| Density, g/cm ³ (max.) | ASTM D 1505 | 200,000 lbs | 0.939 | 0.939 | 0.939 | 0.939 |
| Tensile Properties (each direction) Strength at Break, lb/in-width Elongation at Break, % | ASTM D 6693, Type IV Dumbbell, 2 ipm G.L. 2.0 in | 20,000 lbs | 152 800 | 228 800 | 304 800 | 380 800 |
| Tear Resistance, lb | ASTM D 1004 | 45,000 lbs | 22 | 33 | 44 | 55 |
| Puncture Resistance, lb | ASTM D 4833 | 45,000 lbs | 56 | 84 | 112 | 140 |
| Carbon Black Content ⁽¹⁾ , % (Range) | ASTM D 1603*/4218 | 20,000 lbs | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 |
| Carbon Black Dispersion | ASTM D 5596 | 45,000 lbs | Note ⁽²⁾ | Note ⁽²⁾ | Note ⁽²⁾ | Note ⁽²⁾ |
| Oxidative Induction Time, min | ASTM D 3895, 200°C; O ₂ , 1 atm | 200,000 lbs | >100 | >100 | >100 | >100 |
| Typical Roll Dimensions | | | | | | |
| Roll Length ⁽³⁾ , ft | | | 870 | 560 | 430 | 340 |
| Roll Width ⁽³⁾ , ft | | | 22.5 | 22.5 | 22.5 | 22.5 |
| Roll Area, ft ² | | | 19,575 | 12,600 | 9,675 | 7,650 |

NOTES:

- ⁽¹⁾GSE UltraFlex White Smooth may have an overall ash content greater than 3.0% due to the white layer. These values apply to the black layer only.
- ⁽²⁾Dispersion only applies to near spherical agglomerates. 9 of 10 views shall be Category 1 or 2. No more than 1 view from Category 3.
- ⁽³⁾Roll lengths and widths have a tolerance of $\pm 1\%$.
- GSE UltraFlex White Smooth is available in rolls weighing approximately 4,000 lb.
- All GSE geomembranes have dimensional stability of $\pm 2\%$ when tested according to ASTM D 1204 and LTB of $< -77^{\circ}\text{C}$ when tested according to ASTM D 746.
- *Modified.



| TABLE 1.8: GSE ULTRAFLEX LEAK LOCATION LINER SMOOTH GEOMEMBRANE | | | | | | |
|---|--|-------------|-----------------------|---------------------|---------------------|---------------------|
| Tested Property | Test Method | Frequency | Minimum Average Value | | | |
| | | | 40 mil | 60 mil | 80 mil | 100 mil |
| Thickness, mil Lowest individual reading | ASTM D 5199 | every roll | 40 36 | 60 54 | 80 72 | 100 90 |
| Density, g/cm ³ (max.) | ASTM D 1505 | 200,000 lbs | 0.939 | 0.939 | 0.939 | 0.939 |
| Tensile Properties (each direction) Strength at Break, lb/in-width Elongation at Break, % | ASTM D 6693, Type IV Dumbbell, 2 ipm G.L. 2.0 in | 20,000 lbs | 152 800 | 228 800 | 304 800 | 380 800 |
| Tear Resistance, lb | ASTM D 1004 | 45,000 lbs | 22 | 33 | 44 | 55 |
| Puncture Resistance, lb | ASTM D 4833 | 45,000 lbs | 56 | 84 | 112 | 140 |
| Carbon Black Content ⁽¹⁾ , % (Range) | ASTM D 1603*/4218 | 20,000 lbs | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 |
| Carbon Black Dispersion | ASTM D 5596 | 45,000 lbs | Note ⁽²⁾ | Note ⁽²⁾ | Note ⁽²⁾ | Note ⁽²⁾ |
| Oxidative Induction Time, min | ASTM D 3895, 200°C; O ₂ , 1 atm | 200,000 lbs | >100 | >100 | >100 | >100 |
| Typical Roll Dimensions | | | | | | |
| Roll Length ⁽³⁾ , ft | | | 870 | 560 | 430 | 340 |
| Roll Width ⁽³⁾ , ft | | | 22.5 | 22.5 | 22.5 | 22.5 |
| Roll Area, ft ² | | | 19,575 | 12,600 | 9,675 | 7,650 |

NOTES:

- ⁽¹⁾GSE UltraFlex Leak Location Smooth may have an overall ash content greater than 3.0% due to the conductive layer. These values apply to the non-conductive black layer only.
- ⁽²⁾Dispersion only applies to near spherical agglomerates. 9 of 10 views shall be Category 1 or 2. No more than 1 view from Category 3.
- ⁽³⁾Roll lengths and widths have a tolerance of ±1%.
- GSE UltraFlex Leak Location Smooth is available in rolls weighing approximately 4,000 lb.
- All GSE geomembranes have dimensional stability of ±2% when tested according to ASTM D 1204 and LTB of <-77°C when tested according to ASTM D 746.
- *Modified.



| TABLE 1.9: GSE ULTRAFLEX LEAK LOCATION LINER WHITE SMOOTH GEOMEMBRANE | | | | | | |
|---|--|-------------|-----------------------|---------------------|---------------------|---------------------|
| Tested Property | Test Method | Frequency | Minimum Average Value | | | |
| | | | 40 mil | 60 mil | 80 mil | 100 mil |
| Thickness, mil Lowest individual reading | ASTM D 5199 | every roll | 40 36 | 60 54 | 80 72 | 100 90 |
| Density, g/cm ³ (max.) | ASTM D 1505 | 200,000 lbs | 0.939 | 0.939 | 0.939 | 0.939 |
| Tensile Properties (each direction) Strength at Break, lb/in-width Elongation at Break, % | ASTM D 6693, Type IV Dumbbell, 2 ipm G.L. 2.0 in | 20,000 lbs | 152 800 | 228 800 | 304 800 | 380 800 |
| Tear Resistance, lb | ASTM D 1004 | 45,000 lbs | 22 | 33 | 44 | 55 |
| Puncture Resistance, lb | ASTM D 4833 | 45,000 lbs | 56 | 84 | 112 | 140 |
| Carbon Black Content ⁽¹⁾ , % (Range) | ASTM D 1603*/4218 | 20,000 lbs | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 |
| Carbon Black Dispersion | ASTM D 5596 | 45,000 lbs | Note ⁽²⁾ | Note ⁽²⁾ | Note ⁽²⁾ | Note ⁽²⁾ |
| Oxidative Induction Time, min | ASTM D 3895, 200°C; O ₂ , 1 atm | 200,000 lbs | >100 | >100 | >100 | >100 |
| Typical Roll Dimensions | | | | | | |
| Roll Length ⁽³⁾ , ft | | | 870 | 560 | 430 | 340 |
| Roll Width ⁽³⁾ , ft | | | 22.5 | 22.5 | 22.5 | 22.5 |
| Roll Area, ft ² | | | 19,575 | 12,600 | 9,675 | 7,650 |

NOTES:

- ⁽¹⁾GSE UltraFlex Leak Location White Smooth may have an overall ash content greater than 3.0% due to the white and conductive layers. These values apply to the non-conductive black layer only.
- ⁽²⁾Dispersion only applies to near spherical agglomerates. 9 of 10 views shall be Category 1 or 2. No more than 1 view from Category 3.
- ⁽³⁾Roll lengths and widths have a tolerance of $\pm 1\%$.
- GSE UltraFlex Leak Location White Smooth is available in rolls weighing approximately 4,000 lb.
- All GSE geomembranes have dimensional stability of $\pm 2\%$ when tested according to ASTM D 1204 and LTB of $< -77^{\circ}\text{C}$ when tested according to ASTM D 746.
- *Modified.



- E. Textured surfaced geomembrane shall meet the requirements shown in the following data sheets below.
1. *Table 2.1* for Black coextruded textured HDPE
 2. *Table 2.2* for Green coextruded textured HDPE
 3. *Table 2.3* for White coextruded textured HDPE
 - a. The geomembrane shall be a white-surfaced, coextruded geomembrane.
 - b. The white surface shall be installed upwards.
 4. *Table 2.4* for Leak Location Liner coextruded textured HDPE
 - a. The geomembrane shall be a white-surfaced, coextruded geomembrane.
 - b. The white surface shall be installed upwards.
 5. *Table 2.4* for White Leak Location Liner coextruded textured HDPE
 - a. The geomembrane shall be a white-surfaced, coextruded geomembrane.
 - b. The white surface shall be installed upwards.
 6. *Table 2.6* for Black coextruded textured LLDPE
 7. *Table 2.7* for White coextruded textured LLDPE
 - a. The geomembrane shall be a white-surfaced, coextruded geomembrane.
 - b. The white surface shall be installed upwards.
 8. *Table 2.8* for Leak Location Liner coextruded textured LLDPE
 - a. The geomembrane shall have a coextruded, electrically conductive layer.
 - b. The conductive layer is installed downward.
 - c. Electrical testing shall be performed after liner installation by the INSTALLER.
 9. *Table 2.9* for White Leak Location Liner coextruded textured LLDPE
 - a. The geomembrane shall be a white-surfaced, coextruded geomembrane.
 - b. The white surface shall be installed upwards.
 - c. The geomembrane shall have a coextruded, electrically conductive layer.
 - d. The conductive layer is installed downward.
 - e. Electrical testing shall be performed after liner installation by the INSTALLER.



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TABLE 2.1: GSE HD TEXTURED GEOMEMBRANE

| Tested Property | Test Method | Frequency | Minimum Average Values | | | | |
|---|---|-------------|------------------------|-----------------------|------------------------|-------------------------|-------------------------|
| | | | 30 mil | 40 mil | 60 mil | 80 mil | 100 mil |
| Thickness, mil Lowest individual reading | ASTM D 5994 | every roll | 30 27 | 40 36 | 60 54 | 80 72 | 100 90 |
| Density, g/cm ³ , (min.) | ASTM D 1505 | 200,000 lbs | 0.940 | 0.940 | 0.940 | 0.940 | 0.940 |
| Tensile Properties (each direction) Strength at Break, lb/in-width Strength at Yield, lb/in-width Elongation at Break, % Elongation at Yield, % | ASTM D 6693, Type IV Dumbbell, 2 ipm G.L. 2.0 in G.L. 1.3 in | 20,000 lbs | 45 63 100 12 | 60 84 100 12 | 90 126 100 12 | 120 168 100 12 | 150 210 100 12 |
| Tear Resistance, lb | ASTM D 1004 | 45,000 lbs | 21 | 28 | 42 | 56 | 70 |
| Puncture Resistance, lb | ASTM D 4833 | 45,000 lbs | 45 | 60 | 90 | 120 | 150 |
| Carbon Black Content, % (Range) | ASTM D 1603*/4218 | 20,000 lbs | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 |
| Carbon Black Dispersion | ASTM D 5596 | 45,000 lbs | Note ⁽¹⁾ | Note ⁽¹⁾ | Note ⁽¹⁾ | Note ⁽¹⁾ | Note ⁽¹⁾ |
| Asperity Height, mil | ASTM D 7466 | second roll | 16 | 18 | 18 | 18 | 18 |
| Notch Constant Tensile Load ⁽²⁾ , hr | ASTM D 5397, Appendix | 200,000 lbs | 300 | 300 | 300 | 300 | 300 |
| Oxidative Induction Time, min | ASTM D 3895, 200°C; O ₂ , 1 atm | 200,000 lbs | >100 | >100 | >100 | >100 | >100 |
| Typical Roll Dimensions | | | | | | | |
| Roll Length ⁽³⁾ , ft | Double-Sided Textured | | 830 | 700 | 520 | 400 | 330 |
| | Single-Sided Textured | | 1,010 | 780 | 540 | 410 | 330 |
| Roll Width ⁽³⁾ , ft | | | 22.5 | 22.5 | 22.5 | 22.5 | 22.5 |
| Roll Area, ft ² | Double-Sided Textured | | 18,675 | 15,750 | 11,700 | 9,000 | 7,425 |
| | Single-Sided Textured | | 22,725 | 17,550 | 12,150 | 9,225 | 7,425 |

NOTES:

- ⁽¹⁾Dispersion only applies to near spherical agglomerates. 9 of 10 views shall be Category 1 or 2. No more than 1 view from Category 3.
- ⁽²⁾NCTL for GSE HD Textured is conducted on representative smooth geomembrane samples.
- ⁽³⁾Roll lengths and widths have a tolerance of ± 1%.
- GSE HD Textured is available in rolls weighing approximately 4,000 lb.
- All GSE geomembranes have dimensional stability of ±2% when tested according to ASTM D 1204 and LTB of <-77° C when tested according to ASTM D 746.
- *Modified.



TABLE 2.2 GSE GREEN TEXTURED GEOMEMBRANE

| Tested Property | Test Method | Frequency | Minimum Average Values | | | | |
|---|---|-------------|------------------------|-----------------------|------------------------|-------------------------|-------------------------|
| | | | 30 mil | 40 mil | 60 mil | 80 mil | 100 mil |
| Thickness, mil Lowest individual reading | ASTM D 5994 | every roll | 30 27 | 40 36 | 60 54 | 80 72 | 100 90 |
| Density, g/cm ³ , (min.) | ASTM D 1505 | 200,000 lbs | 0.940 | 0.940 | 0.940 | 0.940 | 0.940 |
| Tensile Properties (each direction) Strength at Break, lb/in-width Strength at Yield, lb/in-width Elongation at Break, % Elongation at Yield, % | ASTM D 6693, Type IV Dumbbell, 2 ipm G.L. 2.0 in G.L. 1.3 in | 20,000 lbs | 45 63 100 12 | 60 84 100 12 | 90 126 100 12 | 120 168 100 12 | 150 210 100 12 |
| Tear Resistance, lb | ASTM D 1004 | 45,000 lbs | 21 | 28 | 42 | 56 | 70 |
| Puncture Resistance, lb | ASTM D 4833 | 45,000 lbs | 45 | 60 | 90 | 120 | 150 |
| Carbon Black Content ⁽¹⁾ , % (Range) | ASTM D 1603*/4218 | 20,000 lbs | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 |
| Carbon Black Dispersion | ASTM D 5596 | 45,000 lbs | Note ⁽²⁾ | Note ⁽²⁾ | Note ⁽²⁾ | Note ⁽²⁾ | Note ⁽²⁾ |
| Asperity Height, mil | ASTM D 7466 | second roll | 16 | 18 | 18 | 18 | 18 |
| Notch Constant Tensile Load ⁽³⁾ , hr | ASTM D 5397, Appendix | 200,000 lbs | 300 | 300 | 300 | 300 | 300 |
| Oxidative Induction Time, min | ASTM D 3895, 200°C; O ₂ , 1 atm | 200,000 lbs | >100 | >100 | >100 | >100 | >100 |
| Typical Roll Dimensions | | | | | | | |
| Roll Length ⁽⁴⁾ , ft | Double-Sided Textured | | 830 | 700 | 520 | 400 | 330 |
| | Single-Sided Textured | | 1,010 | 780 | 540 | 410 | 330 |
| Roll Width ⁽⁴⁾ , ft | | | 22.5 | 22.5 | 22.5 | 22.5 | 22.5 |
| Roll Area, ft ² | Double-Sided Textured | | 18,675 | 15,750 | 11,700 | 9,000 | 7,425 |
| | Single-Sided Textured | | 22,725 | 17,550 | 12,150 | 9,225 | 7,425 |

NOTES:

- ⁽¹⁾GSE Green may have an overall ash content greater than 3.0% due to the green layer. These values apply to the black layer only.
- ⁽²⁾Dispersion only applies to near spherical agglomerates. 9 of 10 views shall be Category 1 or 2. No more than 1 view from Category 3.
- ⁽³⁾NCTL for GSE Green Textured is conducted on representative smooth geomembrane samples.
- ⁽⁴⁾Roll lengths and widths have a tolerance of ±1%.
- GSE Green Textured is available in rolls weighing approximately 4,000 lb.
- All GSE geomembranes have dimensional stability of ±2% when tested according to ASTM D 1204 and LTB of <-77° C when tested according to ASTM D 746.
- *Modified.



TABLE 2.3: GSE WHITE TEXTURED GEOMEMBRANE

| Tested Property | Test Method | Frequency | Minimum Average Values | | | | |
|---|---|-------------|------------------------|-----------------------|------------------------|-------------------------|-------------------------|
| | | | 30 mil | 40 mil | 60 mil | 80 mil | 100 mil |
| Thickness, mil Lowest individual reading | ASTM D 5994 | every roll | 30 27 | 40 36 | 60 54 | 80 72 | 100 90 |
| Density, g/cm ³ , (min.) | ASTM D 1505 | 200,000 lbs | 0.940 | 0.940 | 0.940 | 0.940 | 0.940 |
| Tensile Properties (each direction) Strength at Break, lb/in-width Strength at Yield, lb/in-width Elongation at Break, % Elongation at Yield, % | ASTM D 6693, Type IV Dumbbell, 2 ipm G.L. 2.0 in G.L. 1.3 in | 20,000 lbs | 45 63 100 12 | 60 84 100 12 | 90 126 100 12 | 120 168 100 12 | 150 210 100 12 |
| Tear Resistance, lb | ASTM D 1004 | 45,000 lbs | 21 | 28 | 42 | 56 | 70 |
| Puncture Resistance, lb | ASTM D 4833 | 45,000 lbs | 45 | 60 | 90 | 120 | 150 |
| Carbon Black Content ⁽¹⁾ , % (Range) | ASTM D 1603*/4218 | 20,000 lbs | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 |
| Carbon Black Dispersion | ASTM D 5596 | 45,000 lbs | Note ⁽²⁾ | Note ⁽²⁾ | Note ⁽²⁾ | Note ⁽²⁾ | Note ⁽²⁾ |
| Asperity Height, mil | ASTM D 7466 | second roll | 16 | 18 | 18 | 18 | 18 |
| Notch Constant Tensile Load ⁽³⁾ , hr | ASTM D 5397, Appendix | 200,000 lbs | 300 | 300 | 300 | 300 | 300 |
| Oxidative Induction Time, min | ASTM D 3895, 200°C; O ₂ , 1 atm | 200,000 lbs | >100 | >100 | >100 | >100 | >100 |
| Typical Roll Dimensions | | | | | | | |
| Roll Length ⁽⁴⁾ , ft | Double-Sided Textured | | 830 | 700 | 520 | 400 | 330 |
| | Single-Sided Textured | | 1,010 | 780 | 540 | 410 | 330 |
| Roll Width ⁽⁴⁾ , ft | | | 22.5 | 22.5 | 22.5 | 22.5 | 22.5 |
| Roll Area, ft ² | Double-Sided Textured | | 18,675 | 15,750 | 11,700 | 9,000 | 7,425 |
| | Single-Sided Textured | | 22,725 | 17,550 | 12,150 | 9,225 | 7,425 |

NOTES:

- ⁽¹⁾GSE White may have an overall ash content greater than 3.0% due to the white layer. These values apply to the black layer only.
- ⁽²⁾Dispersion only applies to near spherical agglomerates. 9 of 10 views shall be Category 1 or 2. No more than 1 view from Category 3.
- ⁽³⁾NCTL for GSE White Textured is conducted on representative smooth geomembrane samples.
- ⁽⁴⁾Roll lengths and widths have a tolerance of ±1%.
- GSE White Textured is available in rolls weighing approximately 4,000 lb.
- All GSE geomembranes have dimensional stability of ±2% when tested according to ASTM D 1204 and LTB of <-77° C when tested according to ASTM D 746.
- *Modified.



TABLE 2.4: GSE LEAK LOCATION LINER TEXTURED GEOMEMBRANE

| Tested Property | Test Method | Frequency | Minimum Average Values | | | |
|---|---|-------------|------------------------|------------------------|-------------------------|-------------------------|
| | | | 40 mil | 60 mil | 80 mil | 100 mil |
| Thickness, mil Lowest individual reading | ASTM D 5994 | every roll | 40 36 | 60 54 | 80 72 | 100 90 |
| Density, g/cm ³ , (min.) | ASTM D 1505 | 200,000 lbs | 0.940 | 0.940 | 0.940 | 0.940 |
| Tensile Properties (each direction) Strength at Break, lb/in-width Strength at Yield, lb/in-width Elongation at Break, % Elongation at Yield, % | ASTM D 6693, Type IV Dumbbell, 2 ipm G.L. 2.0 in G.L. 1.3 in | 20,000 lbs | 60 84 100 12 | 90 126 100 12 | 120 168 100 12 | 150 210 100 12 |
| Tear Resistance, lb | ASTM D 1004 | 45,000 lbs | 28 | 42 | 56 | 70 |
| Puncture Resistance, lb | ASTM D 4833 | 45,000 lbs | 60 | 90 | 120 | 150 |
| Carbon Black Content ⁽¹⁾ , % (Range) | ASTM D 1603*/4218 | 20,000 lbs | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 |
| Carbon Black Dispersion | ASTM D 5596 | 45,000 lbs | Note ⁽²⁾ | Note ⁽²⁾ | Note ⁽²⁾ | Note ⁽²⁾ |
| Asperity Height, mil | ASTM D 7466 | second roll | 18 | 18 | 18 | 18 |
| Notch Constant Tensile Load ⁽³⁾ , hr | ASTM D 5397, Appendix | 200,000 lbs | 300 | 300 | 300 | 300 |
| Oxidative Induction Time, min | ASTM D 3895, 200°C; O ₂ , 1 atm | 200,000 lbs | >100 | >100 | >100 | >100 |
| Typical Roll Dimensions | | | | | | |
| Roll Length ⁽⁴⁾ , ft | Double-Sided Textured | | 700 | 520 | 400 | 330 |
| | Single-Sided Textured | | 780 | 540 | 410 | 330 |
| Roll Width ⁽⁴⁾ , ft | | | 22.5 | 22.5 | 22.5 | 22.5 |
| Roll Area, ft ² | Double-Sided Textured | | 15,750 | 11,700 | 9,000 | 7,425 |
| | Single-Sided Textured | | 17,550 | 12,150 | 9,225 | 7,425 |

NOTES:

- ⁽¹⁾GSE Leak Location may have an overall ash content greater than 3.0% due to the conductive layer. These values apply to the non-conductive layer only.
- ⁽²⁾Dispersion only applies to near spherical agglomerates. 9 of 10 views shall be Category 1 or 2. No more than 1 view from Category 3.
- ⁽³⁾NCTL for GSE Leak Location Textured is conducted on representative smooth geomembrane samples.
- ⁽⁴⁾Roll lengths and widths have a tolerance of ±1%.
- GSE Leak Location Textured is available in rolls weighing approximately 4,000 lb.
- All GSE geomembranes have dimensional stability of ±2% when tested according to ASTM D 1204 and LTB of <-77° C when tested according to ASTM D 746.
- *Modified.



TABLE 2.5: GSE LEAK LOCATION LINER WHITE TEXTURED GEOMEMBRANE

| Tested Property | Test Method | Frequency | Minimum Average Values | | | |
|---|---|-------------|------------------------|------------------------|-------------------------|-------------------------|
| | | | 40 mil | 60 mil | 80 mil | 100 mil |
| Thickness, mil Lowest individual reading | ASTM D 5994 | every roll | 40 36 | 60 54 | 80 72 | 100 90 |
| Density, g/cm ³ , (min.) | ASTM D 1505 | 200,000 lbs | 0.940 | 0.940 | 0.940 | 0.940 |
| Tensile Properties (each direction) Strength at Break, lb/in-width Strength at Yield, lb/in-width Elongation at Break, % Elongation at Yield, % | ASTM D 6693, Type IV Dumbbell, 2 ipm G.L. 2.0 in G.L. 1.3 in | 20,000 lbs | 60 84 100 12 | 90 126 100 12 | 120 168 100 12 | 150 210 100 12 |
| Tear Resistance, lb | ASTM D 1004 | 45,000 lbs | 28 | 42 | 56 | 70 |
| Puncture Resistance, lb | ASTM D 4833 | 45,000 lbs | 60 | 90 | 120 | 150 |
| Carbon Black Content ⁽¹⁾ , % (Range) | ASTM D 1603*/4218 | 20,000 lbs | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 |
| Carbon Black Dispersion | ASTM D 5596 | 45,000 lbs | Note ⁽²⁾ | Note ⁽²⁾ | Note ⁽²⁾ | Note ⁽²⁾ |
| Asperity Height, mil | ASTM D 7466 | second roll | 18 | 18 | 18 | 18 |
| Notch Constant Tensile Load ⁽²⁾ , hr | ASTM D 5397, Appendix | 200,000 lbs | 300 | 300 | 300 | 300 |
| Oxidative Induction Time, min | ASTM D 3895, 200°C; O ₂ , 1 atm | 200,000 lbs | >100 | >100 | >100 | >100 |
| Typical Roll Dimensions | | | | | | |
| Roll Length ⁽⁴⁾ , ft | Double-Sided Textured | | 700 | 520 | 400 | 330 |
| | Single-Sided Textured | | 780 | 540 | 410 | 330 |
| Roll Width ⁽⁴⁾ , ft | | | 22.5 | 22.5 | 22.5 | 22.5 |
| Roll Area, ft ² | Double-Sided Textured | | 15,750 | 11,700 | 9,000 | 7,425 |
| | Single-Sided Textured | | 17,550 | 12,150 | 9,225 | 7,425 |

NOTES:

- ⁽¹⁾GSE Leak Location White may have an overall ash content greater than 3.0% due to the conductive and white layers. These values apply to the non-conductive black layer only.
- ⁽²⁾Dispersion only applies to near spherical agglomerates. 9 of 10 views shall be Category 1 or 2. No more than 1 view from Category 3.
- ⁽³⁾NCTL for GSE Leak Location White Textured is conducted on representative smooth geomembrane samples.
- ⁽⁴⁾Roll lengths and widths have a tolerance of ±1%.
- GSE Leak Location White Textured is available in rolls weighing approximately 4,000 lb.
- All GSE geomembranes have dimensional stability of ±2% when tested according to ASTM D 1204 and LTB of <-77° C when tested according to ASTM D 746.
- *Modified.



TABLE 2.6: GSE ULTRAFLEX TEXTURED GEOMEMBRANE

| Tested Property | Test Method | Frequency | Minimum Average Values | | | |
|---|--|-------------|------------------------|---------------------|---------------------|---------------------|
| | | | 40 mil | 60 mil | 80 mil | 100 mil |
| Thickness, mil Lowest individual reading | ASTM D 5199 | every roll | 40 36 | 60 54 | 80 72 | 100 90 |
| Density, g/cm ³ (max.) | ASTM D 1505 | 200,000 lbs | 0.939 | 0.939 | 0.939 | 0.939 |
| Tensile Properties (each direction) Strength at Break, lb/in-width Elongation at Break, % | ASTM D 6693, Type IV Dumbbell, 2 ipm G.L. 2.0 in | 20,000 lbs | 60 250 | 90 250 | 120 250 | 150 250 |
| Tear Resistance, lb | ASTM D 1004 | 45,000 lbs | 22 | 33 | 44 | 55 |
| Puncture Resistance, lb | ASTM D 4833 | 45,000 lbs | 44 | 66 | 88 | 110 |
| Carbon Black Content, % (Range) | ASTM D 1603*/4218 | 20,000 lbs | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 |
| Carbon Black Dispersion | ASTM D 5596 | 45,000 lbs | Note ⁽¹⁾ | Note ⁽¹⁾ | Note ⁽¹⁾ | Note ⁽¹⁾ |
| Asperity Height, mil | ASTM D 7466 | second roll | 18 | 18 | 18 | 18 |
| Oxidative Induction Time, min | ASTM D 3895, 200°C; O ₂ , 1 atm | 200,000 lbs | >100 | >100 | >100 | >100 |
| Typical Roll Dimensions | | | | | | |
| Roll Length ⁽²⁾ , ft | Double-Sided Textured | | 700 | 520 | 400 | 330 |
| | Single-Sided Textured | | 650 | 420 | 320 | 250 |
| Roll Width ⁽²⁾ , ft | | | 22.5 | 22.5 | 22.5 | 22.5 |
| Roll Area, ft ² | Double-Sided Textured | | 15,750 | 11,700 | 9,000 | 7,425 |
| | Single-Sided Textured | | 14,625 | 9,450 | 7,200 | 5,625 |

NOTES:

- ⁽¹⁾Dispersion only applies to near spherical agglomerates. 9 of 10 views shall be Category 1 or 2. No more than 1 view from Category 3.
- ⁽²⁾Roll lengths and widths have a tolerance of $\pm 1\%$.
- GSE UltraFlex Textured is available in rolls weighing approximately 4,000 lb.
- All GSE geomembranes have dimensional stability of $\pm 2\%$ when tested according to ASTM D 1204 and LTB of $< -77^{\circ}\text{C}$ when tested according to ASTM D 746.
- *Modified.



| TABLE 2.7: GSE ULTRAFLEX WHITE TEXTURED GEOMEMBRANE | | | | | | |
|---|--|-------------|------------------------|---------------------|---------------------|---------------------|
| Tested Property | Test Method | Frequency | Minimum Average Values | | | |
| | | | 40 mil | 60 mil | 80 mil | 100 mil |
| Thickness, mil Lowest individual reading | ASTM D 5199 | every roll | 40 36 | 60 54 | 80 72 | 100 90 |
| Density, g/cm ³ (max.) | ASTM D 1505 | 200,000 lbs | 0.939 | 0.939 | 0.939 | 0.939 |
| Tensile Properties (each direction) Strength at Break, lb/in-width Elongation at Break, % | ASTM D 6693, Type IV Dumbbell, 2 ipm G.L. 2.0 in | 20,000 lbs | 60 250 | 90 250 | 120 250 | 150 250 |
| Tear Resistance, lb | ASTM D 1004 | 45,000 lbs | 22 | 33 | 44 | 55 |
| Puncture Resistance, lb | ASTM D 4833 | 45,000 lbs | 44 | 66 | 88 | 110 |
| Carbon Black Content ⁽¹⁾ , % (Range) | ASTM D 1603*/4218 | 20,000 lbs | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 |
| Carbon Black Dispersion | ASTM D 5596 | 45,000 lbs | Note ⁽²⁾ | Note ⁽²⁾ | Note ⁽²⁾ | Note ⁽²⁾ |
| Asperity Height, mil | ASTM D 7466 | second roll | 18 | 18 | 18 | 18 |
| Oxidative Induction Time, min | ASTM D 3895, 200°C; O ₂ , 1 atm | 200,000 lbs | >100 | >100 | >100 | >100 |
| Typical Roll Dimensions | | | | | | |
| Roll Length ⁽³⁾ , ft | Double-Sided Textured Single-Sided Textured | | 700 650 | 520 420 | 400 320 | 330 250 |
| Roll Width ⁽³⁾ , ft | | | 22.5 | 22.5 | 22.5 | 22.5 |
| Roll Area, ft ² | Double-Sided Textured Single-Sided Textured | | 15,750 14,625 | 11,700 9,450 | 9,000 7,200 | 7,425 5,625 |

NOTES:

- ⁽¹⁾GSE UltraFlex White Textured may have an overall ash content greater than 3.0% due to the white layer. These values apply to the black layer only.
- ⁽²⁾Dispersion only applies to near spherical agglomerates. 9 of 10 views shall be Category 1 or 2. No more than 1 view from Category 3.
- ⁽³⁾Roll lengths and widths have a tolerance of $\pm 1\%$.
- GSE UltraFlex White Textured is available in rolls weighing approximately 4,000 lb.
- All GSE geomembranes have dimensional stability of $\pm 2\%$ when tested according to ASTM D 1204 and LTB of $< -77^{\circ}\text{C}$ when tested according to ASTM D 746.
- *Modified.



TABLE 2.8: GSE ULTRAFLEX LEAK LOCATION TEXTURED GEOMEMBRANE

| Tested Property | Test Method | Frequency | Minimum Average Values | | | |
|---|--|-------------|------------------------|---------------------|---------------------|---------------------|
| | | | 40 mil | 60 mil | 80 mil | 100 mil |
| Thickness, mil Lowest individual reading | ASTM D 5199 | every roll | 40 36 | 60 54 | 80 72 | 100 90 |
| Density, g/cm ³ (max.) | ASTM D 1505 | 200,000 lbs | 0.939 | 0.939 | 0.939 | 0.939 |
| Tensile Properties (each direction) Strength at Break, lb/in-width Elongation at Break, % | ASTM D 6693, Type IV Dumbbell, 2 ipm G.L. 2.0 in | 20,000 lbs | 60 250 | 90 250 | 120 250 | 150 250 |
| Tear Resistance, lb | ASTM D 1004 | 45,000 lbs | 22 | 33 | 44 | 55 |
| Puncture Resistance, lb | ASTM D 4833 | 45,000 lbs | 44 | 66 | 88 | 110 |
| Carbon Black Content ⁽¹⁾ , % (Range) | ASTM D 1603*/4218 | 20,000 lbs | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 |
| Carbon Black Dispersion | ASTM D 5596 | 45,000 lbs | Note ⁽²⁾ | Note ⁽²⁾ | Note ⁽²⁾ | Note ⁽²⁾ |
| Asperity Height, mil | ASTM D 7466 | second roll | 18 | 18 | 18 | 18 |
| Oxidative Induction Time, min | ASTM D 3895, 200°C; O ₂ , 1 atm | 200,000 lbs | >100 | >100 | >100 | >100 |
| Typical Roll Dimensions | | | | | | |
| Roll Length ⁽³⁾ , ft | Double-Sided Textured | | 700 | 520 | 400 | 330 |
| | Single-Sided Textured | | 650 | 420 | 320 | 250 |
| Roll Width ⁽³⁾ , ft | | | 22.5 | 22.5 | 22.5 | 22.5 |
| Roll Area, ft ² | Double-Sided Textured | | 15,750 | 11,700 | 9,000 | 7,425 |
| | Single-Sided Textured | | 14,625 | 9,450 | 7,200 | 5,625 |

NOTES:

- ⁽¹⁾GSE UltraFlex Leak Location Textured may have an overall ash content greater than 3.0% due to the conductive layer. These values apply to the non-conductive black layer only.
- ⁽²⁾Dispersion only applies to near spherical agglomerates. 9 of 10 views shall be Category 1 or 2. No more than 1 view from Category 3.
- ⁽³⁾Roll lengths and widths have a tolerance of ±1%.
- GSE UltraFlex Leak Location Textured is available in rolls weighing approximately 4,000 lb.
- All GSE geomembranes have dimensional stability of ±2% when tested according to ASTM D 1204 and LTB of <-77°C when tested according to ASTM D 746.
- *Modified.



| TABLE 2.9: GSE ULTRAFLEX LEAK LOCATION WHITE TEXTURED GEOMEMBRANE | | | | | | |
|---|--|-------------|------------------------|---------------------|---------------------|---------------------|
| Tested Property | Test Method | Frequency | Minimum Average Values | | | |
| | | | 40 mil | 60 mil | 80 mil | 100 mil |
| Thickness, mil Lowest individual reading | ASTM D 5199 | every roll | 40 36 | 60 54 | 80 72 | 100 90 |
| Density, g/cm ³ (max.) | ASTM D 1505 | 200,000 lbs | 0.939 | 0.939 | 0.939 | 0.939 |
| Tensile Properties (each direction) Strength at Break, lb/in-width Elongation at Break, % | ASTM D 6693, Type IV Dumbbell, 2 ipm G.L. 2.0 in | 20,000 lbs | 60 250 | 90 250 | 120 250 | 150 250 |
| Tear Resistance, lb | ASTM D 1004 | 45,000 lbs | 22 | 33 | 44 | 55 |
| Puncture Resistance, lb | ASTM D 4833 | 45,000 lbs | 44 | 66 | 88 | 110 |
| Carbon Black Content ⁽¹⁾ , % (Range) | ASTM D 1603*/4218 | 20,000 lbs | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 | 2.0 - 3.0 |
| Carbon Black Dispersion | ASTM D 5596 | 45,000 lbs | Note ⁽²⁾ | Note ⁽²⁾ | Note ⁽²⁾ | Note ⁽²⁾ |
| Asperity Height, mil | ASTM D 7466 | second roll | 18 | 18 | 18 | 18 |
| Oxidative Induction Time, min | ASTM D 3895, 200°C; O ₂ , 1 atm | 200,000 lbs | >100 | >100 | >100 | >100 |
| Typical Roll Dimensions | | | | | | |
| Roll Length ⁽³⁾ , ft | Double-Sided Textured Single-Sided Textured | | 700 650 | 520 420 | 400 320 | 330 250 |
| Roll Width ⁽³⁾ , ft | | | 22.5 | 22.5 | 22.5 | 22.5 |
| Roll Area, ft ² | Double-Sided Textured Single-Sided Textured | | 15,750 14,625 | 11,700 9,450 | 9,000 7,200 | 7,425 5,625 |

NOTES:

- ⁽¹⁾GSE UltraFlex Leak Location White Textured may have an overall ash content greater than 3.0% due to the white and conductive layers. These values apply to the non-conductive black layer only.
- ⁽²⁾Dispersion only applies to near spherical agglomerates. 9 of 10 views shall be Category 1 or 2. No more than 1 view from Category 3.
- ⁽³⁾Roll lengths and widths have a tolerance of ±1%.
- GSE UltraFlex Leak Location White Textured is available in rolls weighing approximately 4,000 lb.
- All GSE geomembranes have dimensional stability of ±2% when tested according to ASTM D 1204 and LTB of <-77°C when tested according to ASTM D 746.
- *Modified.

F. Extrudate Rod or Bead

1. Extrudate material shall be made from same type resin as the geomembrane.
2. Additives shall be thoroughly dispersed.
3. Materials shall be free of contamination by moisture or foreign matter.

1.9 EQUIPMENT

A. Welding equipment and accessories shall meet the following requirements:

1. Gauges showing temperatures in apparatus such as extrusion welder or fusion welder shall be present.
2. An adequate number of welding apparatus shall be available to avoid delaying work.
3. Power source must be capable of providing constant voltage under combined line load.

1.10 DEPLOYMENT

- A. Assign each panel a simple and logical identifying code. The coding system shall be subject to approval and shall be determined at the job site.
- B. Visually inspect the geomembrane during deployment for imperfections and mark faulty or suspect areas.
- C. Deployment of geomembrane panels shall be performed in a manner that will comply with the following guidelines:

1. Geomembranes shall be installed according to site-specific specifications, and GSE Conductive should be installed with the Conductive layer down.

Note: A spark tester or ohm meter can be used to determine Conductive layer.

2. Unroll geomembrane using methods that will not damage geomembrane and will protect underlying surface from damage (spreader bar, protected equipment bucket).
3. Place ballast (commonly sandbags) on geomembrane which will not damage geomembrane to prevent wind uplift.
4. Personnel walking on geomembrane shall not engage in activities or wear shoes that could damage it. Smoking will not be permitted on the geomembrane.



5. Do not allow heavy vehicular traffic directly on geomembrane. Rubber-tired ATV's and trucks are acceptable if wheel contact is less than 8 psi.
 6. Protect geomembrane in areas of heavy traffic by placing protective cover over the geomembrane.
- D. Sufficient material (slack) shall be provided to allow for thermal expansion and contraction of the material.

1.11 FIELD SEAMING

- A. Seams shall meet the following requirements:
1. To the maximum extent possible, orient seams parallel to the line of the slope, i.e., down and not across slope.
 2. Minimize number of field seams in corners, odd-shaped geometric locations, and outside corners.
 3. Slope seams (panels) shall extend a minimum of 5-ft beyond the grade break into the flat area.
 4. Use a sequential seam numbering system compatible with panel numbering system that is agreeable to the CONSULTANT and INSTALLER.
 5. Align seam overlaps consistent with the requirements of the welding equipment being used. A 6-in overlap is commonly suggested.
- B. During Welding Operations
1. Provide at least one Master Seamer who shall provide direct supervision over other welders as necessary.
- C. Extrusion Welding
1. Hot-air tack adjacent pieces together using procedures that do not damage the geomembrane.
 2. Clean geomembrane surfaces by disc grinder or equivalent.
 3. Purge welding apparatus of heat-degraded extrudate before welding.
- D. Hot Wedge Welding
1. Welding apparatus shall be a self-propelled device equipped with an electronic controller which displays applicable temperatures.



2. Clean seam area of dust, mud, moisture and debris immediately ahead of hot wedge welder.
3. Protect against moisture build-up between sheets.

E. Trial Welds

1. Perform trial welds on geomembrane samples to verify welding equipment is operating properly.
2. Make trial welds under the same surface and environmental conditions as the production welds, i.e., in contact with subgrade and similar ambient temperature.
3. Minimum of two trial welds per day, per welding apparatus, one made prior to the start of work and one completed at mid shift.
4. Cut four, 1-in wide by 6-in long test strips from the trial weld.
5. Quantitatively test specimens for peel adhesion, and then for shear strength.
6. Trial weld specimens shall pass when the results shown in the following tables for HDPE and LLDPE are achieved in both peel and shear test.

| TABLE 1.12.6A: MINIMUM WELD VALUES FOR HDPE GEOMEMBRANES | | | | | | | |
|--|-------------|----|----|-----|-----|-----|-----|
| Property | Test Method | 30 | 40 | 60 | 80 | 100 | 120 |
| Peel Strength (fusion), ppi | ASTM D 6392 | 49 | 65 | 98 | 130 | 162 | 196 |
| Peel Strength (extrusion), ppi | ASTM D 6392 | 39 | 52 | 78 | 104 | 130 | 157 |
| Shear Strength (fusion & ext.), ppi | ASTM D 6392 | 61 | 81 | 121 | 162 | 203 | 242 |

| TABLE 1.2.6B: MINIMUM WELD VALUES FOR LLDPE GEOMEMBRANES | | | | | | |
|--|-------------|----|----|----|-----|-----|
| Property | Test Method | 30 | 40 | 60 | 80 | 100 |
| Peel Strength (extrusion), ppi | ASTM D 6392 | 36 | 48 | 72 | 96 | 120 |
| Peel Strength (fusion), ppi | ASTM D 6392 | 38 | 50 | 75 | 100 | 125 |
| Shear Strength (fusion & ext.), ppi | ASTM D 6392 | 45 | 60 | 90 | 120 | 150 |

- a. The break, when peel testing, occurs in the liner material itself, not through peel separation (FTB).
 - b. The break is ductile.
7. Repeat the trial weld, in its entirety, when any of the trial weld samples fail in either peel or shear.



8. No welding equipment or welder shall be allowed to perform production welds until equipment and welders have successfully completed trial weld.
- F. Seaming shall not proceed when ambient air temperature or adverse weather conditions jeopardize the integrity of the liner installation. INSTALLER shall demonstrate that acceptable seaming can be performed by completing acceptable trial welds.
- G. Defects and Repairs
 1. Examine all seams and non-seam areas of the geomembrane for defects, holes, blisters, undispersed raw materials, and any sign of contamination by foreign matter.
 2. Repair and non-destructively test each suspect location in both seam and non-seam areas. Do not cover geomembrane at locations that have been repaired until test results with passing values are available.

1.12 FIELD QUALITY ASSURANCE

- A. MANUFACTURER and INSTALLER shall participate in and conform to all terms and requirements of the Owner's quality assurance program. CONTRACTOR shall be responsible for assuring this participation.
- B. Quality assurance requirements are as specified in this Section and in the Field Installation Quality Assurance Manual if it is included in the contract.
- C. Field Testing
 1. Non-destructive testing may be carried out as the seaming progresses or at completion of all field seaming.
 - a. Vacuum Testing
 - 1) Shall be performed in accordance with ASTM D 5641, Standard Practice for Geomembrane Seam Evaluation by Vacuum Chamber.
 - b. Air Pressure Testing
 - 1) Shall be performed in accordance with ASTM D 5820, Standard Practice for Pressurized Air Channel Evaluation of Dual Seamed Geomembranes.
 - c. Spark Testing
 - 1) Shall be performed accordance with ASTM D 7240 Standard Practice for Leak Location using Geomembranes with an Insulating Layer in Intimate



Contact with a Conductive Layer via Electrical Capacitance Technique (Conductive Geomembrane Spark Test).

- d. Other approved methods.
2. Destructive Testing (performed by CONSULTANT with assistance from INSTALLER)
 - a. Location and Frequency of Testing
 - 1) Collect destructive test samples at a frequency of one per every 500 lineal feet of seam length.
 - 2) Test locations will be determined after seaming.
 - 3) Exercise Method of Attributes as described by GRI GM-14 (Geosynthetic Research Institute, <http://www.geosynthetic-institute.org>) to minimize test samples taken.
 - b. Sampling Procedures are performed as follows:
 - 1) INSTALLER shall cut samples at locations designated by the CONSULTANT as the seaming progresses in order to obtain field laboratory test results before the geomembrane is covered.
 - 2) CONSULTANT will number each sample, and the location will be noted on the installation as-built.
 - 3) Samples shall be 12-in wide by minimal length with the seam centered lengthwise.
 - 4) Cut a 2-in wide strip from each end of the sample for field-testing.
 - 5) Cut the remaining sample into two parts for distribution as follows:
 - a) One portion for INSTALLER, 12-in by 12-in
 - b) One portion for the Third-Party laboratory, 12-in by 18-in
 - c) Additional samples may be archived if required.
 - 6) Destructive testing shall be performed in accordance with ASTM D 6392, Standard Test Method for Determining the Integrity of Non-Reinforced Geomembrane Seams Produced Using Thermo-Fusion Methods.
 - 7) INSTALLER shall repair all holes in the geomembrane resulting from destructive sampling.
 - 8) Repair and test the continuity of the repair in accordance with these Specifications.



3. Failed Seam Procedures

- a) If the seam fails, INSTALLER shall follow one of two options:
 - 1) Reconstruct the seam between any two passed test locations.
 - 2) Trace the weld to intermediate location at least 10-ft minimum or where the seam ends in both directions from the location of the failed test.
- b) The next seam welded using the same welding device is required to obtain an additional sample, i.e., if one side of the seam is less than 10-ft long.
- c) If sample passes, then the seam shall be reconstructed or capped between the test sample locations.
- d) If any sample fails, the process shall be repeated to establish the zone in which the seam shall be reconstructed.

1.13 REPAIR PROCEDURES

- A. Remove damaged geomembrane and replace with acceptable geomembrane materials if damage cannot be satisfactorily repaired.
- B. Repair any portion of unsatisfactory geomembrane or seam area failing a destructive or non-destructive test.
- C. INSTALLER shall be responsible for repair of defective areas.
- D. Agreement upon the appropriate repair method shall be decided between CONSULTANT and INSTALLER by using one of the following repair methods:
 - 1. Patching- Used to repair large holes, tears, undispersed raw materials and contamination by foreign matter.
 - 2. Abrading and Re-welding- Used to repair short section of a seam.
 - 3. Spot Welding- Used to repair pinholes or other minor, localized flaws or where geomembrane thickness has been reduced.
 - 4. Capping- Used to repair long lengths of failed seams.
 - 5. Flap Welding- Used to extrusion weld the flap (excess outer portion) of a fusion weld in lieu of a full cap.
 - 6. Remove the unacceptable seam and replace with new material.
- E. The following procedures shall be observed when a repair method is used:
 - 1. All geomembrane surfaces shall be clean and dry at the time of repair.



2. Surfaces of the polyethylene which are to be repaired by extrusion welds shall be lightly abraded to assure cleanliness.
3. Extend patches or caps at least 6 inches for extrusion welds and 4 inches for wedge welds beyond the edge of the defect, and around all corners of patch material.

F. Repair Verification

1. Number and log each patch repair (performed by CONSULTANT).
2. Non-destructively test each repair using methods specified in this Specification.



2 OZ GEOTEXTILE

1.1 SCOPE

This specification covers the technical requirements for the Manufacturing and Installation of the nonwoven geotextile. All materials meet or exceed the requirements of this specification, and all work will be performed in accordance with the procedures provided in these project specifications.

1.2 REFERENCES

- A. American Society for Testing and Materials (ASTM)
 - 1. ASTM D 5261, Standard Test Method for Measuring Mass per Unit Area of Geotextiles
 - 2. ASTM D 4632, Standard Test Method for Grab Breaking Load and Elongation of Geotextiles
 - 3. ASTM D 4533, Standard Test Method for Index Trapezoidal Tearing Strength of Geotextiles
 - 4. ASTM D 4833, Standard Test Method for Index Puncture Resistance of Geotextiles, Geomembranes and Related Products
 - 5. ASTM D 4491, Standard Test Method for Water Permeability of Geotextiles by Permittivity
 - 6. ASTM D 4751, Standard Test Method for Determining Apparent Opening Size of a Geotextile
 - 7. ASTM D 4354, Standard Practice for Sampling of Geosynthetics for Testing
 - 8. ASTM D 4759, Standard Practice for Determining the Specifications Conformance of Geosynthetics

1.3 SUBMITTALS

- A. Prior to material delivery to project site, the contractor shall provide the engineer with a written certification or manufacturers quality control data which displays that the geotextile meets or exceeds minimum average roll values (MARV) specified herein.
- B. The contractor shall submit, if required by the engineer, manufacturer's quality control manual for the geotextile to be delivered to the site.



2. PRODUCT

2.1 GEOTEXTILE

- A. The nonwoven needle-punched geotextile specified herein shall be made from staple fiber.
- B. The geotextile shall be manufactured from prime quality virgin polymer.
- C. The geotextile shall be able to withstand direct exposure to ultraviolet radiation from Sun for up to 30 days without any noticeable effect on index or performance properties.
- D. Geotextile shall meet or exceed all material properties listed in *Table 1*.

| TABLE 1: GEOTEXTILE PROPERTIES | | | |
|---|-------------|-------------------------|-------|
| Property | Test Method | Test Frequency | Value |
| Mass per Unit Area, oz/yd ² | ASTM D 5261 | 90,000-ft ² | 12 |
| Grab Tensile Strength, lb | ASTM D 4632 | 90,000-ft ² | 320 |
| CBR Puncture Strength, lb | ASTM D 6241 | 540,000-ft ² | 925 |
| Grab Elongation, % | ASTM D 4632 | 90,000-ft ² | 50 |
| Trapezoidal Tear Strength, lb | ASTM D 4533 | 90,000-ft ² | 125 |
| UV Resistance, % retained after 500 hours | ASTM D 4355 | per formulation | 70 |

2.2 MANUFACTURE

All rolls of the geotextile shall be identified with permanent marking on the roll or packaging, with the manufacturers name, product identification, roll number, and roll dimensions.



2.3 TRANSPORT

- A. Transportation of the geotextile shall be the responsibility of the contractor.
- B. During shipment, the geotextile shall be protected from ultraviolet light exposure, precipitation, mud, dirt, dust, puncture, or other damaging or deleterious conditions.
- C. Upon delivery at the job site, the contractor shall ensure that the geotextile rolls are handled and stored in accordance with the manufacturer's instructions as to prevent damage.

3. EXECUTION

3.1 QUALITY ASSURANCE

- A. The engineer shall examine the geotextile rolls upon delivery to the site and report any deviations from project specifications to the contractor.

3.2 INSTALLATION

- A. The geotextile shall be handled in such a manner as to ensure that it is not damaged in any way. Should the contractor damage the geotextile to the extent that it is no longer usable as determined by these specifications or by the engineer, the contractor shall replace the geotextile at his own cost.
- B. The geotextile shall be installed to the lines and grades as shown on the contract drawings and as described herein.
- C. The geotextile shall be rolled down the slope in such a manner as to continuously keep the geotextile in tension by self-weight. The geotextile shall be securely anchored in an anchor trench where applicable, or by other approved or specified methods.
- D. In the presence of wind, all geotextiles shall be weighted by sandbags or approved equivalent. Such anchors shall be installed during placement and shall remain in place until replaced with cover material.
- E. The contractor shall take necessary precautions to prevent damage to adjacent or underlying materials during placement of the geotextile. Should damage to such material occur due to the fault of the contractor, the latter shall repair the damaged materials at his own cost and to the satisfaction of the engineer.



- F. During placement of the geotextile, care shall be taken not to entrap soil, stones or excessive moisture that could hamper subsequent seaming of the geotextile as judged by the engineer.
- G. The geotextile shall not be exposed to precipitation prior to being installed and shall not be exposed to direct sunlight for more than 15 days after installation.
- H. The geotextile shall be seamed using heat seaming or stitching methods as recommended by the manufacturer and approved by the engineer. Sewn seams shall be made using polymeric thread with chemical resistance equal to or exceeding that of the geotextile. All sewn seams shall be continuous. Seams shall be oriented down slopes perpendicular to grading contours unless otherwise specified. For heat-seaming, fusion welding techniques recommended by the manufacturer shall be used.
- I. The contractor shall not use heavy equipment to traffic above the geotextile without approved protection.
- J. The geotextile shall be covered as soon as possible after installation and approval. Installed geotextile shall not be left exposed for more than 15 days.
- K. Material overlying the geotextile shall be carefully placed to avoid wrinkling or damage to the geotextile.



Single Sided Geocomposite

1.1 SCOPE

This specification covers the technical requirements for the manufacturing and installation of the geocomposite drainage layer. All materials meet or exceed the requirements of this specification, and all work will be performed in accordance with the procedures provided in these project specifications.

1.2 REFERENCES

A. American Society for Testing and Materials (ASTM)

1. ASTM D 1238 Standard Test Method for Melt Flow Rates of Thermoplastics
2. by Extrusion Plastometer
3. D 1505-98 Standard Test Method for Density of Plastics by the Density-Gradient Technique
4. ASTM D 4218, Standard Test Method for Determination of Carbon Black Content in Polyethylene Compounds by the Muffle Furnace Technique D 1603-94 Standard Test Method for Carbon Black in Olefin Plastics
5. D 4355-02 Standard Test Method for Deterioration of Geotextiles by Exposure to Light, Moisture and Heat in a Xenon Arc Type Apparatus
6. D 4491-99 Standard Test Method for Water Permeability of Geotextiles by Permittivity
7. D4533 Standard Test Method for Trapezoid Tearing Strength of Geotextiles
8. D 4716-00 Standard Test Method for Determining the (In-Plane) Flow Rate Per Unit Width and Hydraulic Transmissivity of a Geosynthetic Using a Constant Head
9. D 4751-99 Standard Test Method for Determining Apparent Opening Size of a Geotextile
10. D 6241 Standard Test Method for the Static Puncture Strength of Geotextiles and Geotextile- Related Products Using a 50-mm Probe D 4833-88 (1996) Standard Test Method for Index Puncture Resistance of Geotextiles, Geomembranes and Related Products
11. D 5261-92 (1996) Standard Test Method for Measuring the Mass Per Unit Area of Geotextiles
12. D7005-03 Determining The Bond Strength (Ply-Adhesion) of Geocomposites
13. D 7179 Standard Test Method for Determining Geonet Breaking Force



- B. Relevant publications from the Environmental Protection Agency (EPA):
 - 1. Daniel, D.E. and R.M. Koerner, (1993), Technical Guidance Document: Quality Assurance and Quality Control for Waste Containment Facilities, EPA/600/R-93/182.

1.3 DEFINITIONS

- A. Construction Quality Assurance Consultant (CONSULTANT) – The Party, independent from MANUFACTURER and INSTALLER, that is responsible for observing and documenting activities related to quality assurance during the lining system construction.
- B. ENGINEER - The individual or firm responsible for the design and preparation of the project's Contract Drawings and Specifications.
- C. Geocomposite Manufacturer (MANUFACTURER) - The party responsible for manufacturing the geocomposite rolls.
- D. Geosynthetic Quality Assurance Laboratory (TESTING LABORATORY) - The Party, independent from the MANUFACTURER and INSTALLER, responsible for conducting laboratory tests on samples of geosynthetics obtained at the site or during manufacturing, usually under the direction of the OWNER.
- E. INSTALLER- Party responsible for field handling, transporting, storing and deploying the geocomposite.
- F. Lot- A quantity of resin (usually the capacity of one rail car) used to manufacture polyethylene geocomposite rolls. The finished rolls will be identified by a roll number traceable to the resin lot.

1.4 QUALIFICATIONS

- A. MANUFACTURER
 - 1. Geocomposite shall be manufactured by the following:
 - a. GSE Lining Technology, Inc.
 - b. Approved Equal



2. MANUFACTURER shall have manufactured a minimum of 10,000,000-ft² of polyethylene geocomposite material during the last year.

B. INSTALLER

1. INSTALLER shall have installed a minimum of 500,000 square feet of geocomposite in the last 3 years.
2. INSTALLER shall have worked in a similar capacity on at least 5 projects similar in complexity to the project described in the contract documents, and within at least 50,000 square feet of geonet installation on each project.
3. The Installation Supervisor shall have worked in a similar capacity on projects similar in size and complexity to the project described in the Contract Documents.

1.5 MATERIAL LABELING, DELIVERY, STORAGE AND HANDLING

- A. Labeling- Each roll delivered to the site shall be wrapped and labeled by the MANUFACTURER. The label will identify:
 1. Manufacturer's name
 2. Product identification
 3. Length
 4. Width
 5. Roll number
- B. Delivery- Rolls will be prepared to ship by appropriate means to prevent damage to the material and to facilitate off-loading.
- C. Storage- The on-site storage location provided by the CONTRACTOR to protect the geonet from abrasions, excessive dirt and moisture, shall have the following characteristics:
 1. Level (no wooden pallets)
 2. Smooth
 3. Dry
 4. Protected from theft and vandalism
 5. Adjacent to the area being lined



D. Handling

1. The CONTRACTOR and INSTALLER shall handle all rolls in such a manner to ensure they are not damaged in any way.
2. The INSTALLER shall take any necessary precautions to prevent damage to underlying layers during placement of the drainage material.

1.6 WARRANTY

- A. Material shall be warranted, on a pro-rata basis against defects for a period of 1-year from the date of the geocomposite installation.
- B. Installation shall be warranted against defects in workmanship for a period of 1-year from the date of geocomposite completion.

2. PRODUCTS

2.1 GEOCOMPOSITE PROPERTIES

- A. A geocomposite shall be manufactured by extruding two crossing strands to form a bi-planar drainage net structure with a non-woven geotextile bonded to one or both sides.
- B. The geocomposite specified shall have properties that meet or exceed the values listed in the following data sheets below.



| TABLE 1: GEOCOMPOSITE PROPERTIES | | | |
|--|-------------------------------|---------------------------|-------------------------------|
| Property | Test Method | Frequency | Value |
| Geocomposite | | | |
| Transmissivity (1), gal/min/ft (m2/sec) Single-Sided Composite | ASTM D 4716 | 1/540,000-ft ² | 6.2 (1.3 x 10 ⁻³) |
| Ply Adhesion, lb/in | ASTM D 7005 | 1/50,000-ft ² | 0.5 |
| Geonet | | | |
| Geonet Core Thickness, mil (1) | ASTM D 5199 | 1/50,000-ft ² | 270 |
| Transmissivity (2), gal/min/ft (m2/sec) | ASTM D 4716 | 1/540,000-ft ² | 19 (4 x 10 ⁻³) |
| Compressive Strength, lbs/ft | ASTM D 6364 | 1/540,000-ft ² | 40,000 |
| Density, g/cm ³ | ASTM D 1505 | 1/50,000-ft ² | 0.94 |
| Tensile Strength (MD), lb/in | ASTM D 7179 | 1/50,000-ft ² | 100 |
| Carbon Black Content, % | ASTM D 4218 | 1/50,000-ft ² | 2.0 |
| 8 oz. Geotextile (prior to lamination) | | | |
| Mass per Unit Area, oz/yd ² | ASTM D 5261 | 1/90,000-ft ² | 8 |
| Grab Tensile Strength, lb | ASTM D 4632 | 1/90,000-ft ² | 220 |
| Grab Elongation | ASTM D 4632 | 1/90,000-ft ² | 50% |
| CBR Puncture Strength, lb | ASTM D 6241 | 1/540,000-ft ² | 575 |
| Trapezoidal Tear Strength, lb | ASTM D 4533 | 1/90,000-ft ² | 90 |
| AOS, US Sieve (mm) | ASTM D 4751 | 1/540,000-ft ² | 80 (0.180) |
| Permittivity, sec-1 | ASTM D 4491 | 1/540,000-ft ² | 1.3 |
| Water Flow Rate, gpm/ft ² | ASTM D 4491 | 1/540,000-ft ² | 95 |
| UV Resistance, % Retained | ASTM D 4355 (after 500 hours) | per formulation | 70 |

Note: The design engineer shall prepare the table above based on the GSE product data sheet and then delete this note



C. Resin

1. Resin shall be new first quality, compounded polyethylene resin.
2. Natural resin (without carbon black) shall meet the following additional minimum requirements:

| TABLE 2: RAW MATERIAL PROPERTIES | | |
|----------------------------------|----------------------------|-------|
| Property | Test Method ⁽¹⁾ | Value |
| Density (g/cm ³) | ASTM D 1505 | >0.94 |
| Melt Flow Index (g/10 min) | ASTM D 1238 | ≤ 1.0 |

¹GSE utilizes test equipment and procedures that enable effective and economical confirmation that the product will conform to specifications based on the noted procedures. Some test procedures have been modified for application to geosynthetics. All procedures and values are subject to change without prior notification.

2.2 MANUFACTURING QUALITY CONTROL

The geocomposite shall be manufactured in accordance with the Manufacturer's Quality Control Plan submitted to and approved by the ENGINEER.

The geocomposite shall be tested according to the test methods and frequencies listed on Table 1 which has been prepared based on product data sheets.

3. EXECUTION

3.1 FAMILIARIZATION

A. Inspection

1. Prior to implementing any of the work in the Section to be lined, the INSTALLER shall carefully inspect the installed work of all other Sections and verify that all Work is complete to the point where the installation of the Section may properly commence without adverse impact.
2. If the INSTALLER has any concerns regarding the installed work of other Sections, he shall notify the Project ENGINEER.



3.2 MATERIAL PLACEMENT

- A. The geocomposite roll should be installed in the direction of the slope and in the intended direction of flow unless otherwise specified by the ENGINEER.
- B. If the project contains long, steep slopes, special care should be taken so that only full length rolls are used at the top of the slope.
- C. In the presence of wind, all geocomposites shall be weighted down with sandbags or the equivalent. Such sandbags shall be used during placement and remain until replaced with cover material.
- D. If the project includes an anchor trench at the top of the slopes, the geocomposite shall be properly anchored to resist sliding. Anchor trench compacting equipment shall not come into direct contact with the geocomposite.
- E. In applying fill material, no equipment can drive directly across the geocomposite. The specified fill material shall be placed and spread utilizing vehicles with a low ground pressure.
- F. The cover soil shall be placed in the geocomposite in a manner that prevents damage to the geocomposite. Placement of the cover soil shall proceed immediately following the placement and inspection of the geocomposite.

3.3 SEAMS AND OVERLAPS

- A. Each component of the geocomposite will be secured or seamed to the like component at overlaps.
- B. Geonet Components
 - 1. Adjacent edges of the geonet along the length of the geocomposite roll shall be placed with the edges of each geonet butted against each other.
 - 2. The overlaps shall be joined by tying the geonet structure with cable ties. These ties shall be spaced every 5 feet along the roll length.
 - 3. Adjoining geocomposite rolls (end to end) across the roll width should be shingled down in the direction of the slope, with the geonet portion of the top overlapping the geonet portion of the bottom geocomposite a minimum of 12 inches across the roll width.



4. The geonet portion should be tied every 6 inches in the anchor trench or as specified by the ENGINEER.

3.4 REPAIR

- A. Prior to covering the deployed geocomposite, each roll shall be inspected for damage resulting from construction.
- B. Any rips, tears or damaged areas on the deployed geocomposite shall be removed and patched. The patch shall be secured to the original geonet by tying every 6 inches with the approved tying devices. If the area to be repaired is more than 50 percent of the width of the panel, the damaged area shall be cut out and the two portions of the geonet shall be cut out and the two portions of the geonet shall be joined in accordance with *Subsection 3.03*.



Appendix D

Operating and Maintenance Plan



OPERATION AND MAINTENANCE PROCEDURES

In this plan, underlined text represents the language of the Rule.

The operator will operate and maintain the lined earthen containment to contain liquids and solids (blow sand and minimal precipitates from the treated produced water) and maintain the integrity of the liner system in a manner that prevents contamination of fresh water and protects public health and the environment as described below. The purpose of the lined earthen containment is to facilitate recycling, reuse, and reclamation of produced water derived from nearby oil and gas wells. During periods when water for E&P operations is not needed, produced water will discharge to one of the injection wells in the operator's SWD system. The containment will not be used for the disposal of produced water or other oilfield waste.

The operation of the Recycling Containment is summarized below:

1. Via pipeline, produced water generated from nearby oil and gas wells is delivered to a treatment system located as indicated in the C-147.
2. After treatment, the produced water discharges into the containment.
3. When required, treated produced water is removed from the containment for E&P operations. At this time, treated produced water will be used for drilling beneath the fresh water zones (beneath surface casing), for well stimulation (e.g. hydraulic fracturing) and other E&P uses as approved by OCD.
4. Whenever the maximum fluid capacity of the containment is reached, treatment and discharge to the containment ceases (see Freeboard and Overtopping Plan, below).
5. The operator will keep accurate records and shall report monthly to the division the total volume of water received for recycling, with the amount of fresh water received listed separately, and the total volume of water leaving the facility for disposition by use on form C-148.
6. The operator will maintain accurate records that identify the sources and disposition of all recycled water that shall be made available for review by the division upon request.
7. The containment shall be deemed to have ceased operations if less than 20 % of the total fluid capacity is used every six months following the first withdrawal of produced water for use. The operator will report cessation of operations to the appropriate division district office. The appropriate division district office may grant an extension to this determination of cessation of operations not to exceed six months.



The operation of the lined earthen containment will follow the mandates listed below:

1. The operator will not discharge into or store any hazardous waste (as defined by 40 CFR 261 and NMAC 19.15.2.7.H.3) in the containments.
2. If the containment's primary liner is compromised above the fluid's surface, the operator will repair the damage or initiate replacement of the primary liner within 48 hours of discovery or seek an extension of time from the Division District office.
3. If the primary liner is compromised below the fluid's surface, the operator will remove all fluid above the damage or leak within 48 hours of discover, notify the division district office, and repair the damage or replace the primary liner.
4. If any penetration of the containment liner is confirmed by sampling of fluid in the leak detection system (see Inspection and Monitoring Plan), the operator will:
 - a. Begin and maintain fluid removal from the leak detection/pump-back system,
 - b. Notify the District office within 48 hours (phone or email) of the discovery,
 - c. Identify the location of the leak, and
 - d. Repair the damage or, if necessary, replace the containment liner.
5. The operator will install, or maintain onsite, an oil absorbent boom or other device to contain an unanticipated release and the operator will remove any visible layer of oil from the surface of the recycling containment.
6. The operator will report releases of fluid in a manner consistent with NMAC 19.15.29.
7. The containment will be operated to prevent the collection of surface water run-on.
8. The operator will maintain the containment free of miscellaneous solid waste or debris.
9. The operator will maintain at least 3-ft of freeboard for the containment and will use a free-standing staff gauge to allow easy determination of the required 3-ft of freeboard.
10. As described in the design/construction plan, the injection or withdrawal of fluids from the containment is accomplished through hardware that prevents damage to the liner by erosion, fluid jets, or impact from installation and removal of hoses or pipes.
11. The operator shall ensure that all gates associated with the fence are closed and locked when responsible personnel are not onsite.
12. The operator will maintain the fences in good repair.



MONITORING, INSPECTION, AND REPORTING PLAN

The operator will inspect the recycling containment and associated leak detection systems weekly while it contains fluids. The operator shall maintain a current log of such inspections and make the log available for review by the division upon request.

Weekly inspections consist of:

1. Reading and recording the fluid height of staff gauges,
2. Recording any evidence that the pond surface shows visible oil,
3. Visually inspecting the containment's exposed liners, and
4. Checking the leak detection system for any evidence of a loss of integrity of the primary liner.

As stated above, if a liner's integrity is compromised, or if any penetration of the liner occurs above the water surface, then the operator will notify the District office within 48 hours (phone or email).

Monthly, the operator will:

1. Inspect diversion ditches and berms around the containment to check for erosion and collection of surface water run-on.
2. Inspect the leak detection system for evidence of damage or malfunction and monitor for leakage.
3. Inspect the containment for dead migratory birds and other wildlife. Within 30 days of discovery, report the discovery of dead migratory birds or other wildlife to the appropriate wildlife agency and to the division district office in order to facilitate assessment and implementation of measures to prevent incidents from reoccurring.
4. Report to the division the total volume of water received for recycling, with the amount of fresh water received listed separately, and the total volume of water leaving the facility for disposition by use on form C-148.
5. Record sources and disposition of all recycled water.

The operator will maintain a log of all inspections and make the log available for the appropriate Division District office's review upon request. An example of the log is attached to this section of the permit application.



FREEBOARD AND OVERTOPPING PREVENTION PLAN

The method of operation of the containment allows for maintaining freeboard with very few potential problems. When the capacity of the containment is reached (3-ft of freeboard), the discharge of treated produced water ceases and the produced water generated by nearby oil and gas wells is managed by one of the injection wells as identified in *Appendix E*.

If rising water levels suggest that 3-ft of freeboard will not be maintained, the operator will implement one or more of the following options:

1. Cease discharging treated produced water to the containment.
2. Accelerate re-use of the treated produced water for purposes approved by the Division.
3. Transfer treated produced water from the containment to injection wells.

The reading of the staff gauge typically occurs daily when treatment operations are ongoing and weekly when discharge to the containment is not occurring.

PROTOCOL FOR LEAK DETECTION MONITORING, FLUID REMOVAL, AND REPORTING

As shown in *Appendix A*, the leak detection system includes a monitoring system. Any fluid released from the primary liner will flow to the collection sump, where fluid level monitoring is possible at the monitoring riser pipe associated with the leak detection system.

Staff may employ a portable electronic water level meter to determine if fluid exists in the monitoring riser pipe. Obtaining accurate readings of water levels in a sloped pipe beneath a containment can be a challenge. An electrician's wire snake may be required to push the probe to the bottom of the port and the probe may be fixed in a 2-in pipe "dry housing" to avoid false readings due to water condensation on the pipe. There are many techniques to determine the existence of water in the sumps, including low-flow pumps and a simple small bailer affixed to an electrician's snake. The operator will use the method that works best for this containment.



If seepage from the containment into the leak detection system is suspected by a positive fluid level measurement, the operator will:

1. Re-measure fluid levels in the monitoring riser pipe on a daily basis for one week to determine the rate of seepage.
2. Collect a water sample from the monitoring riser pipe to confirm the seepage is treated produced water from the containment via field conductivity and chloride measurements.
3. Notify NMOCD of a confirmed positive detection in the system within 48 hours of sampling (initial notification).
4. Install a pump into the monitoring riser pipe sump to continually (manually on a daily basis or via automatic timers) remove fluids from the leak detection system into the containment until the liner is repaired or replaced.
5. Dispatch a liner professional to inspect the portion of the containment suspected of leakage during a "low water" monitoring event.
6. Provide NMOCD a second report describing the inspection and/or repair within 20 days of the initial notification.

If the point of release is obvious from a low water inspection, the liner professional will repair the loss of integrity. If the point of release cannot be determined by the inspection, the liner professional will develop a more robust plan to identify the point(s) of release. The inspection plan and schedule will be submitted to OCD with the second report. The operator will implement the plan upon OCD approval.



Appendix E

Closure Plan



CLOSURE PLAN

In this plan, underlined text represents the language of the Rule.

After operations cease, the operator will remove all fluids within 60 days and close the containment within six months from the date the operator ceases operations from the containment for use.

The operator shall substantially restore the impacted surface area to

1. The condition that existed prior to the construction of the recycling containment or
2. To a condition imposed by federal, state trust land, or tribal agencies on lands managed by those agencies as these provisions govern the obligations of any operator subject to those provisions.

EXCAVATION AND REMOVAL CLOSURE PLAN - PROTOCOLS AND PROCEDURES

The workover pit is expected to contain a small volume of solids, the majority of which will be windblown sand and dust with some mineral precipitates from the water.

1. The operator will remove all liquids from the pits and either:
 - a. Dispose of the liquids in a division-approved facility, or
 - b. Recycle, reuse, or reclaim the water for reuse in drilling and stimulation
2. The operator will close the recycling containment by first removing all fluids, contents, and synthetic liners and transferring these materials to a Division approved facility.
3. After the removal of the pit contents and liners, soils beneath the workover pit will be tested by collection of a five-point (minimum) composite sample, which includes stained or wet soils, if any. That sample shall be analyzed for the constituents listed in Table 1 of 19.15.34.14.
4. After review of the laboratory results:
 - a. If any contaminant concentration is higher than the parameters listed in Table 1, additional delineation may be required, and the operator must receive approval before proceeding with closure.



- b. If all contaminant concentrations are less than or equal to the parameters listed in Table 1, then the operator will proceed to:
 - i. Backfill with non-waste containing, uncontaminated earthen material or
 - ii. Undertake an alternative closure process pursuant to a variance request after approval by OCD.
5. The operator will reclaim the containment's location to a safe and stable condition that blends with the surrounding undisturbed area.
6. Topsoils and subsoils shall be replaced to their original relative positions and contoured so as to achieve erosion control, long-term stability, and preservation of surface water flow patterns.
7. The disturbed area shall then be reseeded in the first favorable growing season following closure of a recycling containment.

CLOSURE DOCUMENTATION

Within 60 days of closure completion, the operator shall submit a closure report on Form C-147, including required attachments, to document all closure activities including sampling results and the details on any backfilling, capping or covering, where applicable. The closure report shall certify that all information in the report and attachments is correct and that the operator has complied with all applicable closure requirements and conditions specified in division rules or directives.

The operator shall notify the division when reclamation and re-vegetation are complete. Specifically, the notice will document that all ground surface disturbing activities at the site have been completed, and a uniform vegetative cover has been established that reflects a life-form ratio of plus or minus fifty percent (50%) of pre-disturbance levels and a total percent plant cover of at least seventy percent (70%) of pre-disturbance levels, excluding noxious weeds.



Appendix F

Waters of the US Delineation Report



Waters of the U.S. Delineation Report

3Bear Produced Water Recycling Facility—Eddy County, New Mexico

EnviroTech Engineering and Consulting, Inc. and
3Bear Energy LLC

July 2018



COX | McLAIN
Environmental Consulting

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1.0 Introduction and Purpose

This waters of the U.S. delineation report evaluates the potential for impacts to waters of the U.S., including wetlands, which may result from the construction of a proposed produced water recycling facility. The proposed facility includes two (2) 500,000 barrels per day storage pits. Produced water will flow to the pits from oil/gas producing wells. Both pits will be lined with a 60-mil primary and 40-mil secondary high-density polyethylene liner with a leachate collection system installed between the liners. The study area for the proposed project is located on approximately 60 acres of land in Eddy County, New Mexico (**Figure 1** in **Attachment A**). The Owner/Operator, 3Bear Energy LLC, proposes to utilize this approximately 60-acre study area to construct and operate the proposed produced water recycling facility.

The purpose of this report is to identify, delineate, and describe potentially jurisdictional waters of the U.S., including wetlands, located within the study area to assist in avoidance of impacts and to determine whether United States Army Corps of Engineers (USACE) permit authorization would be required. Conclusions contained in this report are the opinions of the professionals who conducted the study and are subject to confirmation by the USACE-Albuquerque District.

The USACE regulates the discharge of dredged and fill material into wetlands and other waters of the U.S. under Section 404, subsection 330.5(a)(21) of the Clean Water Act. Section 10 of the Rivers and Harbors Act of 1899 authorizes the USACE to regulate any work in or affecting navigable waters of the U.S. Authorization is required from the USACE for any activity that would result in the discharge of dredged or fill material into waters of the U.S. Regulated activities may be permitted through the USACE via Individual Permits, Regional General Permits, Nationwide Permits, or Letters of Permission.

Project Information

| | | |
|-------------------------|--|--|
| Study Area: | An approximately 60 acre proposed produced water recycling facility in Eddy County, New Mexico (Figure 1) | |
| Size: | Approximately 60 acres | |
| County: | Eddy County, Texas | |
| USGS 7.5' Quads: | <i>Kitchen Cove, New Mexico</i> (Figure 2) | |
| Client: | 3Bear Energy LLC | |
| Client Address: | 1512 Larimer Street, Suite 540 Denver, CO 80202 | |
| Client Contact: | Kevin Burns Phone: 432-386-2973 Email: kburns@3bearllc.com | |

2.0 Methods

2.1 Data Review

Qualified wetland ecologists reviewed a number of published data resources prior to the field investigations in order to identify potentially jurisdictional waters of the U.S. which may be located within the study area. Reviewed sources include U.S. Fish and Wildlife Service National Wetland Inventory (NWI) maps, the National Hydrography Dataset (NHD), the Natural Resources Conservation Service (NRCS) Soil Survey for Eddy County, a U.S. Geological Survey (USGS) 7.5-minute quadrangle sheet (*Kitchen Cove, New Mexico*), Geologic Map of New Mexico maps (*Otis 7.5-minute quadrangle sheet*), Federal Emergency Management Agency (FEMA) floodplain maps (FEMA, 2010), and recent aerial photography.

2.2 Field Delineation

Qualified wetland ecologists conducted field investigations within the study area in July 2018. The routine method of wetland delineation outlined in the *Field Guide for Wetland Delineation: 1987 Corps of Engineers Manual* (Wetland Training Institute, 1991) as modified by the *Interim Regional Supplement to the USACE Wetland Delineation Manual: Arid West Region, Version 2.0* (USACE, 2008) was utilized for wetland determinations within the study area. Field activities focused on wetland and waters of the U.S. delineation and description.

The 1987 USACE wetland delineation manual defines wetlands based on three criteria: hydrophytic vegetation, hydric soils, and wetland hydrology. In general, all three criteria must be present for an area to qualify as a wetland. Some exceptions can occur in disturbed areas or in newly formed wetlands, where one indicator (such as hydric soils) might be lacking. These areas would be addressed on an individual basis as outlined in the *Field Guide for Wetland Delineation*.

In addition to the jurisdictional wetlands defined above, the Clean Water Act regulates impacts to other waters of the U.S. The term “waters of the United States” has broad meaning and incorporates both deepwater aquatic habitats and special aquatic sites, including wetlands, as listed below:

1. The territorial seas with respect to the discharge of fill material
2. Coastal and inland waters, lakes, rivers, and streams that are navigable waters of the U.S., including their adjacent wetlands
3. Tributaries to navigable waters of the U.S., including adjacent wetlands
4. Interstate waters and their tributaries, including adjacent wetlands

On August 28, 2015, the EPA finalized the Clean Water Rule: Definition of “Waters of the United States” (EPA, 2015a). However, on October 9, 2015, the U.S. Court of Appeals for the Sixth Circuit issued a stay of the rule (EPA, 2015b).

For linear waters of the U.S., the Ordinary High Water Mark (OHWM) was determined by assessing a combination of factors at each site. In accordance with Sec. 328.3(e) of the Clean Water Act and Regulatory Guidance Letter 05-05 (USACE, December 7, 2005), the following factors were considered in determining the jurisdictional boundary:

- Natural line impressed on the bank
- Shelving
- Changes in the character of soil
- Destruction of terrestrial vegetation
- Presence of litter and debris
- Wracking
- Vegetation matted down, bent, or absent
- Sediment sorting
- Leaf litter disturbed or washed away
- Scour
- Deposition
- Multiple observed flow events
- Bed and banks
- Water staining
- Change in plant community
- Other appropriate means that consider the characteristics of the surrounding areas

Following the completion of preliminary data gathering and synthesis, the routine method of wetland determination was used to identify potentially jurisdictional areas within the study area. Potential wetland sites were evaluated in the field while localized hydrologic characteristics and the dominant vegetative species observed at the study area were described. Photographs of the evaluated aquatic features are found in **Attachment B** of this report. Boundaries of likely waters of the U.S., including wetlands, were recorded using a handheld Trimble GeoXT Global Positioning System (GPS) unit and confirmed using aerial photography; these are shown in **Figure 6**. GPS data was post-processed using Trimble Pathfinder Office software to achieve sub-meter accuracy.

3.0 Results

3.1 General Description of the Study Area

Vicinity and Study Area Description

The study area is located in a largely undeveloped rural area with surrounding land use consisting exclusively of oil and gas production facilities. United States Highway (US) 62 (National Parks Highway) and County Road (CR) 408 (Dark Canyon Road) are located to the west of the study area.

Geology

The study area is located on a single geologic formation, 'Piedmont Alluvial Deposits' (**Figure 3**; U.S. Geological Survey, 1992). The 'Piedmont Alluvial Deposits' formation includes deposits of higher gradient tributaries bordering major stream valleys, alluvial veneers of the piedmont slope, and alluvial.

Information regarding soils within the study area was obtained from the U.S. Department of Agriculture NRCS Soil Survey for Eddy County (NRCS, 2017). Two soil map units are found within the study area. Information on soils in the study area is presented in **Table 1**, and the soils are shown in **Figure 4**.

Table 1: Soils within the Study Area

| Map Unit Code | Map Unit | Hydric? (Yes/No) |
|---------------|---|------------------|
| RE | Reagan-Upton association, 0 to 9 percent slopes | No |
| UG | Upton gravelly loam, 0 to 9 percent slopes | No |

Source: NRCS, 2016.

Hydrology

According to the NWI and NHD maps (**Figure 5**), one aquatic feature (a PEM1Jx = palustrine, emergent, persistent, intermittently flooded, excavated, freshwater pond) is mapped within the study area. The study area has some natural topographical variation; and according to the USGS topographic map rises to approximately 3,268 feet above mean sea level (MSL) near the northwest corner of the study area and falls to 3,252 feet above MSL near the southeastern corner of the study area (**Figure 2**). The study area generally drains to the southeast and is located within the Pecos River watershed.

Vegetation

The study area is characteristic of the Interior Desert Subregion of the Arid West Region (USACE 2008). The study area is sporadically covered with creosote bush (*Larrea tridentata*) approximately 12-36 inches tall. Grasses and forbs were dominant throughout the majority of

the study area which entirely lacks a canopy of trees and a woody vine strata. Herbaceous species include cane bluestem (*Bothriochloa barbinodis*), slim tridens (*Tridens muticus*), hairy woollygrass (*Erioneuron pilosum*), bristlecup sandmat (*Chamaesyce chaetocalyx*), narrowleaf globemallow (*Sphaeralcea angustifolia*), woolly paperflower (*Psilostrophe tagentina*), bush croton (*Croton fruticosus*), white ratany (*Krameria grayi*), Dakota mock vervain (*Glandularia bipinnatifida*), and broom snakeweed (*Gutierrezia sarothrae*).

3.2 Descriptions of Evaluated Aquatic Features

The entire study area was traversed and aquatic features were examined in accordance with the *Field Guide for Wetland Delineation: 1987 Corps of Engineers Manual* (Wetland Training Institute, 1991). Five wetland determination data forms were completed along five pedestrian transects (**Figure 6**). One aquatic feature, a likely non-jurisdictional stock tank, was identified within the study area (**Attachment C, Table 2, and Figures 6**).

Table 2: Summary of Waters of the U.S. within the Study Area

| | Name of Water Body | Aquatic Feature Class | Average OHWM width (feet) | Water of the U.S.? (Yes/No) | Linear Feet/Acres of Water Body within the Study Area |
|--|--------------------|-----------------------|---------------------------|-----------------------------|---|
| | OW1 | Stock Tank | N/A | No | N/A 0.21 acres |

Stock Tank (OW1)

OW1 (open water 1) is best described as a likely non-jurisdictional upland stock tank (**Figure 6**). Although OW1 is depicted on USGS topographic maps and on NWI maps, it does not lie within the FEMA-designated 100-year floodplain. Approximately 0.21 acre of OW1 is located within the study area. No standing water, saturation, or presence of a high-water table was observed within OW1 during field investigations. Soil underlying OW1 is mapped as Reagan-Upton association, 0 to 9 percent slopes, and is not listed as hydric. Transect 4 (Wetland Determination Data Form 4) and Transect 5 (Wetland Determination Data Form 5) were completed within OW1. The bottom of OW1 was vegetated with herbaceous and shrubby species including creosote bush, broom snakeweed, bush croton, woolly paperflower, Dakota mock vervain, and slim tridens. OW1 has closed contours and does not appear to have a downstream hydrologic connection with any other identified aquatic features. This upland stock tank was likely excavated in uplands to provide supplemental water for agricultural operations. It appears that under current USACE guidance, OW1 would likely not be considered a water of the U.S. because it lacks a clear downstream hydrologic connection to any other identified aquatic feature. See **Figure 6, Wetland Determination Data Forms 4-5 and Photos 16, 21, and 26**.

3.3 USACE Permitting

The Owner/Operator proposes to use the approximate 60-acre study area for the proposed produced water recycling facility. In total, one likely non-jurisdictional stock tank (OW1) is located within the limits of the study area. It is our opinion that this aquatic feature would not be subject to USACE jurisdiction and placement of dredged or fill material into this aquatic feature would not require USACE authorization. It is also our opinion that no portion of the study area falls under USACE jurisdiction and that construction of the proposed project would not require Department of the Army Permit Authorization. Although not required, the Owner/Operator has the ability to request concurrence of our findings from the Albuquerque District - USACE.

4.0 Conclusions

A delineation of waters of the U.S., including wetlands, was conducted for the approximately 60-acre study area in July 2018. This delineation effort resulted in the identification of one aquatic feature, a likely non-jurisdictional stock tank (OW1) within the limits of the study area. It is our opinion that this aquatic feature would not be subject to USACE jurisdiction, and placement of dredged or fill material into this aquatic feature would not require USACE authorization. It is also our opinion that no portion of the study area falls under USACE jurisdiction and that construction of the proposed project would not require Department of the Army Permit Authorization. Although not required, the Owner/Operator has the ability to request concurrence of our findings from the Albuquerque District - USACE.

This report was prepared by:



Garrett P. Weiberg, Ecologist/GIS Specialist
Cox|McLain Environmental Consulting, Inc.

July 23, 2018

Date



Ryan Blankenship, AWB
Cox|McLain Environmental Consulting, Inc.

July 23, 2018

Date

5.0 References

- Environmental Protection Agency. 2015a. Clean Water Rule: Definition of “Waters of the United States”; Final Rule. Federal Register, Volume 80, Number 124. June 29, 2015.
- Environmental Protection Agency. 2015b. Clean Water Rule Litigation Statement Website; <https://www.epa.gov/cleanwaterrule/clean-waterrule-litigation-statement>. Accessed March 5, 2018.
- Federal Emergency Management Agency. 2010. Federal Insurance Rate Maps (FIRMs)—Map # 350120. Accessed July 16, 2018.
- New Mexico Bureau of Geology and Mineral Resources. 2004. Geologic Map of New Mexico—Geologic Map of the Otis 7.5-minute Quadrangle, Eddy County, New Mexico. <https://geoinfo.nmt.edu/publications/maps/geologic/ofgm/details.cfm?Volume=76>. Accessed July 16, 2018.
- U.S. Army Corps of Engineers (USACE). 2008. *Regional Supplement to the Corps of Engineers Wetland Delineation Manual: Aris West Region (Version 2.0)*,. ERDC/EL TR-08-28. Vicksburg, MS: U.S. Army Engineer Research and Development Center.
- U.S. Department of Agriculture, Natural Resources Conservation Service, Soil Survey Staff. 2017. Web Soil Survey, <http://websoilsurvey.nrcs.usda.gov/>. Accessed July 11, 2018.
- U. S. Department of Agriculture, Natural Resources Conservation Service. 2016. Hydric Soils List, <http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/use/hydric/>. Accessed June 18, 2018.
- U.S. Geological Survey, 1992, The Digital Geologic Map of New Mexico in ARC/INFO Format: U.S. Geological Survey Open-File Report 97-0052, 9 p. Accessed July 23, 2018.
- Wetland Training Institute, Inc. 1991. *Field Guide for Wetland Delineation: 1987 Corps of Engineers Manual*. WTI 91-2.

This report was written on behalf of EnviroTech Engineering and Consulting, Inc.
and 3Bear Energy LLC by
Cox|McLain Environmental Consulting, Inc.



COX | McLAIN
Environmental Consulting

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Attachment A

Figures

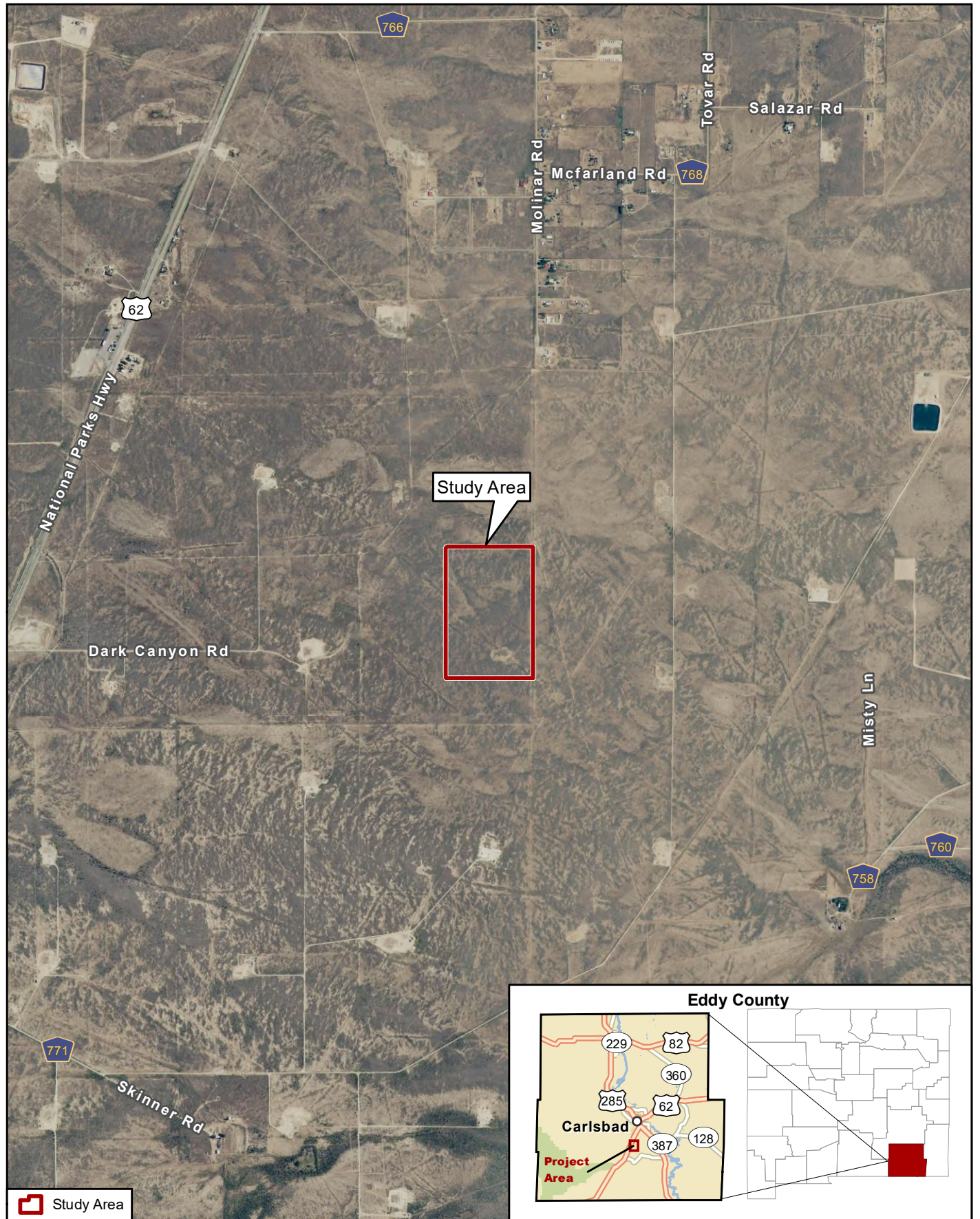
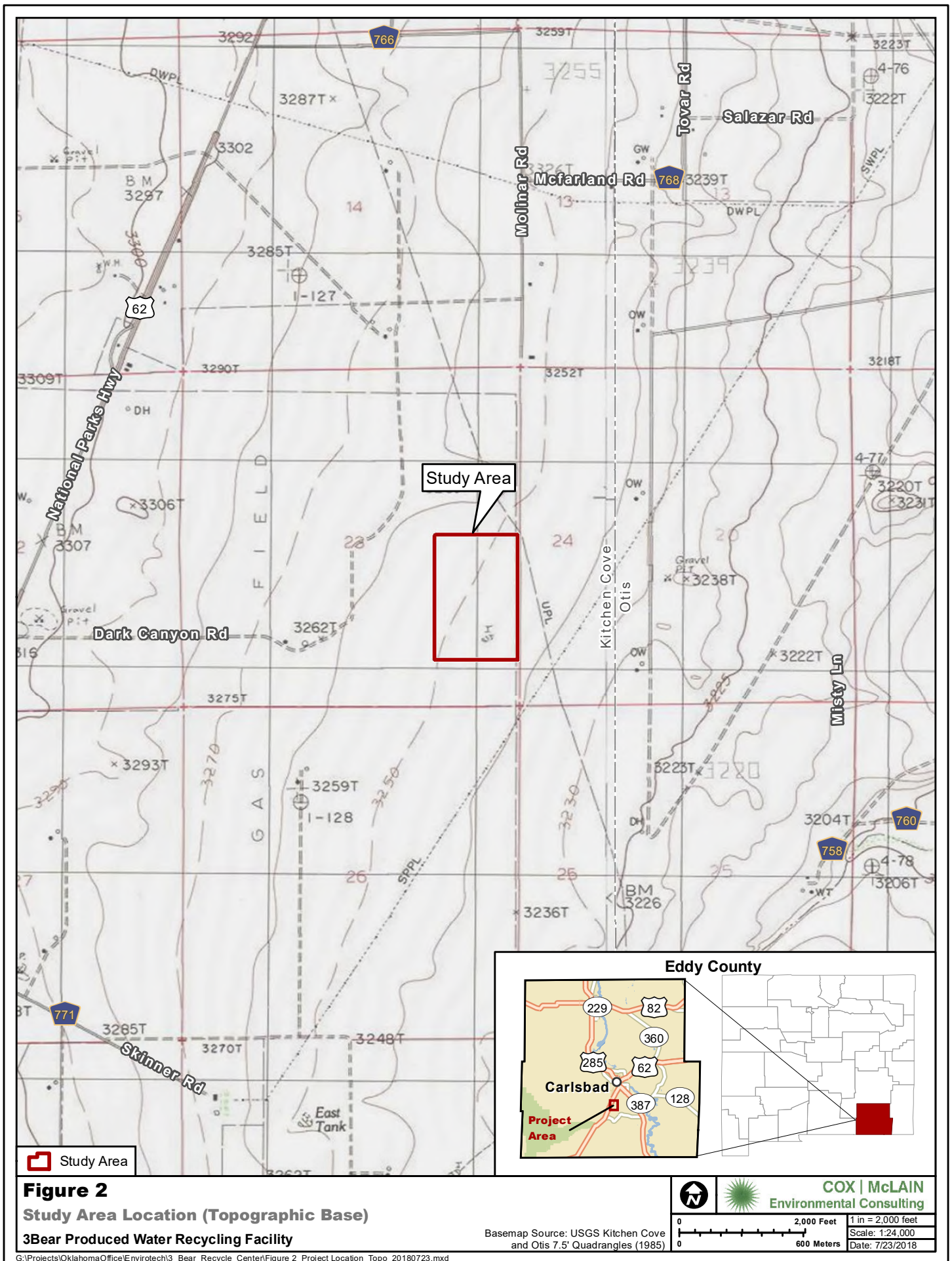


Figure 1
Study Area Location (Aerial Base)
3Bear Produced Water Recycling Facility

COX | McLAIN
 Environmental Consulting

0 2,000 Feet 1 in = 2,000 feet
 0 600 Meters Scale: 1:24,000
 Date: 7/23/2018

Aerial Source: NAIP (2016)



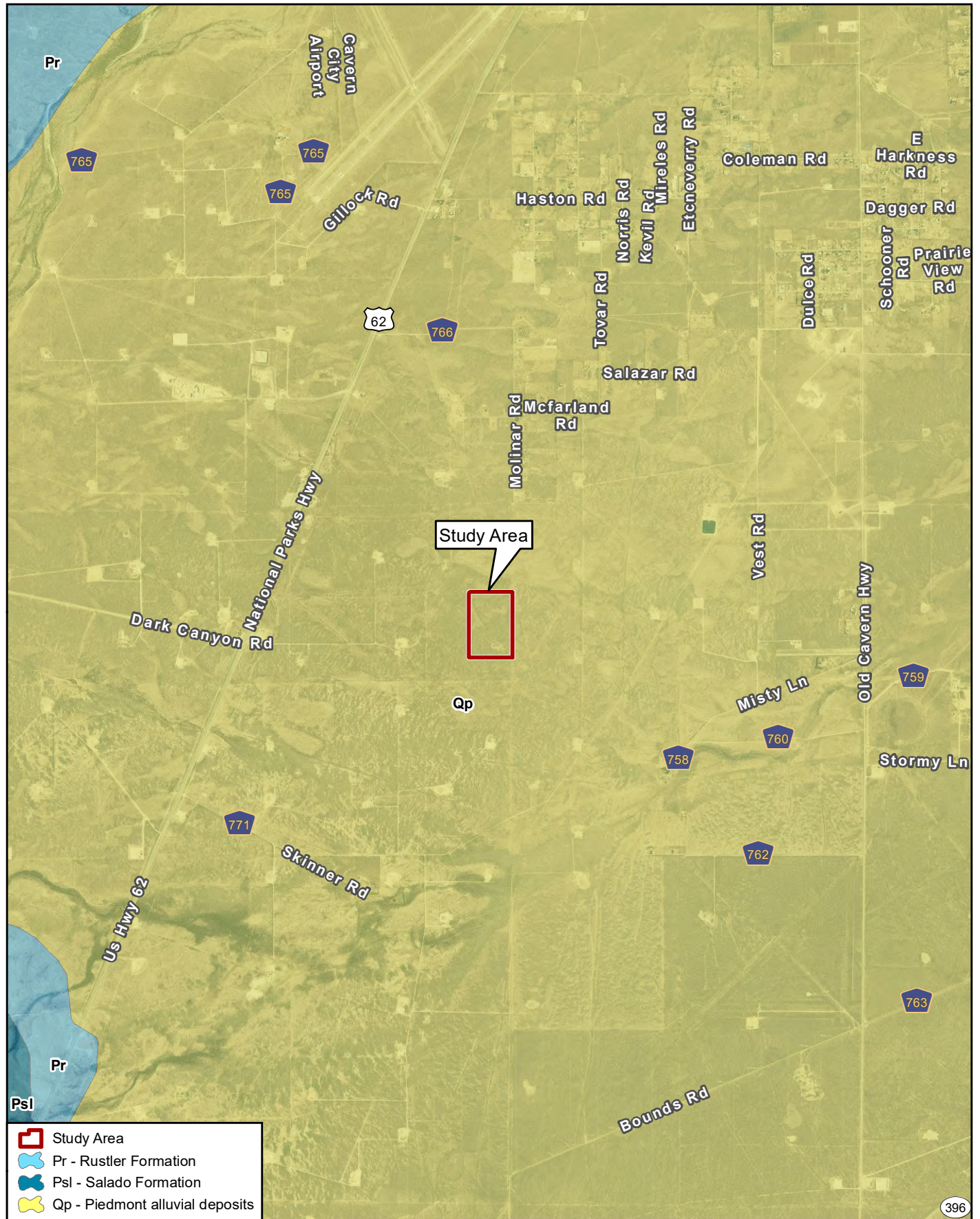


Figure 3

Study Area Geology

3Bear Produced Water Recycling Facility



COX | McLAIN
Environmental Consulting

0 4,000 Feet
0 1 Kilometers

1 in = 4,000 feet
Scale: 1:48,000
Date: 7/23/2018

Data Source: USGS (1992)
Aerial Source: NAIP (2016)

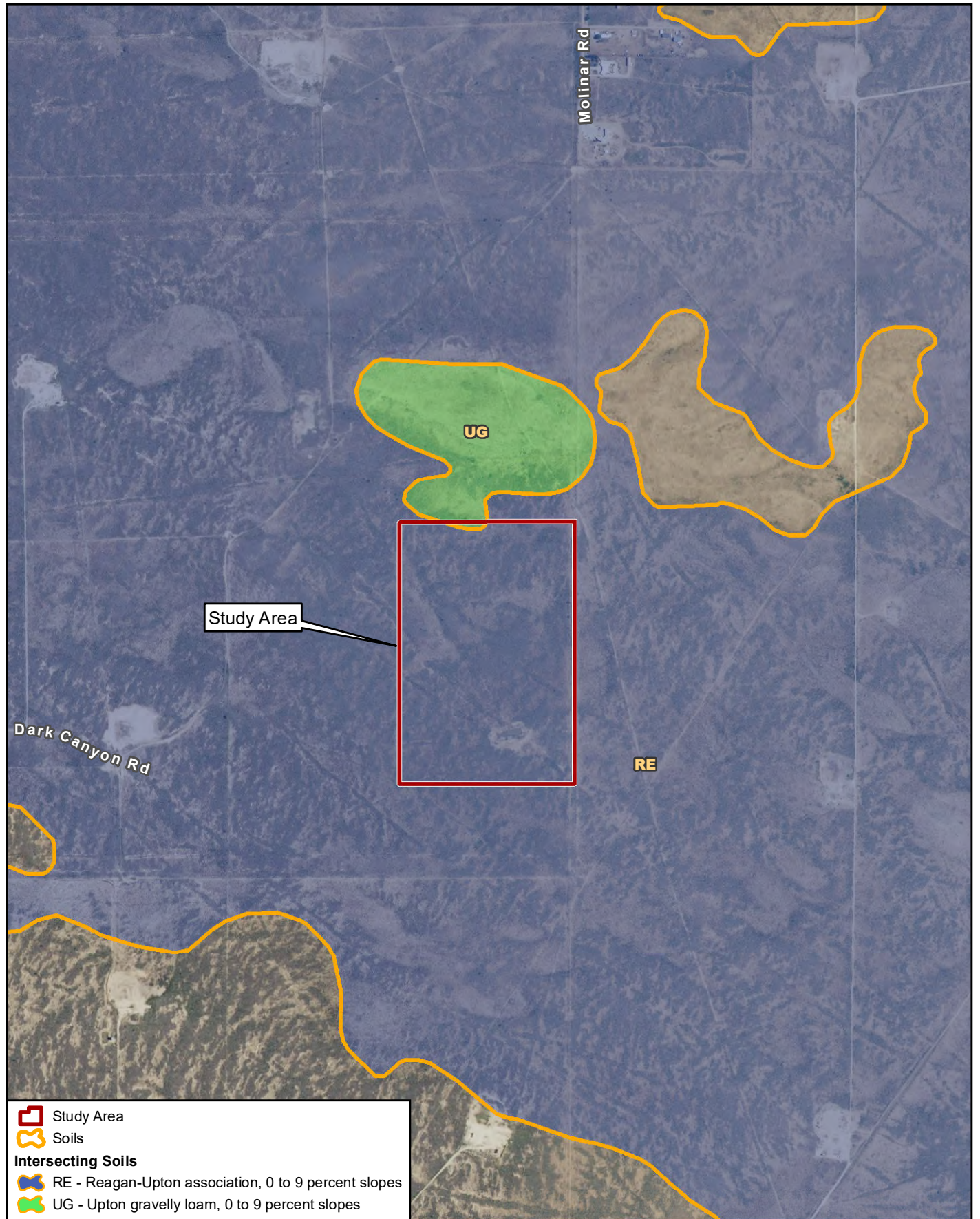


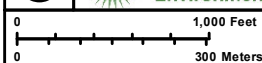
Figure 4

Study Area Soils

3Bear Produced Water Recycling Facility



COX | McLAIN
Environmental Consulting



1 in = 1,000 feet
Scale: 1:12,000
Date: 7/23/2018

Data Source: NRCS (2017)
Aerial Source: NAIP (2016)



Figure 5

Study Area Water Resources

3Bear Produced Water Recycling Facility

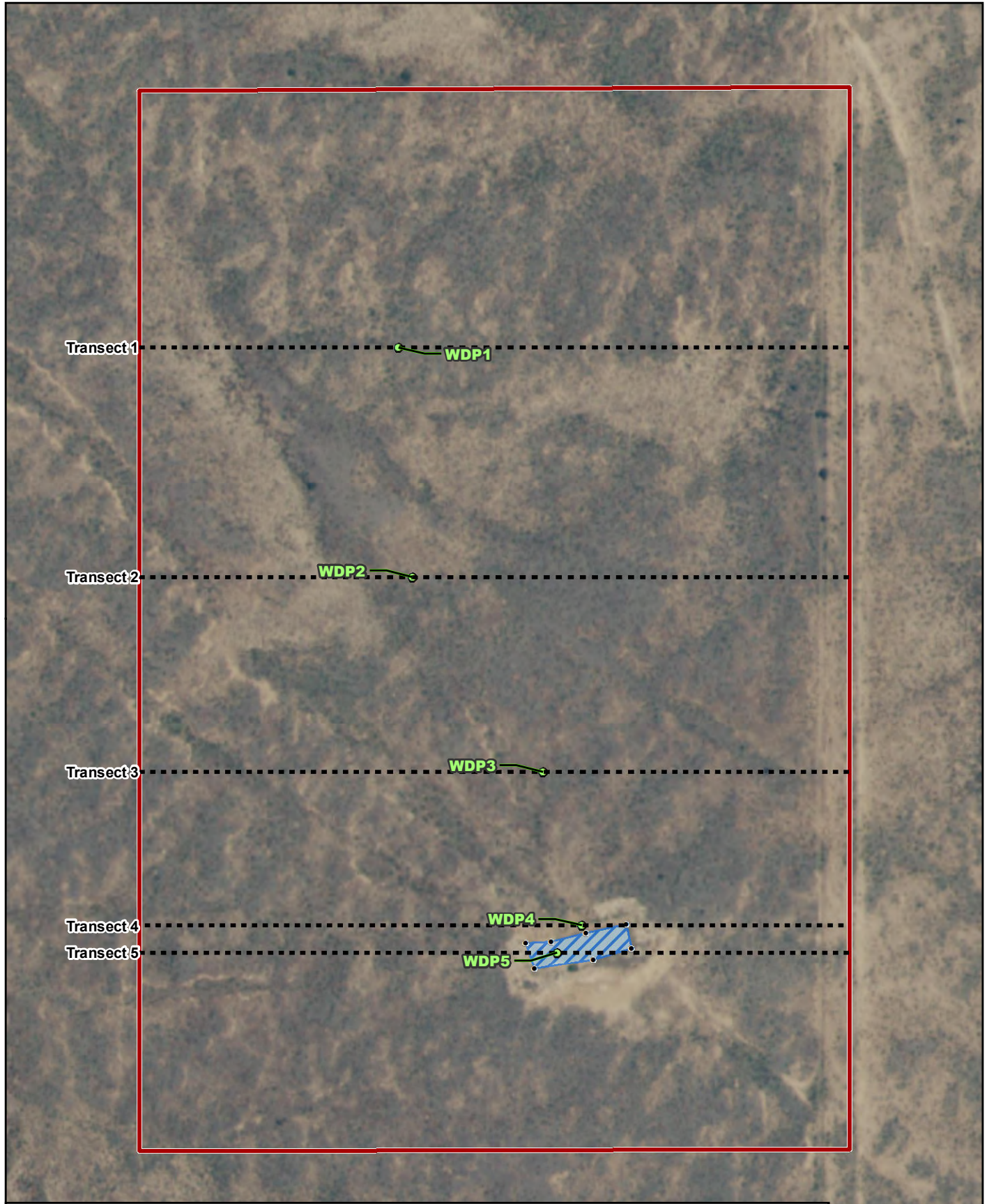
Data Sources: NHD (2018), NWI (2018),
FEMA NFHL (2018), CMEC (2018)
Aerial Source: NAIP (2016)



COX | McLAIN
Environmental Consulting

0 1,000 Feet
0 300 Meters

1 in = 1,000 feet
Scale: 1:12,000
Date: 7/23/2018



Study Area
 Transects
 Delineated Stock Tank (OW1)
 • GPS Point
 • Wetland Determination Point

Figure 6
Potential Waters of the U.S.
3Bear Produced Water Recycling Facility

Data Source: CMEC (2018)
 Aerial Source: NAIP (2016)

| | | |
|------|---|--|
| | COX McLAIN Environmental Consulting | |
| | 0 250 Feet 0 70 Meters | 1 in = 250 feet Scale: 1:3,000 Date: 7/23/2018 |

Attachment B

Study Area Photographs



PHOTO 1: WETLAND DETERMINATION DATA POINT #1 (UPLAND). LOCATED ON TRANSECT 1 IN THE NORTHERN EXTENT OF THE STUDY AREA.



PHOTO 2: THE STUDY AREA AT WETLAND DETERMINATION DATA POINT #1 ON TRANSECT 1. VIEWING NORTH.



PHOTO 3: THE STUDY AREA AT WETLAND DETERMINATION DATA POINT #1 ON TRANSECT 1. VIEWING EAST.



PHOTO 4: THE STUDY AREA AT WETLAND DETERMINATION DATA POINT #1 ON TRANSECT 1. VIEWING SOUTH.



PHOTO 5: THE STUDY AREA AT WETLAND DETERMINATION DATA POINT #1 ON TRANSECT 1. VIEWING WEST.



PHOTO 6: WETLAND DETERMINATION DATA POINT #2 (UPLAND). LOCATED ON TRANSECT 2 IN THE NORTHERN EXTENT OF THE STUDY AREA.



PHOTO 7: THE STUDY AREA AT WETLAND DETERMINATION DATA POINT #2 ON TRANSECT 2. VIEWING NORTH.



PHOTO 8: THE STUDY AREA AT WETLAND DETERMINATION DATA POINT #2 ON TRANSECT 2. VIEWING EAST.



PHOTO 9: THE STUDY AREA AT WETLAND DETERMINATION DATA POINT #2 ON TRANSECT 2. VIEWING SOUTH.



PHOTO 10: THE STUDY AREA AT WETLAND DETERMINATION DATA POINT #2 ON TRANSECT 2. VIEWING WEST.



PHOTO 11: WETLAND DETERMINATION DATA POINT #3 (UPLAND). LOCATED ON TRANSECT 3 IN THE CENTRAL PORTION OF THE STUDY AREA.



PHOTO 12: THE STUDY AREA AT WETLAND DETERMINATION DATA POINT #3 ON TRANSECT 3. VIEWING NORTH.



PHOTO 13: THE STUDY AREA AT WETLAND DETERMINATION DATA POINT #3 ON TRANSECT 3. VIEWING EAST.



PHOTO 14: THE STUDY AREA AT WETLAND DETERMINATION DATA POINT #3 ON TRANSECT 3. VIEWING SOUTH.



PHOTO 15: THE STUDY AREA AT WETLAND DETERMINATION DATA POINT #3 ON TRANSECT 3. VIEWING WEST.



PHOTO 16: WETLAND DETERMINATION DATA POINT #4 (UPLAND). LOCATED ON TRANSECT 4 IN THE SOUTHERN EXTENT OF THE STUDY AREA, ADJACENT TO A LIKELY NON-JURISDICTIONAL STOCK TANK (OW1).



PHOTO 17: THE STUDY AREA AT WETLAND DETERMINATION DATA POINT #4 ON TRANSECT 4. VIEWING NORTH.



PHOTO 18: THE STUDY AREA AT WETLAND DETERMINATION DATA POINT #4 ON TRANSECT 4. VIEWING EAST.



PHOTO 19: THE STUDY AREA AT WETLAND DETERMINATION DATA POINT #4 ON TRANSECT 4. VIEWING SOUTH.



PHOTO 20: THE STUDY AREA AT WETLAND DETERMINATION DATA POINT #4 ON TRANSECT 4. VIEWING WEST.



PHOTO 21: WETLAND DETERMINATION DATA POINT #5 (UPLAND). LOCATED ON TRANSECT 5 IN THE SOUTHERN EXTENT OF THE STUDY AREA WITHIN A LIKELY NON-JURISDICTIONAL STOCK TANK (OW1).



PHOTO 22: THE STUDY AREA AT WETLAND DETERMINATION DATA POINT #5 ON TRANSECT 5. VIEWING NORTH.



PHOTO 23: THE STUDY AREA AT WETLAND DETERMINATION DATA POINT #5 ON TRANSECT 5. VIEWING EAST.



PHOTO 24: THE STUDY AREA AT WETLAND DETERMINATION DATA POINT #5 ON TRANSECT 5. VIEWING SOUTH.



PHOTO 25: THE STUDY AREA AT WETLAND DETERMINATION DATA POINT #5 ON TRANSECT 5. VIEWING WEST.



PHOTO 26. A LIKELY NON-JURISDICTIONAL STOCK TANK (OW1) IN THE SOUTHERN EXTENT OF THE STUDY AREA. VIEWING WEST.



PHOTO 27. TYPICAL GRASSLAND VEGETATION FOUND WITHIN A LIKELY NON-JURISDICTIONAL STOCK TANK (OW1).



PHOTO 28. TYPICAL SHRUB/SCRUB VEGETATION FOUND THROUGHOUT THE STUDY AREA.

Attachment C

Wetland Determination Data Forms

WETLAND DETERMINATION DATA FORM – Arid West Region

Project/Site: 3 Bear Recycling Center City/County: Eddy County Sampling Date: 07/11/2018
 Applicant/Owner: 3Bear Energy LLC State: NM Sampling Point: WDP 1
 Investigator(s): Ryan Blankenship and Garrett Weiberg Section, Township, Range: Section 23, Township 23S, 26E
 Landform (hillslope, terrace, etc.): vegetated flat Local relief (concave, convex, none): none Slope (%): 0-1
 Subregion (LRR): D - West range and irrigated Lat: 32.288822 Long: -104.257721 Datum: NAD83
 Soil Map Unit Name: Reagan-Upton association, 0 to 9 percent slopes NWI classification: upland

Are climatic / hydrologic conditions on the site typical for this time of year? Yes ☒ No ☐ (If no, explain in Remarks.)
 Are Vegetation ☐, Soil ☐, or Hydrology ☐ significantly disturbed? Are "Normal Circumstances" present? Yes ☒ No ☐
 Are Vegetation ☐, Soil ☐, or Hydrology ☐ naturally problematic? (If needed, explain any answers in Remarks.)

SUMMARY OF FINDINGS – Attach site map showing sampling point locations, transects, important features, etc.

| | |
|--|--|
| Hydrophytic Vegetation Present? Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> | Is the Sampled Area within a Wetland? Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> |
| Hydric Soil Present? Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> | |
| Wetland Hydrology Present? Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> | |
| Remarks: None of the three necessary wetland indicators are present. The WDP is not located within a wetland. | |

VEGETATION – Use scientific names of plants.

| Tree Stratum (Plot size: <u>30'</u>) | Absolute % Cover | Dominant Species? | Indicator Status | Dominance Test worksheet: Number of Dominant Species That Are OBL, FACW, or FAC: <u>0</u> (A) Total Number of Dominant Species Across All Strata: <u>2</u> (B) Percent of Dominant Species That Are OBL, FACW, or FAC: <u>0</u> (A/B) | |
|--|------------------|-------------------|------------------|---|--|
| 1. <u>None</u> | | | | | |
| 2. _____ | | | | | |
| 3. _____ | | | | | |
| 4. _____ | | | | | |
| <u>0</u> = Total Cover | | | | | |
| Sapling/Shrub Stratum (Plot size: <u>15'</u>) | | | | Prevalence Index worksheet: Total % Cover of: _____ Multiply by: _____ OBL species _____ x 1 = _____ FACW species _____ x 2 = _____ FAC species _____ x 3 = _____ FACU species _____ x 4 = _____ UPL species _____ x 5 = _____ Column Totals: _____ (A) _____ (B) Prevalence Index = B/A = _____ | |
| 1. <u>Larrea tridentata</u> | <u>20</u> | <u>Y</u> | <u>UPL</u> | | |
| 2. _____ | | | | | |
| 3. _____ | | | | | |
| 4. _____ | | | | | |
| <u>20</u> = Total Cover | | | | | |
| Herb Stratum (Plot size: <u>5'</u>) | | | | Hydrophytic Vegetation Indicators: ___ Dominance Test is >50% ___ Prevalence Index is ≤3.0 ¹ ___ Morphological Adaptations ¹ (Provide supporting data in Remarks or on a separate sheet) ___ Problematic Hydrophytic Vegetation ¹ (Explain) | |
| 1. <u>Bothriochloa barbinodis</u> | <u>40</u> | <u>Y</u> | <u>UPL</u> | | |
| 2. <u>Gutierrezia sarothrae</u> | <u>10</u> | <u>N</u> | <u>UPL</u> | | |
| 3. <u>Croton fruticosus</u> | <u>10</u> | <u>N</u> | <u>UPL</u> | | |
| 4. <u>Krameria grayi</u> | <u>5</u> | <u>N</u> | <u>UPL</u> | | |
| <u>65</u> = Total Cover | | | | | |
| Woody Vine Stratum (Plot size: <u>30'</u>) | | | | Hydrophytic Vegetation Present? Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> | |
| 1. <u>None</u> | | | | | |
| 2. _____ | | | | | |
| <u>0</u> = Total Cover | | | | | |
| % Bare Ground in Herb Stratum <u>40</u> % Cover of Biotic Crust <u>0</u> | | | | | |

¹Indicators of hydric soil and wetland hydrology must be present, unless disturbed or problematic.

Hydrophytic Vegetation Present?

Yes ☐ No ☒

Remarks:

The vegetative community did not pass the dominance test.

SOILSampling Point: WDP 1**Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)**

| Depth (inches) | Matrix | | Redox Features | | | | Texture | Remarks |
|-------------------|---------------|-----|----------------|---|-------------------|------------------|------------|---------|
| | Color (moist) | % | Color (moist) | % | Type ¹ | Loc ² | | |
| 0-18 | 5YR 5/4 | 100 | none | | | | silty clay | |
| | | | | | | | | |
| | | | | | | | | |
| | | | | | | | | |
| | | | | | | | | |
| | | | | | | | | |
| | | | | | | | | |
| | | | | | | | | |
| | | | | | | | | |

¹Type: C=Concentration, D=Depletion, RM=Reduced Matrix, CS=Covered or Coated Sand Grains.²Location: PL=Pore Lining, M=Matrix.**Hydric Soil Indicators: (Applicable to all LRRs, unless otherwise noted.)**

☐ Histosol (A1)
☐ Histic Epipedon (A2)
☐ Black Histic (A3)
☐ Hydrogen Sulfide (A4)
☐ Stratified Layers (A5) (**LRR C**)
☐ 1 cm Muck (A9) (**LRR D**)
☐ Depleted Below Dark Surface (A11)
☐ Thick Dark Surface (A12)
☐ Sandy Mucky Mineral (S1)
☐ Sandy Gleyed Matrix (S4)

☐ Sandy Redox (S5)
☐ Stripped Matrix (S6)
☐ Loamy Mucky Mineral (F1)
☐ Loamy Gleyed Matrix (F2)
☐ Depleted Matrix (F3)
☐ Redox Dark Surface (F6)
☐ Depleted Dark Surface (F7)
☐ Redox Depressions (F8)
☐ Vernal Pools (F9)

Indicators for Problematic Hydric Soils³:

☐ 1 cm Muck (A9) (**LRR C**)
☐ 2 cm Muck (A10) (**LRR B**)
☐ Reduced Vertic (F18)
☐ Red Parent Material (TF2)
☐ Other (Explain in Remarks)

³Indicators of hydrophytic vegetation and wetland hydrology must be present, unless disturbed or problematic.

Restrictive Layer (if present):

Type: _____

Depth (inches): _____

Hydric Soil Present? Yes _____ No ☒

Remarks:

No hydric soil indicators are present.

HYDROLOGY**Wetland Hydrology Indicators:**

Primary Indicators (minimum of one required; check all that apply)

☐ Surface Water (A1)
☐ High Water Table (A2)
☐ Saturation (A3)
☐ Water Marks (B1) (**Nonriverine**)
☐ Sediment Deposits (B2) (**Nonriverine**)
☐ Drift Deposits (B3) (**Nonriverine**)
☐ Surface Soil Cracks (B6)
☐ Inundation Visible on Aerial Imagery (B7)
☐ Water-Stained Leaves (B9)

☐ Salt Crust (B11)
☐ Biotic Crust (B12)
☐ Aquatic Invertebrates (B13)
☐ Hydrogen Sulfide Odor (C1)
☐ Oxidized Rhizospheres along Living Roots (C3)
☐ Presence of Reduced Iron (C4)
☐ Recent Iron Reduction in Tilled Soils (C6)
☐ Thin Muck Surface (C7)
☐ Other (Explain in Remarks)

Secondary Indicators (2 or more required)

☐ Water Marks (B1) (**Riverine**)
☐ Sediment Deposits (B2) (**Riverine**)
☐ Drift Deposits (B3) (**Riverine**)
☐ Drainage Patterns (B10)
☐ Dry-Season Water Table (C2)
☐ Crayfish Burrows (C8)
☐ Saturation Visible on Aerial Imagery (C9)
☐ Shallow Aquitard (D3)
☐ FAC-Neutral Test (D5)

Field Observations:Surface Water Present? Yes _____ No ☒ Depth (inches): _____Water Table Present? Yes _____ No ☒ Depth (inches): _____Saturation Present? Yes _____ No ☒ Depth (inches): _____
(includes capillary fringe)**Wetland Hydrology Present?** Yes _____ No ☒

Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

Remarks:

No wetland hydrology indicators are present.

WETLAND DETERMINATION DATA FORM – Arid West Region

Project/Site: 3 Bear Recycling Center City/County: Eddy County Sampling Date: 07/11/2018
 Applicant/Owner: 3Bear Energy LLC State: NM Sampling Point: WDP 2
 Investigator(s): Ryan Blankenship and Garrett Weiberg Section, Township, Range: Section 23, Township 23S, 26E
 Landform (hillslope, terrace, etc.): vegetated flat Local relief (concave, convex, none): none Slope (%): 0-1
 Subregion (LRR): D - West range and irrigated Lat: 32.287643 Long: -104.257635 Datum: NAD83
 Soil Map Unit Name: Reagan-Upton association, 0 to 9 percent slopes NWI classification: none

Are climatic / hydrologic conditions on the site typical for this time of year? Yes ☒ No ☐ (If no, explain in Remarks.)
 Are Vegetation ☐, Soil ☐, or Hydrology ☐ significantly disturbed? Are "Normal Circumstances" present? Yes ☒ No ☐
 Are Vegetation ☐, Soil ☐, or Hydrology ☐ naturally problematic? (If needed, explain any answers in Remarks.)

SUMMARY OF FINDINGS – Attach site map showing sampling point locations, transects, important features, etc.

| | |
|--|--|
| Hydrophytic Vegetation Present? Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> | Is the Sampled Area within a Wetland? Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> |
| Hydric Soil Present? Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> | |
| Wetland Hydrology Present? Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> | |
| Remarks: None of the three necessary wetland indicators are present. The WDP is not located within a wetland. | |

VEGETATION – Use scientific names of plants.

| Tree Stratum (Plot size: <u>30'</u>) | Absolute % Cover | Dominant Species? | Indicator Status | Dominance Test worksheet: Number of Dominant Species That Are OBL, FACW, or FAC: <u>0</u> (A) Total Number of Dominant Species Across All Strata: <u>3</u> (B) Percent of Dominant Species That Are OBL, FACW, or FAC: <u>0</u> (A/B) | |
|--|------------------|-------------------|------------------|---|--|
| 1. <u>None</u> | | | | | |
| 2. _____ | | | | | |
| 3. _____ | | | | | |
| 4. _____ | | | | | |
| <u>0</u> = Total Cover | | | | | |
| Sapling/Shrub Stratum (Plot size: <u>15'</u>) | | | | Prevalence Index worksheet: Total % Cover of: _____ Multiply by: _____ OBL species _____ x 1 = _____ FACW species _____ x 2 = _____ FAC species _____ x 3 = _____ FACU species _____ x 4 = _____ UPL species _____ x 5 = _____ Column Totals: _____ (A) _____ (B) Prevalence Index = B/A = _____ | |
| 1. <u>Larrea tridentata</u> | <u>20</u> | <u>Y</u> | <u>UPL</u> | | |
| 2. _____ | | | | | |
| 3. _____ | | | | | |
| 4. _____ | | | | | |
| <u>20</u> = Total Cover | | | | | |
| Herb Stratum (Plot size: <u>5'</u>) | | | | Hydrophytic Vegetation Indicators: ___ Dominance Test is >50% ___ Prevalence Index is ≤3.0 ¹ ___ Morphological Adaptations ¹ (Provide supporting data in Remarks or on a separate sheet) ___ Problematic Hydrophytic Vegetation ¹ (Explain) | |
| 1. <u>Bothriochloa barbinodis</u> | <u>20</u> | <u>Y</u> | <u>UPL</u> | | |
| 2. <u>Gutierrezia sarothrae</u> | <u>10</u> | <u>Y</u> | <u>UPL</u> | | |
| 3. <u>Erioneuron pilosum</u> | <u>5</u> | <u>N</u> | <u>UPL</u> | | |
| 4. <u>Tridens muticus</u> | <u>5</u> | <u>N</u> | <u>UPL</u> | | |
| <u>40</u> = Total Cover | | | | | |
| Woody Vine Stratum (Plot size: <u>30'</u>) | | | | Hydrophytic Vegetation Present? Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> | |
| 1. <u>None</u> | | | | | |
| 2. _____ | | | | | |
| <u>0</u> = Total Cover | | | | | |
| % Bare Ground in Herb Stratum <u>50</u> % Cover of Biotic Crust <u>0</u> | | | | | |

¹Indicators of hydric soil and wetland hydrology must be present, unless disturbed or problematic.

Hydrophytic Vegetation Present?

Yes ☐ No ☒

Remarks:

The vegetative community did not pass the dominance test.

SOIL

Sampling Point: WDP 2

[illegible]

HYDROLOGY

| Wetland Hydrology Indicators: | | |
|--|--|---|
| Primary Indicators (minimum of one required; check all that apply) | | Secondary Indicators (2 or more required) |
| <input type="checkbox"/> Surface Water (A1) | <input type="checkbox"/> Salt Crust (B11) | <input type="checkbox"/> Water Marks (B1) (Riverine) |
| <input type="checkbox"/> High Water Table (A2) | <input type="checkbox"/> Biotic Crust (B12) | <input type="checkbox"/> Sediment Deposits (B2) (Riverine) |
| <input type="checkbox"/> Saturation (A3) | <input type="checkbox"/> Aquatic Invertebrates (B13) | <input type="checkbox"/> Drift Deposits (B3) (Riverine) |
| <input type="checkbox"/> Water Marks (B1) (Nonriverine) | <input type="checkbox"/> Hydrogen Sulfide Odor (C1) | <input type="checkbox"/> Drainage Patterns (B10) |
| <input type="checkbox"/> Sediment Deposits (B2) (Nonriverine) | <input type="checkbox"/> Oxidized Rhizospheres along Living Roots (C3) | <input type="checkbox"/> Dry-Season Water Table (C2) |
| <input type="checkbox"/> Drift Deposits (B3) (Nonriverine) | <input type="checkbox"/> Presence of Reduced Iron (C4) | <input type="checkbox"/> Crayfish Burrows (C8) |
| <input type="checkbox"/> Surface Soil Cracks (B6) | <input type="checkbox"/> Recent Iron Reduction in Tilled Soils (C6) | <input type="checkbox"/> Saturation Visible on Aerial Imagery (C9) |
| <input type="checkbox"/> Inundation Visible on Aerial Imagery (B7) | <input type="checkbox"/> Thin Muck Surface (C7) | <input type="checkbox"/> Shallow Aquitard (D3) |
| <input type="checkbox"/> Water-Stained Leaves (B9) | <input type="checkbox"/> Other (Explain in Remarks) | <input type="checkbox"/> FAC-Neutral Test (D5) |
| Field Observations: Surface Water Present? Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> Depth (inches): _____ Water Table Present? Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> Depth (inches): _____ Saturation Present? Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> Depth (inches): _____ (includes capillary fringe) | | Wetland Hydrology Present? Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> |
| Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available: | | |
| Remarks: | | |
| No wetland hydrology indicators are present. | | |

WETLAND DETERMINATION DATA FORM – Arid West Region

Project/Site: 3 Bear Recycling Center City/County: Eddy County Sampling Date: 07/11/2018
 Applicant/Owner: 3Bear Energy LLC State: NM Sampling Point: WDP 3
 Investigator(s): Ryan Blankenship and Garrett Weiberg Section, Township, Range: Section 23, Township 23S, 26E
 Landform (hillslope, terrace, etc.): vegetated flat Local relief (concave, convex, none): none Slope (%): 0-1
 Subregion (LRR): D - West range and irrigated Lat: 32.286644 Long: -104.256849 Datum: NAD83
 Soil Map Unit Name: Reagan-Upton association, 0 to 9 percent slopes NWI classification: none

Are climatic / hydrologic conditions on the site typical for this time of year? Yes ☒ No ☐ (If no, explain in Remarks.)
 Are Vegetation ☐, Soil ☐, or Hydrology ☐ significantly disturbed? Are "Normal Circumstances" present? Yes ☒ No ☐
 Are Vegetation ☐, Soil ☐, or Hydrology ☐ naturally problematic? (If needed, explain any answers in Remarks.)

SUMMARY OF FINDINGS – Attach site map showing sampling point locations, transects, important features, etc.

| | | | |
|--|---|--|---|
| Hydrophytic Vegetation Present? | Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> | Is the Sampled Area within a Wetland? | Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> |
| Hydric Soil Present? | Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> | | |
| Wetland Hydrology Present? | Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> | | |
| Remarks: None of the three necessary wetland indicators are present. The WDP is not located within a wetland. | | | |

VEGETATION – Use scientific names of plants.

| | |
|--|---|
| Tree Stratum (Plot size: <u>30'</u>) 1. <u>None</u> 2. _____ 3. _____ 4. _____ _____ = Total Cover Sapling/Shrub Stratum (Plot size: <u>15'</u>) 1. <u>Larrea tridentata</u> 2. _____ 3. _____ 4. _____ 5. _____ _____ = Total Cover Herb Stratum (Plot size: <u>5'</u>) 1. <u>Bothriochloa barbinodis</u> 2. <u>Erioneuron pilosum</u> 3. <u>Gutierrezia sarothrae</u> 4. <u>Croton fruticosus</u> 5. _____ 6. _____ 7. _____ 8. _____ _____ = Total Cover Woody Vine Stratum (Plot size: <u>30'</u>) 1. <u>None</u> 2. _____ _____ = Total Cover % Bare Ground in Herb Stratum <u>50</u> % Cover of Biotic Crust <u>0</u> | Dominance Test worksheet: Number of Dominant Species That Are OBL, FACW, or FAC: <u>0</u> (A) Total Number of Dominant Species Across All Strata: <u>4</u> (B) Percent of Dominant Species That Are OBL, FACW, or FAC: <u>0</u> (A/B) Prevalence Index worksheet: Total % Cover of: _____ Multiply by: _____ OBL species _____ x 1 = _____ FACW species _____ x 2 = _____ FAC species _____ x 3 = _____ FACU species _____ x 4 = _____ UPL species _____ x 5 = _____ Column Totals: _____ (A) _____ (B) Prevalence Index = B/A = _____ Hydrophytic Vegetation Indicators: ___ Dominance Test is >50% ___ Prevalence Index is ≤3.0 ¹ ___ Morphological Adaptations ¹ (Provide supporting data in Remarks or on a separate sheet) ___ Problematic Hydrophytic Vegetation ¹ (Explain) ¹ Indicators of hydric soil and wetland hydrology must be present, unless disturbed or problematic. Hydrophytic Vegetation Present? Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> |
|--|---|

Remarks:
The vegetative community did not pass the dominance test.

SOIL

Sampling Point: WDP 3

[illegible]

HYDROLOGY

| Wetland Hydrology Indicators: | | |
|---|--|---|
| Primary Indicators (minimum of one required; check all that apply) | | Secondary Indicators (2 or more required) |
| <input type="checkbox"/> Surface Water (A1) | <input type="checkbox"/> Salt Crust (B11) | <input type="checkbox"/> Water Marks (B1) (Riverine) |
| <input type="checkbox"/> High Water Table (A2) | <input type="checkbox"/> Biotic Crust (B12) | <input type="checkbox"/> Sediment Deposits (B2) (Riverine) |
| <input type="checkbox"/> Saturation (A3) | <input type="checkbox"/> Aquatic Invertebrates (B13) | <input type="checkbox"/> Drift Deposits (B3) (Riverine) |
| <input type="checkbox"/> Water Marks (B1) (Nonriverine) | <input type="checkbox"/> Hydrogen Sulfide Odor (C1) | <input type="checkbox"/> Drainage Patterns (B10) |
| <input type="checkbox"/> Sediment Deposits (B2) (Nonriverine) | <input type="checkbox"/> Oxidized Rhizospheres along Living Roots (C3) | <input type="checkbox"/> Dry-Season Water Table (C2) |
| <input type="checkbox"/> Drift Deposits (B3) (Nonriverine) | <input type="checkbox"/> Presence of Reduced Iron (C4) | <input type="checkbox"/> Crayfish Burrows (C8) |
| <input type="checkbox"/> Surface Soil Cracks (B6) | <input type="checkbox"/> Recent Iron Reduction in Tilled Soils (C6) | <input type="checkbox"/> Saturation Visible on Aerial Imagery (C9) |
| <input type="checkbox"/> Inundation Visible on Aerial Imagery (B7) | <input type="checkbox"/> Thin Muck Surface (C7) | <input type="checkbox"/> Shallow Aquitard (D3) |
| <input type="checkbox"/> Water-Stained Leaves (B9) | <input type="checkbox"/> Other (Explain in Remarks) | <input type="checkbox"/> FAC-Neutral Test (D5) |
| Field Observations: Surface Water Present? Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> Depth (inches): _____ Water Table Present? Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> Depth (inches): _____ Saturation Present? Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> Depth (inches): _____ (includes capillary fringe) | | Wetland Hydrology Present? Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> |
| Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available: | | |
| Remarks: No wetland hydrology indicators are present. | | |

WETLAND DETERMINATION DATA FORM – Arid West Region

Project/Site: 3 Bear Recycling Center City/County: Eddy County Sampling Date: 07/11/2018
 Applicant/Owner: 3Bear Energy LLC State: NM Sampling Point: WDP 4
 Investigator(s): Ryan Blankenship and Garrett Weiberg Section, Township, Range: Section 23, Township 23S, 26E
 Landform (hillslope, terrace, etc.): depression Local relief (concave, convex, none): concave Slope (%): 0-1
 Subregion (LRR): D - West range and irrigated Lat: 32.285855 Long: -104.256615 Datum: NAD83
 Soil Map Unit Name: Reagan-Upton association, 0 to 9 percent slopes NWI classification: none

Are climatic / hydrologic conditions on the site typical for this time of year? Yes ☒ No ☐ (If no, explain in Remarks.)
 Are Vegetation ☐, Soil ☐, or Hydrology ☐ significantly disturbed? Are "Normal Circumstances" present? Yes ☒ No ☐
 Are Vegetation ☐, Soil ☐, or Hydrology ☐ naturally problematic? (If needed, explain any answers in Remarks.)

SUMMARY OF FINDINGS – Attach site map showing sampling point locations, transects, important features, etc.

| | |
|--|--|
| Hydrophytic Vegetation Present? Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> | Is the Sampled Area within a Wetland? Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> |
| Hydric Soil Present? Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> | |
| Wetland Hydrology Present? Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> | |
| Remarks: None of the three necessary wetland indicators are present. The WDP is not located within a wetland. | |

VEGETATION – Use scientific names of plants.

| Tree Stratum (Plot size: <u>30'</u>) | Absolute % Cover | Dominant Species? | Indicator Status | Dominance Test worksheet: Number of Dominant Species That Are OBL, FACW, or FAC: <u>0</u> (A) Total Number of Dominant Species Across All Strata: <u>4</u> (B) Percent of Dominant Species That Are OBL, FACW, or FAC: <u>0</u> (A/B) |
|--|------------------|-------------------|------------------|---|
| 1. <u>None</u> | | | | |
| 2. _____ | | | | |
| 3. _____ | | | | |
| 4. _____ | | | | |
| <u>Sapling/Shrub Stratum</u> (Plot size: <u>15'</u>) <div> 1. <u>Larrea tridentata</u> <div>5</div> </div> <div> 2. _____ <div></div> </div> <div> 3. _____ <div></div> </div> <div> 4. _____ <div></div> </div> <div> 5. _____ <div>5 = Total Cover</div> </div> | | | | Prevalence Index worksheet: Total % Cover of: _____ Multiply by: _____ OBL species _____ x 1 = _____ FACW species _____ x 2 = _____ FAC species _____ x 3 = _____ FACU species _____ x 4 = _____ UPL species _____ x 5 = _____ Column Totals: _____ (A) _____ (B) Prevalence Index = B/A = _____ |
| <u>Herb Stratum</u> (Plot size: <u>5'</u>) <div> 1. <u>Gutierrezia sarothrae</u> <div>10</div> </div> <div> 2. <u>Croton fruticosus</u> <div>10</div> </div> <div> 3. <u>Psilostrophe tagentina</u> <div>5</div> </div> <div> 4. _____ <div></div> </div> <div> 5. _____ <div></div> </div> <div> 6. _____ <div></div> </div> <div> 7. _____ <div></div> </div> <div> 8. _____ <div>25 = Total Cover</div> </div> | | | | |
| <u>Woody Vine Stratum</u> (Plot size: <u>30'</u>) <div> 1. <u>None</u> <div></div> </div> <div> 2. _____ <div>0 = Total Cover</div> </div> | | | | |
| % Bare Ground in Herb Stratum <u>80</u> % Cover of Biotic Crust _____ | | | | |
| Remarks: | | | | |

Hydrophytic Vegetation Indicators:
 ___ Dominance Test is >50%
 ___ Prevalence Index is ≤3.0¹
 ___ Morphological Adaptations¹ (Provide supporting data in Remarks or on a separate sheet)
 ___ Problematic Hydrophytic Vegetation¹ (Explain)

¹Indicators of hydric soil and wetland hydrology must be present, unless disturbed or problematic.

Hydrophytic Vegetation Present? Yes ☐ No ☒

The vegetative community did not pass the dominance test.

SOIL

Sampling Point: WDP 4

Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)

| Depth (inches) | Matrix | | Redox Features | | | | Texture | Remarks |
|-------------------|---------------|-----|----------------|---|-------------------|------------------|------------|--------------|
| | Color (moist) | % | Color (moist) | % | Type ¹ | Loc ² | | |
| 0-6 | 7.5YR 4/3 | 100 | none | | | | silty clay | mixed gravel |
| | | | | | | | | |
| | | | | | | | | |
| | | | | | | | | |
| | | | | | | | | |
| | | | | | | | | |
| | | | | | | | | |
| | | | | | | | | |
| | | | | | | | | |
| | | | | | | | | |

¹Type: C=Concentration, D=Depletion, RM=Reduced Matrix, CS=Covered or Coated Sand Grains. ²Location: PL=Pore Lining, M=Matrix.

Hydric Soil Indicators: (Applicable to all LRRs, unless otherwise noted.)

- ☐ Histosol (A1)
- ☐ Histic Epipedon (A2)
- ☐ Black Histic (A3)
- ☐ Hydrogen Sulfide (A4)
- ☐ Stratified Layers (A5) (**LRR C**)
- ☐ 1 cm Muck (A9) (**LRR D**)
- ☐ Depleted Below Dark Surface (A11)
- ☐ Thick Dark Surface (A12)
- ☐ Sandy Mucky Mineral (S1)
- ☐ Sandy Gleyed Matrix (S4)

- ☐ Sandy Redox (S5)
- ☐ Stripped Matrix (S6)
- ☐ Loamy Mucky Mineral (F1)
- ☐ Loamy Gleyed Matrix (F2)
- ☐ Depleted Matrix (F3)
- ☐ Redox Dark Surface (F6)
- ☐ Depleted Dark Surface (F7)
- ☐ Redox Depressions (F8)
- ☐ Vernal Pools (F9)

Indicators for Problematic Hydric Soils³:

- ☐ 1 cm Muck (A9) (**LRR C**)
- ☐ 2 cm Muck (A10) (**LRR B**)
- ☐ Reduced Vertic (F18)
- ☐ Red Parent Material (TF2)
- ☐ Other (Explain in Remarks)

³Indicators of hydrophytic vegetation and wetland hydrology must be present, unless disturbed or problematic.

Restrictive Layer (if present):

Type: rock
Depth (inches): 6

Hydric Soil Present? Yes ☐ No ☒

Remarks:

No hydric soil indicators are present.

HYDROLOGY

Wetland Hydrology Indicators:

Primary Indicators (minimum of one required; check all that apply)

- ☐ Surface Water (A1)
- ☐ High Water Table (A2)
- ☐ Saturation (A3)
- ☐ Water Marks (B1) (**Nonriverine**)
- ☐ Sediment Deposits (B2) (**Nonriverine**)
- ☐ Drift Deposits (B3) (**Nonriverine**)
- ☐ Surface Soil Cracks (B6)
- ☐ Inundation Visible on Aerial Imagery (B7)
- ☐ Water-Stained Leaves (B9)

- ☐ Salt Crust (B11)
- ☐ Biotic Crust (B12)
- ☐ Aquatic Invertebrates (B13)
- ☐ Hydrogen Sulfide Odor (C1)
- ☐ Oxidized Rhizospheres along Living Roots (C3)
- ☐ Presence of Reduced Iron (C4)
- ☐ Recent Iron Reduction in Tilled Soils (C6)
- ☐ Thin Muck Surface (C7)
- ☐ Other (Explain in Remarks)

Secondary Indicators (2 or more required)

- ☐ Water Marks (B1) (**Riverine**)
- ☐ Sediment Deposits (B2) (**Riverine**)
- ☐ Drift Deposits (B3) (**Riverine**)
- ☐ Drainage Patterns (B10)
- ☐ Dry-Season Water Table (C2)
- ☐ Crayfish Burrows (C8)
- ☐ Saturation Visible on Aerial Imagery (C9)
- ☐ Shallow Aquitard (D3)
- ☐ FAC-Neutral Test (D5)

Field Observations:

Surface Water Present? Yes ☐ No ☒ Depth (inches): _____
Water Table Present? Yes ☐ No ☒ Depth (inches): _____
Saturation Present? Yes ☐ No ☒ Depth (inches): _____
(includes capillary fringe)

Wetland Hydrology Present? Yes ☐ No ☒

Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

Remarks:

No wetland hydrology indicators are present.

WETLAND DETERMINATION DATA FORM – Arid West Region

Project/Site: 3 Bear Recycling Center City/County: Eddy County Sampling Date: 07/11/2018
 Applicant/Owner: 3Bear Energy LLC State: NM Sampling Point: WDP 5
 Investigator(s): Ryan Blankenship and Garrett Weiberg Section, Township, Range: Section 23, Township 23S, 26E
 Landform (hillslope, terrace, etc.): depression Local relief (concave, convex, none): concave Slope (%): 0-1
 Subregion (LRR): D - West range and irrigated Lat: 32.285715 Long: -104.256763 Datum: NAD83
 Soil Map Unit Name: Reagan-Upton association, 0 to 9 percent slopes NWI classification: PEM1Jx

Are climatic / hydrologic conditions on the site typical for this time of year? Yes ☒ No ☐ (If no, explain in Remarks.)
 Are Vegetation ☐, Soil ☐, or Hydrology ☐ significantly disturbed? Are "Normal Circumstances" present? Yes ☒ No ☐
 Are Vegetation ☐, Soil ☐, or Hydrology ☐ naturally problematic? (If needed, explain any answers in Remarks.)

SUMMARY OF FINDINGS – Attach site map showing sampling point locations, transects, important features, etc.

| | |
|--|--|
| Hydrophytic Vegetation Present? Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> | Is the Sampled Area within a Wetland? Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> |
| Hydric Soil Present? Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> | |
| Wetland Hydrology Present? Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> | |
| Remarks: None of the three necessary wetland indicators are present. The WDP is not located within a wetland. | |

VEGETATION – Use scientific names of plants.

| Tree Stratum (Plot size: <u>30'</u>) | Absolute % Cover | Dominant Species? | Indicator Status | Dominance Test worksheet: Number of Dominant Species That Are OBL, FACW, or FAC: <u>1</u> (A) Total Number of Dominant Species Across All Strata: <u>2</u> (B) Percent of Dominant Species That Are OBL, FACW, or FAC: <u>50</u> (A/B) |
|---|------------------|-------------------|------------------|---|
| 1. <u>None</u> | | | | |
| 2. _____ | | | | |
| 3. _____ | | | | |
| 4. _____ | | | | |
| <u>0</u> = Total Cover | | | | Prevalence Index worksheet: Total % Cover of: _____ Multiply by: _____ OBL species _____ x 1 = _____ FACW species _____ x 2 = _____ FAC species _____ x 3 = _____ FACU species _____ x 4 = _____ UPL species _____ x 5 = _____ Column Totals: _____ (A) _____ (B) Prevalence Index = B/A = _____ |
| Sapling/Shrub Stratum (Plot size: <u>15'</u>) | | | | |
| 1. <u>None</u> | | | | |
| 2. _____ | | | | |
| 3. _____ | | | | |
| 4. _____ | | | | |
| 5. _____ | | | | |
| <u>0</u> = Total Cover | | | | |
| Herb Stratum (Plot size: <u>5'</u>) | | | | |
| 1. <u>Glandularia bipinnatifida</u> | <u>25</u> | <u>Y</u> | <u>UPL</u> | |
| 2. <u>Tridens muticus</u> | <u>15</u> | <u>Y</u> | <u>FAC</u> | |
| 3. <u>Sphaeralcea angustifolia</u> | <u>10</u> | <u>N</u> | <u>UPL</u> | |
| 4. <u>Chamaesyce chaetocalyx</u> | <u>5</u> | <u>N</u> | <u>UPL</u> | |
| 5. _____ | | | | |
| 6. _____ | | | | |
| 7. _____ | | | | |
| 8. _____ | | | | |
| <u>55</u> = Total Cover | | | | |
| Woody Vine Stratum (Plot size: <u>30'</u>) | | | | |
| 1. <u>None</u> | | | | |
| 2. _____ | | | | |
| <u>0</u> = Total Cover | | | | |
| % Bare Ground in Herb Stratum <u>60</u> % Cover of Biotic Crust _____ | | | | |
| Remarks: The vegetative community did not pass the dominance test. | | | | |
| | | | | |

SOIL

Sampling Point: WDP 5

[illegible]

HYDROLOGY

| Wetland Hydrology Indicators: | | |
|--|--|---|
| Primary Indicators (minimum of one required; check all that apply) | | Secondary Indicators (2 or more required) |
| <input type="checkbox"/> Surface Water (A1) | <input type="checkbox"/> Salt Crust (B11) | <input type="checkbox"/> Water Marks (B1) (Riverine) |
| <input type="checkbox"/> High Water Table (A2) | <input type="checkbox"/> Biotic Crust (B12) | <input type="checkbox"/> Sediment Deposits (B2) (Riverine) |
| <input type="checkbox"/> Saturation (A3) | <input type="checkbox"/> Aquatic Invertebrates (B13) | <input type="checkbox"/> Drift Deposits (B3) (Riverine) |
| <input type="checkbox"/> Water Marks (B1) (Nonriverine) | <input type="checkbox"/> Hydrogen Sulfide Odor (C1) | <input type="checkbox"/> Drainage Patterns (B10) |
| <input type="checkbox"/> Sediment Deposits (B2) (Nonriverine) | <input type="checkbox"/> Oxidized Rhizospheres along Living Roots (C3) | <input type="checkbox"/> Dry-Season Water Table (C2) |
| <input type="checkbox"/> Drift Deposits (B3) (Nonriverine) | <input type="checkbox"/> Presence of Reduced Iron (C4) | <input type="checkbox"/> Crayfish Burrows (C8) |
| <input type="checkbox"/> Surface Soil Cracks (B6) | <input type="checkbox"/> Recent Iron Reduction in Tilled Soils (C6) | <input type="checkbox"/> Saturation Visible on Aerial Imagery (C9) |
| <input type="checkbox"/> Inundation Visible on Aerial Imagery (B7) | <input type="checkbox"/> Thin Muck Surface (C7) | <input type="checkbox"/> Shallow Aquitard (D3) |
| <input type="checkbox"/> Water-Stained Leaves (B9) | <input type="checkbox"/> Other (Explain in Remarks) | <input type="checkbox"/> FAC-Neutral Test (D5) |
| Field Observations: Surface Water Present? Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> Depth (inches): _____ Water Table Present? Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> Depth (inches): _____ Saturation Present? Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> Depth (inches): _____ (includes capillary fringe) | | Wetland Hydrology Present? Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> |
| Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available: | | |
| Remarks: | | |
| No wetland hydrology indicators are present. | | |